

Lones Schuler Ross

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SURVEYING

AND

NAVIGATION,

WITH A PRELIMINARY TREATISE ON

TRIGONOMETRY AND MENSURATION,

BE

A. SCHUYLER, M. A.,

Professor of Mathematics and Philosophy in Kansan Westerna University.

Anthor of Principles of Logic, Empirical and Rational Psychology
and of a Series of Mathematical Works.

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PREFACE.

Nearly twenty years ago the Publishers made the following announcement: "Surveying and Nacigation; containing Surveying and Leveling, Navigation, Barometric Heights, etc."

To redeem this promise, the present work now appears.

It is customary to preface works on Surveying by a meager sketch of Plane Trigonometry, but it has been thought best to include in this work a thorough treatment of Plane and Spherical Trigonometry and Mensuration. These subjects have been treated in view of the wants of our best High Schools and Colleges.

Certain modern writers have defined the Trigonometric functions as ratios; for example, in a right triangle, the sine of an angle is the ratio of the opposite side to the hypotenuse, etc.

The historical method of considering the sine, co-sine, tangent, etc., as linear functions of the arc, explains the origin of these terms—avoids the ambiguity of the word ratio; explains how the logarithm of the sine, for example, can reach the limit 10, which would be impossible if the limit of the sine itself is 1, and is much more readily apprehended by the student.

The advantages in analytic investigations resulting from defining these functions as ratios have been secured in the principles relating to the Right Triangle, Art. 64.

Each of the circular functions has, in the first place, been considered by itself, and its value traced, for all arcs, from 0 to 360°.

Then follows the solution of triangles, right and oblique, the general relations of the circular functions, the functions of the sum or difference of two angles, and a variety of interesting practical applications.

It is hoped that Spherical Trigonometry has been made intelligible to the diligent student. More than ordinary care has been given to the development of Napier's principles, and to the discussion of the species of the parts of both right and oblique spherical triangles, Arts. 126, 129, 145, 148, 151.

Mensuration, a subject at once interesting and practically important, has been discussed at length, and formulas have been developed instead of rules for the solution of the problems.

In the Surveying, the instruments are first represented and described, and the methods of making the adjustments given in detail.

The Author takes this opportunity to express his obligations to Messrs. W. & L. E. Gurley, Manufacturers of Surveying and Engineering Instruments, Troy, N. Y., who have kindly granted him the use of their Manual for the delineation and description of the instruments. In consequence of this courtesy, much better drawings and descriptions have been made than would otherwise have been possible.

The instruments themselves should, however, be accessible to the student, who should study them in connection with the descriptions in the book, and learn to use them in practical work, guided by a competent instructor.

The Rectangular method of surveying the Public lands, now, brought to great perfection under the direction of the Government, has been minutely explained, and illustrated by field notes of actual surveys. In this portion of the work, the United States Manual of Surveying Instructions has been taken as authority, and thus the authorized methods, which must form the basis for subsequent surveys, have been made accessible to the student.

The methods of finding the true meridian and the variation of the needle have been given at length; also specific direc-

tions for finding corners, taking bearings, measuring lines, recording field notes, and plotting.

In addition to the ordinary method of finding the area, a new method, developed by E. M. Pogue, of Kentucky, is given in Art. 304. This method has the merit of giving always a uniform result from the same field notes, and thus avoids disputes about the different results of the ordinary method, unavoidably attending the various distribution of errors by different calculators.

The methods of supplying omissions are explained and illustrated by examples.

Laying out and dividing land, operations admitting of an unlimited variety of applications, have been treated in view of the wants of the practical surveyor. The subject is also full of interest to the student, who can not fail to receive from it new views of the resources of mathematical science.

Leveling, the construction of railroad curves, embankments and excavations, the method of making Topographical surveys, with the authorized conventional symbols, Berometric beights, etc., have been explained and illustrated by diagrams and examples.

It has been thought best to give a clear, elementary treatment of Navigation, not only on account of those who may desire to pursue the subject further, but for the take of gratifying the wishes of intelligent persons who may desire to know something of Navigation. The limits of the work, however, forbid the discussion of Nautical Astronomy. The examples in Navigation have been selected from the English work of J. R. Young.

The tables of Logarithms, Natural and Logarithmic sines, etc., have been carried only to five decimal places, and for the purposes intended will be found practically better than tables to six or seven places.

The Traverse table has been thrown into a new form, at once condensed and convenient.

These tables have been compiled by Mr. Henry H. Vail, and

by him compared with Babbage's and Wittstein's tables, then by the Author with Vega's tables to seven decimal places. It is hoped that by this double comparison perfect accuracy has been attained.

The table of Meridional Parts, taken from "Projection Tables for the use of the United States Navy," prepared by the Bureau of Navigation, and issued from the Government Printing office, was calculated in the Hydrographic office for the terrestrial spheroid, compression Tables. This table, now for the first time published in a text-book, is believed to be more correct than those in general use.

The Author takes pleasure in acknowledging his obligations to Prof. E. H. Warner for critical suggestions and acceptable aid in reading proof and testing the accuracy of the answers.

With the hope that the book will be attractive and useful to the student, teacher, and practical surveyor, it is sent forth to accomplish its work.

A. SCHUYLER.

Baldwin University, Benea, O., June 12, 1878.

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INTRODUCTION.

LOGARITHMS.

1. Definition.

A logarithm of a number is the exponent denoting the power to which a fixed number, called the base, must be raised in order to produce the given number.

Thus, in the equation, $b^x = n$, b is the base of the system, n is the number whose logarithm is to be taken, and x is the logarithm of n to the base b, which may be written: $x = \log_x n$.

Any positive number, except 1, may be assumed as the base, but when assumed, it remains fixed for a system; hence, there may be an infinite number of systems, since there may be an infinite number of bases.

2. Common Logarithms.

Common logarithms are the logarithms of numbers in the system whose base is 10.

$$10^{\circ} = 1$$
; '.'. by def., $\log 1 = 0$.
 $10^{\circ} = 10$; .'. by def., $\log 10 = 1$.
 $10^{\circ} = 100$; .'. by def., $\log 100 = 2$.
 $10^{\circ} = 1000$; .'. by def., $\log 1000 = 3$.

Hence, In the common system, the logarithm of an exact power of 10 is the whole number equal to the exponent of the power.

THE CHARACTERISTIC.

3. Consequences.

1. If the number is greater than 1 and less than 10, its logarithm is greater than 0 and less than 1, or is 0 + a decimal.

2. If the number is greater than 10 and less than 100, its logarithm is greater than 1 and less than 2, or is 1 -1- a decimal.

3. In general, if the number is not an exact power of 10, its logarithm, in the common system, will consist of two parts—an entire part and a decimal part.

The entire part is called the characteristic and the decimal part is called the mantissa.

4. Problem.

To find the laws for the characteristic.

Let (1) $10^s = n$; then, by def., $\log n = x$. But (2) $10^s = 10$.

(1)
$$\div$$
 (2) = (3) $10^{n-1} = \frac{n}{10}$; then, by def., $\log \frac{n}{10} = x - 1$.
 $\cdot \cdot \cdot \log \frac{n}{10} = \log n - 1$.

Hence, The logarithm of the quotient of any number by 10 is less by 1 than the logarithm of the number.

Let us now take the number 8979 and its logarithm 3.95323, as given in a table of logarithms, and divide the number successively by 10, and for each division subtract 1 from the logarithm of the dividend, then we have,

Log 8979 = 3.95323. 897.9 = 2.95323. 897.9 = 1.95323. 89.79 = 1.95323. 89.79 = 0.95323. 8979 = 0.95323. 8979 = 3.95323. 8979 = 3.95323. The minus sign applies only to the characteristic over which it is placed.

The mantissa is always positive, and is the same for all positions of the decimal point.

An inspection of the above will reveal the following laws:

1. If the number is integral or mixed, the characteristic is positive and is one less than the number of integral figures.

2. If the number is entirely decimal, the characteristic is negative and is one greater, numerically, than the number of O's immediately following the decimal point.

5. Exercises on the Characteristic.

- 1. What is the characteristic of the logarithm of 7?
- 2. What is the characteristic of the logarithm of 465?
- 3. What is the characteristic of the logarithm of 4678?
- 4. What is the characteristic of the logarithm of 34.75?
- 5. What is the characteristic of the logarithm of .65?
- 6. What is the characteristic of the logarithm of .0789?
- 7. What is the characteristic of the logarithm of .00084?
- 8. If the characteristic of the logarithm of a number is 2, how many integral places has that number?
- 9. If the characteristic of the logarithm of a number is 5, how many integral places has that number?
- 10. If the characteristic of the logarithm of a number?
- 11. If the characteristic of the logarithm of a number is 0, how many integral places has that number?

12. If the characteristic of the logarithm of a number is negative, is the number integral, decimal, or mixed?

13. If the characteristic of the logarithm of a number is 4, how many 0's immediately follow the decimal point?

14. If the characteristic of the logarithm of a number is 2, how many 0's immediately follow the decimal point?

15. If the characteristic of the logarithm of a number is 1, how many 0's immediately follow the decimal point?

TABLE OF LOGARITHMS.

6. Description of the Table.

The table of logarithms annexed gives the mantissa of the logarithm of every number from 1000 to 10900. The characteristic can be found by the preceding laws.

It follows, from Art. 4, that the mantissa of the logarithm of a number is the same as the mantissa of the logarithm of the product or quotient of that number by any power of 10. Thus:

Log 12 = 1.07918.

" 120 = 2.07918.

" .012 = 2.07918.

Hence, we can determine from the table the logarithm of any number less than 1000. Thus, the mantissa of the logarithm of 8 is the same as that of the logarithm of 8000.

In the table, the first three or four figures of each number are given in the left-hand column, marked N. The next figure is given at the head and foot of one of the columns of mantissas.

The mantissas, in the column under 0, are given to five decimal places. The first and second decimal figures of this column are understood to be repeated in the spaces below, and to be prefixed, across the page, to the three figures of the remaining columns.

When the third decimal digit changes from 9 to 0, the second is increased by the 1 carried; and the corresponding mantissa, and all to the right, commence with a smaller figure, to indicate that the first two decimal figures, to be prefixed, are to be taken from the line below.

The last column, marked D, contains the difference of two successive mantissas, called the tabular difference.

7. Problem.

To find the logarithm of a given number.

1. Find the logarithm of 3675,

The characteristic is 3. Opposite 367, in the column headed N, and under the column headed 5, we find 526, to which prefix the two figures, 56, in the column headed 0, and we have for the mantises .56526.

2. Find the logarithm of 76.

The characteristic is 1, and the mantissa is the same as that of 7600, which is .88081.

3. Find the logarithm of .004268.

The characteristic is 3, and the mantissa is the same as that of 4268. Looking opposite 426, and under 8, we find 022, of which the 0 is a small figure. Prefixing

63, from the line below, in the column headed 0, we have for the mantissa .63022

. log 004268 3.63022.

4. Find the logarithm of 109684.

The characteristic = 5.

The mantissa of log 1006 = .03981

Tab. diff. is 40; and $40 \times .84 = -34$ $\log 109684 = 5.04015$

The reason for multiplying the tabular difference by M will be apparent from the following:

 $\log 109600 = 5.03981$, $\log 109700 = 5.04021$.

The difference of the logarithms is 40 hundred-thousandths, and the difference of the numbers is 100 but the difference of 109600 and 109684 is 84, which is 4 of 100; hence, the difference of the logarithms of 109600 and 109684 is .84 of 40 hundred-thousandths, which is 40 hundred-thousandths - .81 .34 hundred-thousandths, nearly.

It is assumed that the difference of the logarithms of two numbers is proportional to the difference of the numbers, which is approximately true, especially if the numbers are large.

5. Find the logarithm of 123,613.

The characteristic 2. The mantissa of $\log 1236 = .00202$ Tab. diff. is 35; and $85 \times .13 = .5$

-*. log 123.613 . 2.09207

The tabular difference is .00035, and .00035 × .13 = 0000455. But since the logarithms in this table are taken only to five decimal places, the two last figures,

55, are rejected, and 1 is carried to .00001, making .00005 for the correction.

In general, when the left-hand figure of the part rejected exceeds 4, carry 1.

When the tabular difference is large, as in the first part of the table, there may be small errors. Accordingly, for numbers between 10000 and 10500, it will be better to use the last two pages instead of the first page.

8. Rule.

1. If the number, or the product of the number by as a power of 10, is found in the table, take the corresponding name them from the table, and prefix the proper characteristic

2. If the number, without reference to the decimal point or 0's on the right, is expressed by more than five papers, take from the table the mantions corresponding to the first four or five figures on the left, multiply the corresponding to the first tabular difference by the number expressed by the remaining figures, considered as a decimal, reject from the product as many figures on the right as are in the multiplier, curement to the nearest unit, and add the result as so many hundred thousandths to the mantions before found, and to the sum profix the proper characteristic.

9. Examples.

1,	What	ie	the	logarithm	of	2347 7	Ans. 3 37051.
2.	What	is	the	logarithm	of	1084579	Ana 5 (0520)
13	What	is	the	logarithm	of	3765429	Ana 5 57581.
-4.	What	is	the	logarithm	of	229,7052 1	Ans. 2.36117
-5,	What	is	the	logarithm	of	1128737 ?	Apr. 605260
-6.	What	is	the	logarithm	of	.30365 ?	Jus 1 18237
H fie	What	is	the	logarithm	of	,0042683 ?	7.00 S 63025
- 8.	What	116	the	logarithm	of	12454002	16.000 6.000331

n by annexing c.

10. Problem.

To find the number corresponding to a given logarithm.

1 What number corresponds to logarithm 2 03262?

The mantissa is found in the column headed 8, and opposite 107 in the column headed N. Hence, without reference to the decimal point, the number corresponding is 1078; but since the characteristic is 2, the number is entirely decimal, and one 0 immediately follows the decimal point. Hence, the number corresponding is .01078.

2. What number corresponds to logarithm 2.83037?

Since this logarithm can not be found in the table, take the next less, which is 2.83033, and the corresponding number, without reference to the decimal point, which is 6766.

The difference between the given logarithm and the next less is 4, and the tabular difference is 6, which is the difference of the logarithms of the two numbers, 67.55 and 67.67, whose difference is 1.

If the tabular difference of the logarithms, 6, corresponds to a difference in the numbers of 1, the difference of the logarithms, 4, will correspond to a difference of the logarithms, reduced to a decimal, and annexed to 6766, will give for the number, without reference to the decimal point, 676666. But since the characteristic is 2, there will be three integral places: hence, 676,666 is the number required.

3. What number corresponds to logarithm 2.76395?

The given log 2.76395 ... number 580.737 Next less log 2.76395 ... number 580.737

Tab difference 8,200 difference

37 = correction.

It is necessary to write only that part of the next less logarithm which differs from the given logarithm Conceive 0s annexed to the difference, and divide by the tabular difference, and annex the quotient to the number corresponding to the next less logarithm

In practical work abbreviate thus, fat I denote the given logarithm; I', the next less logarithm, n and n', the corresponding numbers; I, the tabular difference; d, difference of logarithms; *, the correction

4. What number corresponds to logarithm 173045?

l = 1.73048 .*. n = .537625 l' = 1.73046 .*. n' = .5376t = .8)2 = d, n' is found first, then

11. Rule.

1. If the given mantion can be found in the table, take the number corresponding, and place the decimal point membering to the law for the characteristic.

2 If the given mantissa can not be found in the table, take the next less and the corresponding number. Subtract this mantissa from the meet notation, amore Os to the remainder, divide the acid by the tabular difference, amore the quotient to the number corresponding to the lowerth is next low than the given logarithm, and place the devical point according to the law for the characteristic.

12. Examples.

- 1. What number corresponds to logarithm 4 55703.2
- 2. What number corresponds to logarithm 3.95147?

 Aus. 8942 ×
- 3. What number corresponds to logarithm 2 11430?

 _doc.025781

4. What number corresponds to logarithm 1 18237?

Ans. .30365.

5. What number corresponds to logarithm 3.65025?

Ans. .0042683.

MULTIPLICATION BY LOGARITHMS.

13. Proposition.

The logarithm of the product of two numbers is equal to the sum of their logarithms.

Let $\begin{cases} (1) \ b^* = m; \text{ then, by def, log } m = x. \\ (2) \ b^* = n; \text{ then, by def, log } n = y. \end{cases}$

(1) < (2) = (3) b^{r+s} mn; then, by def., $\log m n = x \cdot y$. \cdot · · log m n · log m + log n.

14. Rule.

1. Find the logarithms of the factors and take their num,

2 Find the number corresponding which will be their partnet.

15. Examples.

1 Find the product of 57846 and .003927.

 $\log 57846 = 4.76228$

log 003927 = 3 59406

log product = 2.35634, ... product == 227.16.

2. Find the product of 37.58 and 75864.

Am. 2851000.

3. Find the product of .3754 and .00756.

Ann. ,002838.

4. Find the product of 999.75 and 75.85.

Ann. 75831,667.

5. Find the product of 85, .097, and .125. .4mm. 1.03062.

DIVISION BY LOGARITHMS.

16. Proposition.

The logarithm of the quotient of two numbers is equal to the logarithm of the dividend minus the logarithm of the divisor.

Let $\begin{cases} (1) & b^* = m; \text{ then, by def., } \log m = x. \\ (2) & b^* = n; \text{ then, by def., } \log n = y. \end{cases}$

(1) 2 (3) $b^{my} = \frac{m}{n}$; then, by def, $\log \frac{m}{n} = x - y$. $\log \frac{m}{n} = \log m - \log n$

17. Rule.

1. Find the logarithms of the numbers, subtract the logarithm of the divisor from the logarithm of the dividend, and the remainder will be the logarithm of the quotient.

2. Find the number corresponding which will be the quotient,

18. Examples.

1. Divide 73 125 by .125.

1 2 73 125 1 86407

log .125 1.096901

log quotient 2.76716, ... quotient 585

2 Divide 7.5 by 000025

Airs. 300000.

3. Divide 87.9 by 0345

Aug. 2547-824.

4. Divide 31852 by ,00789,

Aus 44 171.

5. Divide 85734 by 12.7523.

Aux 672%

ARITHMETICAL COMPLEMENT.

19. Definition.

The arithmetical complement of a logarithm is the result obtained by subtracting that logarithm from 10. Thus, denoting the logarithm by l_{α} and its arithmetical complement by a c, l_{α} we shall have the formula,

a. c.
$$l = 10 - L$$

The arithmetical complement of a logarithm is most radily found by commencing at the left of the logarithm, and subtracting each digit from 9 till we come to the last numeral digit, which must be subtracted from 10.

Thus, to find the a. c of 3 47540, we say: 3 from 9, 6, 4 from 9, 5; 7 from 9, 2; 5 from 9, 4, 4 from 10, 6; 0 from 0, 0.

. a. c. of 3.47540 6.52460.

20. Proposition.

The difference of two logarithms is equal to the surmend, plus the arithmetical complement of the subtrahend, nor - 10.

For,
$$l-l'=l+(10-l')-10$$
.

It is convenient to use the a, c, in division when either the dividend or the divisor is the indicated product of two or more factors. Thus, let it be required to find z in the proportion:

37.5 : 67.8.5 :: 27.56 :
$$x$$
; .*, $x = \frac{678.5 \times 27.56}{37.5}$.

.*. $\log x = \log 678.5 + \log 27.56 + a$, c. $\log 37.5 - 10$.

 $\log 678.5 - 2.83155$
 $\log 27.56 := 1.44028$
 a , c. $\log 37.5 - 8.42597$
 $\log x = 2.69780$
•, $x = 498.656$.

21. Examples.

1. Given 125.5 : .0756 :: x : .0034532, to find x Ans. 5.7325

2. Given 843 : x :: 732 534 : .759, to find x.

Ann. .87346

3. Given z : .034 :: .784 : .00489, to find z.

Ans. 5 451125.

4. Given $x = \frac{32.015 \times .874}{000216 \times 90257}$, to find x. Ans. 1 4353

5 Given .753 · 12 231 · 87.5 · 3 7547 ... 56.5 · x, to find x Ans. 2014 96.

INVOLUTION BY LOGARITHMS.

22. Proposition.

The logarithm of any power of a number is equal to the logarithm of the number multiplied by the exponent of the power

Let (1) b^r ., then, by def, $\log n$ r. (1)r=(2) b^{pr} n^r , then, by def, $\log n^r$ px, $\log n^r$ $p\log n$

23. Rule.

- the epenent of the pare, and the public will be the legaration of the pane.
 - 2. Find the neither corresponding which well be the powers

24. Examples,

1 Find the cube of 034

(1) $\log .034 = 2.53148$ (1) $\times 3 = (2) \log .034^3 = 5.59444$ (3) 000039303 2. Find the equare of 25.7. And 6 at 47

PLANE TRIGONOMETRY.

23

3. Find the fourth power of .75.

Ame. 3164.

4. Find the cube of SOT.

Aus. 525.55.

5. Find the lifth power of 3

Ans. ,59047,

EVOLUTION BY LOGARITHMS.

25. Proposition.

The logarith a of any root of a number is equal to the inner of the number divided by the index of the root.

Let (1) by n; then, by def., log n z.

 $i_{1}(1) + 2$ $b_{r} = i\sqrt{n}i_{r}$ then, by def., $\log i_{r} n = \frac{x}{r} + \log i_{r} = \frac{\log n}{r}$.

26. Rule.

1 Find the logarithm of the number, divide it by the roles of the root, and the quotient will be the logarithm of the root.

2. Find the number corresponding which will be the root.

27. Examples.

1 Extract the square root of .75.

(1) log .75 - 187506

(1 2 2) log 1 75 1.93753 . 1 .75 8cop2.

N 5-Jones 1 87506 2 (2 + 1.87506) 2 1 9 1758

3 Find the value of \$ 1.5 Aux \$9143.

5 Find the value of $\sqrt[4]{\frac{37.5 \times (.78)^2}{12.5 \times 5.9}}$.1n. .676317.

TRIGONOMETRY.

28. Definition and Classification.

Trigonometry is that branch of Mathematics which treats of the solution of triangles.

Trigonometry is divided into two branches - Plane and Spherical.

PLANE TRIGONOMETRY.

29. Definition.

Plane Trigonometry is that branch of Trigonometry which tracts of the solution of plane triangles

30. Parts of a Triangle.

Every triangle has six parts—three sides and three angles

If three parts are given, one being a side, the re-

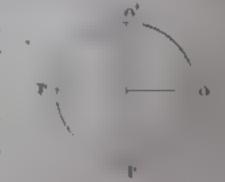
If the three angles only are given, the triangle is indeterminate, since an infinite number of similar triangles will satisfy the conditions.

31. Sexagesimal Division of Angles and Arcs.

The horizontal diameter, O P, called the privary disameter, and the vertical diameter,

O' P', called the secondary diameter, divide the circumference into four equal parts, called quadrants.

O O' is the first quadrant, O' P the second, P P' the third, and P' O the fourth



PLANE TRIGONOMETRY:

25

A degree is one-ninetieth of a right angle, or of a

A minute is one-sixtieth of a degree.

A second is one-sixtieth of a minute.

Thus, 25° 34′ 46″ denote 25 degrees, 34 minutes, and

An angle, whose vertex is at the center, has the same number of degrees, minutes, and seconds, as the arc of the cummicrence intercepted by its sides.

32. Centesimal Division of Angles and Arcs.

A grade is one-hundreth of a right-angle, or of a questiant.

A minute is one-hundreth of a grade.

A second is one-hundreth of a minute.

Thus, 7* 24° 40" denotes 7 grades, 24 minutes, and to seconds.

$$1^{\circ} = \frac{10^{\circ}}{9}, \quad 1' = \frac{50^{\circ}}{27}, \quad 1'' = \frac{250^{\circ}}{81}.$$

$$1' = \frac{9^{\circ}}{10} \cdot 1' - \frac{27'}{50} \cdot 1'' - \frac{81''}{250}$$

Let d, m, s, respectively, denote an angle expressed in degrees, sexagesimal minutes and seconds, and let a, s, respectively, denote the same angle expressed in grades, centesimal minutes and seconds, then expressing the ratio of the angle to a right angle in a h kind of units, we shall have:

$$\frac{d}{90} = \frac{g}{100}, \quad \frac{m}{5400} = \frac{\mu}{10000}, \quad \frac{s}{324000} = \frac{\sigma}{1000000},$$

$$\therefore \quad d = \frac{9}{10}g, \quad m = \frac{27}{50}\mu, \quad s = \frac{81}{250}s,$$

$$\therefore \quad g = \frac{10}{9}d, \quad \mu = \frac{50}{27}m, \quad \sigma = \frac{250}{81}s.$$

Let r denote the radius, and $\pi=3.14159265358979...$

 $\pi r = a$ semi-circumference = 150° = 200° = two right angles.

$$\frac{\pi}{2}r = a$$
 quadrant = 90° 100° one right angle,

 $2\pi r = a$ circumference 360° 400° four right angles.

If r=1, the above expressions become, respectively, π , $\frac{\pi}{2}$, 2π .

33. Unit of Circular Measure.

The unit of circular measure is that angle at the center whose intercepted are is equal in length to the radius.

Let a denote the unit of circular measure, and a the

Then, since $\pi r =$ the semi-circumference, $\pi u = 180^{\circ}$ 200%.

Let d g, c, respectively, denote the number of degrees, grades, and units of circular measure in an angle, then,

$$\frac{d}{\tau} = \frac{180}{\tau} c_1 \quad q = \frac{200}{\tau} c_1 \quad c = \frac{\pi}{180} d_1 \quad c = \frac{\pi}{200} g_1$$

34. Origin, Termini and Situation of Arcs.

The origin of an arc is the extremity at which it begins.

The primary origin of area is at the right extremity . of the primary diameter.

The secondary origin of area is at the upper extremity of the vertical diameter.

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The terminus of an are is the extremity at which it ends.

An are is said to be situated in that quadrant in which its terminus is situated, thus:

The are OT is in the first qual

The are $\partial \partial' T'$ is in the second quadrant.

The arc OPT" is in the third quadrant.

The arc OPT" is in the fourth quadrant.

35. Positive and Negative Arcs.

Positive arcs are those which are estimated in the direction contrary to that of the motion of the hands of a watch.

Negative arcs are those which are estimated in the same direction as that of the motion of the hands of a watch.

Thus, OT, OT', OT", OT", estimated to the left are positive, and OT", OT", OT', OT, estimated to the right, are negative.

36. The Complement of an Arc.

The complement of an arc or angle is 90° minus that are or angle.

If the are or angle is less than 90°, its comple-

If the are or angle is greater than 90°, its comple-

The complement of an are, geometrically considered is the arc estimated from the terminus of the given are to the secondary origin. Therefore, by the preceding article, the complement of an arc will be positive

or negative, according as the are is less or greater than 90°.

TO' is the complement of OT, and is positive. T'O' is the complement of OT', and is negative. T''O' is the complement of OT', and is negative. T''O' is the complement of OT'', and is negative.

37. The Supplement of an Arc.

The supplement of an arc or angle is 180° minus that are or angle.

If the arc or angle is less than 180°, its supplement is positive.

if the arc or angle is greater than 1900, its supplement is negative.

The supplement of an arc, geometrically considered, is the arc estimated from the terminus of the given arc to the left-hand extremity of the primary diameter. Therefore, by article 35, the supplement of an arc will be positive or negative, according as the arc is less or greater than 180°.

TP is the supplement of OT, and is positive. T'P is the supplement of OT', and is positive. T''P is the supplement of OT'', and is negative. T'''P is the supplement of OT''', and is negative.

TRIGONOMETRICAL FUNCTIONS.

38. Preliminary Definitions and Remarks.

- 1. A function of a quantity is a quantity whose value depends on the given quantity.
- 2 The trigonometrical functions, called also ciscular functions, are auxiliary lines, which are functions of an are or of the angle which has the same measure as that are.

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3. These functions are eight in number, and are eight the common content of the common content of the common and common, which are abbreviated this are on the common time, of, we come.

I have slation of triangles is accomplished by the localities functions, since they enable us to ascertain to blat, as which exist between the sides and angles of triangles.

5 The primary origin will be taken as the common origin of the arcs, unless the contrary is stated.

6. The origin of any are, wherever situated, may be considered the primary origin of that are; and its secondary origin is a quadrant's distance from the primary or gin, in the direction of the positive or negative ares, as ording as the given are is positive or negative.

7 An are will be considered positive unless the contrity is stated.

S The primary diameter passes through the primary erigin; and the secondary diameter, through the secondary origin.

9 lanes estimated upward, toward the right, or from the center toward the terminus of the are, are considered postere.

10 Lines estimated downward, toward the left, or from the center and the terminus of the are, are considered mention.

11. The limiting values of the circular functions are thour values for the arcs 0°, 90°, 180°, 270°, 360°.

12. The sign of a varying quantity, up to a limit

13. Point out positive arcs in the following diagram, and the origin and terminus of each.

14 Point out negative ares, the origin, terminus and primary diameter of each.

15. Point out the positive lines, also the negative.

39. The Sine of an Arc.

The sine of an arc is the perpendicular distance of its terminus from the primary diameter.

MT is the sine of the arc OT.

M'T' is the sine of the arc OT'.

M'T" is the sine of the arc OT".

MT" is the sine of the arc OT".

By the arcs OT" and OT", we are to understand the positive arcs, and not the negative arcs designated by

the same letters.



The sine of an arc is the sine of the angle measured by that are

Thus, MT, the sine of the arc OT, is the sine of the angle OCT, which is measured by the arc OT; and similarly for the other arcs and angles.

The arcs OT and OT' are in the first and second quadrants, respectively, and their sines MT and MT' are estimated opened, and are therefore postere, hence,

The sine of an arc in the first or second quadrant is

The arcs OT'' and OT''' are in the third and fourth quadrants, respectively, and their sines, M'T'' and MT''', are estimated dominant, and are therefore negative; hence,

The sine of an arc in the third or fourth quadrant is negative.

Let the chord TT' be parallel to the primary diameter OP, then will M'T' be equal to MT, and the are OT will be equal to the are $T'P_i$ but the are T'P is the supplement of the are OT'; therefore, the are OT is the supplement of the are OT'; but M'T',

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the sine of the arc OT, is equal to MT, the sine of the are UT, the supplement of OT'; hence,

Then e f an are is opini to the sine of its supplement.

The sine of 0° is 0. As the are increases from 0° to 90°, the sine increases from 0 to +1. As the an in reases from 90° to 180°, the sine decreases from +1 to 0 As the are increases from 150° to 270°, the sale passes through 0, changes its sign from + to -, and mercases numerically, but decreases algebraically from 0 to 1. As the arc increases from 270° to Bear', the sine decreases numerically, but increases algenerally from 1 to -0.

Henre, for the limiting values of the sine, we have $\sin 0^{\circ} = 0$, $\sin 90^{\circ} = +1$, $\sin 180^{\circ} = +0$, $\sin 270^{\circ} = -1$, $\sin 360^{\circ} = -0$.

40. The Co-sine of an Arc.

The co-sine of an are is the perpendicular distance of its terminus from the secondary diameter.

NT is the co-sine of the ar · OT. NT' is the co-sine of the arc OT'. N'T'' is the co-sine of the arc OT''. N'T''' is the co-sine of the arc OT''. The ares OT and OT" are in the first and fourth quadrants, respective ly, and their co-sines NT and N'T"

are estimated toward the right, and are therefore per tire, honce,

The orner of an are in the first or fourth quadrant "

The ans OT' and OT" are in the second and third quadrants, respectively, and their co-sines, NT' and N'T", are estimated toward the left, and are therefore negative; hence,

The co-sine of an arc in the second or third quadrant is negative.

The word cosine is an abbreviation of complementi sinus, the sine of the complement. In fact, NT, the co-sine of OT, is the sine of OT, the complement of OT; hence,

The co-sine of an arc is the sine of its complement.

MT, the sine of OT, is the co-sine of OT, the complement of OT; hence,

The sine of an are is the co-sine of its complement.

Since the radius CO is perpendicular to the chord TT', NT and NT' are numerically equal; but since NT is estimated toward the right, and NT' toward the left, they have contrary signs; hence, NT = NT'; but NT is the co-sine of OT, and NT' is the co-sine of OT', the supplement of OT_i hence,

The co-sine of an are is equal to minus the co-sine of its supplement.

It is evident that CN is equal to the sine of OT, or of OT', and that CX' is equal to the sine of OT'', or of OT"; hence,

The sine of an are is equal to that part of the secondary diameter from the center to the foot of the co-eine.

It is evident that CM is equal to the co-sine of OT, or of OT", and that CM' is equal to the co-sine of OT' or of OT"; hence,

The co-sine of an arc is equal to that part of the primary diameter from the center to the foot of the sine,

The cosine of 0° is + 1. As the are increases from 0° to 90° , the co-sine decreases from +1 to +0. As the are increases from 90° to 180°, the co-sine passes through 0, changes its sign from + to -, and increases namerically, but decreases algebraically from 0 to

1. As the are increases from 180° to 270°, the cosine decreases numerically, but increases algebraically

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from 1 to -0 As the are increases from 270° to 500°, the cosmic passes through 0, changes its sign from - to 4, and in reases from +0 to +1.

Hence, for the limiting values of the co-sine, we have $\cos 0^{\circ} = 1$, $\cos 90^{\circ} = +0$, $\cos 180^{\circ} = -1$, $\cos 270^{\circ} = 0$, $\cos 360^{\circ} = +1$.

41. The Versed-Sine of an Arc.

The versed-sine of an are is the perpendicular dis-

MO is the versed-sine of the arc OT, and of the arc OT'''.

M'D is the versed-sine of the arc OT', and of the arc OT''.

The versed-sine of an are, in any product, is estimated to the right, and is therefore product; hence,

The record-some is always positive

The versed sing of 0° is 0. As the arc increases from 0° to 90°, the versed-sine increases from 0 to 4. As the arc increases from 90° to 180°, the versed-sine increases from ±1 to ±2. As the arc increases from 180° to 270°, the versed-sine decreases from ±2 to ±1. As the arc increases from ±2 to ±1. As the arc increases from 270° to 300°, the versed-sine decreases from ±1 to ±0.

Hence, the limiting values of the versed-sine are vers 0° 0, vers $90^{\circ} = +1$, vers $180^{\circ} = +2$. vers 270° -1, vers $360^{\circ} = +0$.

What are the least and greatest values of the sine, and what are the corresponding ares?

What are the least and greatest values of the co-sine, and what are the corresponding ares?

What are the least and greatest values of the versedsine, and what are the corresponding arcs?

42. The Co-versed-sine of an Arc.

The co-versed-sine of an arc is the perpendicular distance of the secondary origin from the co-sine.

Thus, see diagram of the last article, NO' is the coversed-sine of the arc OT, and of the arc OT'; N'U' is the co-versed-sine of the arc OT'', and of the arc OT''',

The co-versed-sine of an arc in any quadrant is estimated upward, and is therefore positive; hence,

The co-versed-sine is always positive.

The word co-rerect-sine is an abbreviation of complementi versatus sinus, the versed or turned sine of the complement. In fact, NO', the co-versed-sine of OT, is the versed-sine of OT, the complement of OT; hence,

The co-versal-sine of an arc is the versal-sine of its complement,

. MO, the versed-sine of OT, is the co-versed sine of OT, the complement of OT; hence,

The versal-sine of an arc is the co-versal-sine of its complement.

The co-versed-sine of 0° is 1. As the are increases from 0° to 90°, the co-versed sine decreases from ± 1 to 0. As the arc increases from 90° to 180°, the co-versed sine mercuses from ± 0 to ± 1 . As the arc increases from 180° to 270°, the co-versed-sine increases from ± 1 to ± 2 . As the arc increases from ± 1 to ± 2 . As the arc increases from ± 2 0° to 360°, the co-versed-sine decreases from ± 2 to ± 1 . Hence, the limiting values of the co-versed-sine arc, covers $0° = \pm 1$, covers $90° = \pm 0$, covers $180° = \pm 1$, covers $270° = \pm 2$, covers $360° = \pm 1$.

What are the least and greatest values of the coversed sine, and what are the corresponding ares?

Trace the area from 0° to 360°, and the changing functions.

43. The Tangent of an Arc.

The tangent of an are is the perpendicular to the primary dismeter, produced from the primary origin, the heats the prelongation of the diameter through the terminus of the are

OR is the tangent of the ares OT and OT''

and OT''OR is the tangent of the arcs OT' reached OT'''.

The ares OT and OT" are in the best and third quadrants, respectively,

and their tangent, OR, is estimated appeared, and is tarred to postery hence,

The tangent of an are in the first or third quadrant is positive.

The ares OT' and OT'' are in the second and fourth quadrants, respectively, and their tangent, OR', is estimated decount, and is therefore negative; hence,

The tangent of an are in the second or fourth qual and is

Let the arc OT be equal to the arc TP. Then, c.e. TP is the supplement of OT', OT will be the supplement of OT'; but the arc T"O is the supplement of OT'; but the arc T"O, and the angle OCT is equal to the angle OCT". The angle COR is equal to the angle COR', since each is a right angle. Hence, the two triangles COR and COR' have two angles, and the included side of the one equal to two angles, and the included side of the other, each to each, and are therefore equal in all their parts. Hence, OR, opposite the angle OCR, is equal to OR', opposite the equal angle OCR'. Since OR is estimated upward, and OR' downward, they have contrary since the equal angle OCR'. But OR is the tangent

of the arc OT, and OR' is the tangent of the arc OT', the supplement of OT_T hence,

The tangent of an are is equal to minus the tangent of ats supplement,

The tangent of 0° is 0. As the arc increases from 0° to 90° , the tangent increases from 0 to $+\infty$. As the arc increases from 90° to 180° , the tangent passes through ∞ , changes its sign from + to -, and decreases numerically, but increases algebraically from $-\infty$ to -0. As the arc increases from 180° to 270° , the tangent passes through 0, changes its sign from to -, and increases from +0 to $+\infty$. As the arc increases from 270° to 360° , the tangent passes through ∞ , changes its sign from + to -, and decreases numerically, but increases algebraically from $-\infty$ to -0. Hence, for the limiting values of the tangent we have $\tan 0^{\circ} - 0$, $\tan 90^{\circ} - \infty$, $\tan 180^{\circ} - 0$, $\tan 270^{\circ} - \infty$, $\tan 360^{\circ} - 0$.

44. The Co-tangent of an Arc.

The co-tangent of an are is the perpendicular to the secondary diameter, produced from the secondary origin, tid it rights the prolongation of the diameter through the terminus of the are

OS is the costangent of OT and OT".

OS' is the co-tingent of OT' and OT".

The arcs OT and OT'' are in the first and third quadrants, respectively, and their costangent, OS, is estimated to the right, and is therefore positive; hence,

The costingent of an arc in the first or third quadrant is positive

The ares OT' and OT''' are in the second and fourth quadrants respectively, and their co-tangent, OS', is estimated to the htt, and is therefore hotelor, hence,

The retainment of an are in the second or fourth quadrant

The word on the 2 is an abbreviation of complement:

In fact, ∂S_i I word on the complement. In fact, ∂S_i I word of ∂T_i is the tangent of ∂T_i the com-

The cost is x of of an are is the tangent of its complement.

OR, the tangent of OT, is the costangent of OT, the x is pleasant of OT; hence,

Let the arcs OT and T'P be equal. Then, since TP is the supplement of OT', OT will be the supplement of OT'.

The arcs OT and OT are equal, since they are on planents of the equal arcs OT and TT; hence, the angles OCT and OCT, measured by these equal arcs, are equal. The angles COS and COS are equal, so each is a right angle. Hence, the two triangles COS and COS have the common side CO, and the two an, ceut angles equal, and are therefore equal in all their parts; and OS, opposite the angle OCS, is equal to OS, opposite the equal angle OCS.

Since OS is estimated to the right, and OS' to the Id', they have contrary signs; hence, OS = OS'. But OS is the co-tangent of OT, and OS' is the cotangent of OT, the supplement of OT; hence,

Tern tangent of an are is equal to minus the co-tangent of

The cotangent of 0° is $+\infty$. As the arc increases from 0° to 90°, the co-tangent decreases from 4 or to +0. As the arc increases from 90° to 180°, the cotangent passes through 0, changes its sign from + to --. and increases numerically, but decreases algebratedly from -0 to --∞. As the arc increases from 180° to 270°, the co-tangent passes through ∞, changes

its sign from — to \pm , and decreases from \pm to \pm 0. As the arc increases from 270° to 360°, the co-tangent passes through 0, changes its sign from \pm to —, and mereases numerically, but decreases algebraically from 0 to — x.

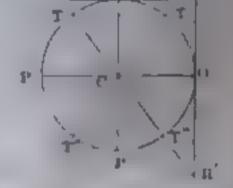
Hence, the limiting values of the co-tangent are cut $0^{\circ} + \infty$, cut $90^{\circ} + 0$, cut $180^{\circ} - \infty$, cut $270^{\circ} + 0$, cut $360^{\circ} - \infty$.

45. The Secant of an Arc.

The secant of an arc is the line drawn from the center of the circle to the terminus of the tangent.

CR is the secant of OT and OT''. CR' is the secant of OT' and OT'''.

The arcs OT and OT" are in the first and fourth quadrants, respectively, and their sceants, CR and



CR' are estimated from the center toward the termini of the ares, and are therefore positive; hence,

The secant of an arc in the first or fourth quadrant is

The arcs OT' and OT" are in the second and third quadrants, respectively, and their secants, CR' and CR, are estimated from the center, from the termini of the arcs and are therefore negative; hence,

The second of an are in the second or third quadrant is negative.

Let the area OT and T'P be equal. Then, since T'P is the supplement of OT', OT is the supplement of OT'; but T'''O is the supplement of OT'; therefore, T'''O is equal to OT, and the angle T'''CO, measured by T'''O, is equal to the angle OCT, measured by the equal are OT. The right angles COR and COR' are

equal Report in the triangles having the common size CO, and the two adjacent angles equal, CR is equal to CR, but CR, the secant of OT, is positive; and CR, the secant of OT, the supplement of OT, is a partie, hence, CR = CR; hence,

I'm would of an are is equal to names the second of its

The scant of 0° is + 1. As the are increases from to to wor, the secant increases from + I to + z. As the are marriages from 90' to 180°, the secant passes through x, changes its sign from + to , and de-*reas * num, really, but mereases algebraically from the secant increases numerically, but decreases algetra, ally from I to - x. As the are increases from 27 ° to 360°, the scennt passes through or, changes its · za from - to -, and decreases from + 55 to + 1. Hone, for the limiting values of the secant we have 401 11, sec 90° , 50, sec $180^{\circ} = -1$, sec 270 - - x, sec 360° + 1.

46. The Co-secant of an Arc.

The co-secant of an are is the line drawn from the center of the circle to the terminus of the co-tangent

CS is the co-secant of OT and OT".

The arcs ∂T and $\partial T'$ are in the

is ly, and their co-scenits (S and CS')

the and and are therefore positive; hence,

The consecunt of an arc in the first or second quadrant

The arcs OT'' and OT''' are in the third and fourth quadrants, respectively, and their co-securits, CS and CS', are estimated from the center and the termini of the arcs, and are therefore negative; hence,

The consecunt of an arc in the third or fourth quadrant is magative.

The word co-secunt is an abbreviation of complementisecure, the secunt of the complement. In fact, CS, the co-secunt of OT_i is the secunt of OT_i , the complement of OT_i hence,

The constraint of an arc is the secant of its complement t|R, the secant of OT, is the consecunt of OT, the complement of OT; hence,

The second of an are is the co-second of its complement.

Let the ar s OT and T'P be equal. Then, since TP is the supplement of OT'. OT = O'T', since they are complements of equal ares. Hence, the angle OCT, measured by the are $O'T_i$ is equal to the angle OCT', measured by the equal are O'T'. The right angles, COS and COS, are equal.

Hence, in the triangles having the common side CO, and the two adjacent angles equal, CS is equal to CS, but CS is the consecunt of OT, and positive, and CS' is the consecunt of OT, and positive, hence,

The ersect of its no are is equal to the consecunt of its

The oscillation of 0° is $+\infty$. As the are increases from to 0° 0°, the co-secant decreases from $+\infty$ to +1. As the are increases from 1° 0° to 1° 0°, the co-secant increases from 1° 1 to 270° 1, the co-secant passes through z1, changes its sign from +10°, and decreases numerically, but increases algebraically from $+\infty$ 10°, the co-secant increases from 270° 10°, the co-secant increases

numerically, but decreases algebraically from -1 to -x. Hence, the limiting values of the co-secant are $\cos x (0^{2} + x)$, $\cos x (0) + x$, $\csc y (0) + 1$, $\csc y (10) + x$, $\csc y (10) + 1$, $\cot y (10) + 1$,

To aid the memory, and for convenience of reference, we give the f llowing tabular summaries:

47. Signs of the Circular Functions.

Functions.	1st q.	2d q.	3d q.	4th q
*1110	1 +	+		
C1643336	+	strated:		
versid sine.	+	+	+	
covered sine.	+ !	+	+	+
tangent.	+		+	_
co-tangent.	+		+	-
secant.	1 + 1	****	_	
cossenant.	+	4		-

48. Limiting Values of the Circular Functions.

At = +1 400 - + 2 400 - 1	0.		150°		3602
then $+\infty$ case $= -1$ case $-+\infty$ case -1 case -1	$voin = 0$ $cvs = \pm 1$ $ton = \pm 0$ $cot = \pm x$ $sec = \pm 1$	v-in = 1 $cvs = 0$ $tan = x$ $cot = 0$ $sec = +x$	$cos = -1$ $vsin = +2$ $cvs = +1$ $tan = -0$ $cot = -\infty$	$cos = -0$ $vsin = +1$ $cvs = +2$ $tan = +\infty$ $cot = +0$	cos - 1 vsin 0 cvs - 1 tan == 0 cot - 2

49. Problem.

To find any function of an angle to the radius R, in terms of the corresponding function of the same angle to the radius 1, and the reverse.

Let $\sin C_1$ denote $\sin C$ to the radius CT = 1, and $\sin C_2$ denote $\sin C$ to the radius CT' = R.



From similar triangles,

8, N. 4

or 1 :
$$CT'$$
 :: MT : $M'T'$,

... (1)
$$\sin C_n = \sin C_1 \times R$$
. ... (2) $\sin C_1 = \frac{\sin C_n}{R}$.

Let formulas for other functions be deduced; hence,

- 1. Any function of an angle to the radius R is equal to the corresponding function of the same angle to the radius 1, multiplied by R.
- 2. Any function of an angle to the radius 1 is equal to the corresponding function of the same angle to the radius R_i divided by R_i .

TABLE OF NATURAL FUNCTIONS.

50. Description of the Table.

This table gives, to the radius I, the values of the sine, co-sine, tangent, and co-tangent, to five decimal places, for every 10' from 0° to 90°.

For sines and tangents, the degrees are given in the left column, and the minutes at the top.

For co-sines and co-tangents, the degrees are given in the right-hand column, and the minutes at the bettern

51. Problem.

In had the matural and, comme, tangent, or co-tangent

Let us find the natural sine of 35° 42' 24".

The difference between the natural sines of 35° 40° and 35° 50°, as given in the table, is .00236. Now 2' 24° 24 of 10°, which is found thus: 60' 24

10; 2.4 .24

Then take Nat sin 35° 40′=-.58307 Correction for 2′ 24″ - .00236 - .24 - ..00057 ... Nat sin 35° 42′ 24″ - .58364

In case of co-sine or co-tangent, the correction must be subtracted, since, between 0° and 90°, the greater the angle, the less the co-sine and co-tangent.

52. Examples.

1 Find the natural sine of 75° 45' 30".

Ann. .96927.

2. Find the natural co-sine of 15° 36' 12".

Just 96315

3 Find the natural tangent of 43° 33' 15".

Ans. .95079.

4. Find the natural co-tangent of 84° 28' 30".

Ans. .08673.

53. Problem.

To find the angle corresponding to a given natural sine, connuc, tangent, or co-tangent,

I Find the angle corresponding to the natural sine

Looking in the table we find the angle 30° 30'.

2. Find the angle whose natural sine \sim .82468. The next less sine, sin 55° 30′ \sim .82413.

Difference 55

Difference corresponding to 10' = 164

, . Correction $10' \cdot \frac{55}{164} = 3' \cdot 21''$.

... Angle 55° 30′ + 3′ 21″ 55° 33′ 21″.

In case of co-sine and co-tangent, the angular differonce must be subtracted, since the greater the co-sine or co-tangent, the less the angle, for values between 0° and 90°.

54. Examples.

1. Find the angle whose sine is .75684.

Ans 49° 11' 13".

2 Find the angle whose co-sine is .67898.

Ann. 47° 14' 10".

3. Find the angle whose tangent is 1.34567.

Ana. 53° 22' 59".

4. Find the angle whose co-tangent is .98765.

Ans. 45° 21' 22".

TABLE OF LOGARITHMIC FUNCTIONS.

55. Description of the Table.

The table of logarithmic functions gives to the radius [0,000,000,000 the logarithm of the sine, co-sine, tangent, and co-tangent, for every minute, from 0° to 90°,

The expression, logarithmic sine, tangent, etc., is equivalent to the logarithm of the sine, of the tangent, etc.

For sines and tangents, the degrees are given at the top of the page, and the minutes in the left-hand column.

LOGARITHMIC FUNCTIONS.

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For co-sines and co-tangents, the degrees are given at the bettom of the page, and the minutes in the right. In I od man

The columns marked D I" contain the difference for 1"

56. Problem.

Find the logarithmic sine of 48° 25' 30".

log sin 48° 25′. 9 87390.

D 1″ .19. . Correct for 30″ .19 - 30 6

. log sin 48° 25′ 30″ 9 87396

In case of co-sine or co-tangent, the correction must be subtracted, since between 0° and 90°, the greater the angle, the less the co-sine and co-tangent.

57. Examples.

1. Find the logarithmic sine of 75° 35'.

.1nx 9.98610.

2. Find the logarithmic sine of 25° 40' 24".

Ans. 9,63673.

3. Find the logarithmic co-sine of 29° 55′ 55″

Ans. 9,93782

4. Find the logarithmic tangent of 50° 50′ 50″.

Ana, 10.08927.

5. Find the logarithmic co-tangent of 65° 45' 30".

Ans. 9,65349.

58. Problem.

To first the angle corresponding to a given logarithmic sine, resine, tangent, or co-tangent.

Find the angle whose logarithmic sine 9 84567 For next less we have sin 44° 30′ 9 84566

D 1" = .21 ... Correc. $-1" \times \frac{1}{.21} = 5"$, .21)1 00(5. ... Angle -44° 30' 05".

In ease of co-sine and co-tangent, the correction for seconds must be subtracted, since the greater the co-sine or co-tangent, and consequently the greater the logarithm, the less the angle for values between 0° and 90°.

59. Examples.

- Find the angle whose logarithmic sine is 9.98437.
 Ans. 71° 43′ 17″.
- 2 Find the angle whose logarithmic co-sine is 9.78456. Ann. 52° 29′ 19″.
- 3. Find the angle whose logarith, tangent is 10 12316.

 Ana, 53° 02' 11".
- 4. Find the angle whose logarith, co-tangent is 9 99999.

 Ans. 45° 00' 03".

60. Problem.

logarithmic function.

1st Solution.

Find from the natural function the corresponding angle, then, from the angle, the corresponding logarithmic function.

2d Solution.

Let a denote any are or angle, $f(a)_1$ any function of a to the radius 1, and $f(a)_n$ the corresponding

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fraction of a to the radius R. Then, by article 49 me have,

 $|f_{-1}\rangle_{\mathbf{z}} = |f_{N}a\rangle_{\mathbf{y}} \otimes |R_{i}|$

Silentitating the value of R in the second meriber, $f(a)_{R} = f(a)_{1} + 10,000,000,000$,

... $\log f(a) = \log f(a)_1 + 10$.

Hence, Ald 10 to the logarithm of the natural function.

61. Examples.

t Given nat. sin $a \sim .98457$, required a and $\log s$, s, n a = .999325

2. Green nat. cos a = .63878, required a and log cos a = .20° 17′ 52″, log cos a = 9.80536.

3. Given nat. tan a 1.68685, required a and log tan a 450 a 59° 20° 23″, log tan a 10° 22708

4 Given nat. cot a = 1.41987, required a and $\log a = 35^{\circ} 09' 24''$, $\log \cot a = 10.15225$

62. Problem.

Given any logarithmic function, to find the corresponding natural function.

1st Solution.

Find from the logarithmic function the corresponding angle; then, from the angle, the corresponding natural function.

2d Solution.

From article 49 we have,

$$f(a)_1 = \frac{f(a)_R}{R}$$
.
 $\cdot \cdot \cdot \log f(a)_1 = \log f(a)_R - 10.$

Hence, Subtract 10 from the logarithmic function, and find the number corresponding to the resulting logarithm.

63. Examples.

1. Given log sin a = 9.87654, required a and nat $\sin a$. Ans. $a = 48^{\circ} 48' 41''$, nat. $\sin a = .75255$.

2. Given log cos a = 9.84877, required a and nat. cos a. Ans. $a = 45^{\circ} 05' 41''$, nat. cos a. .70595.

3. Given log tan a=10.22708, required a and nat. tan a=168685

4. Given log cot a = 10.15225, required a and nat cot a. Ans. $a = 35^{\circ} 09' 24''$, nat. cot a = 141957.

RIGHT TRIANGLES.

$$\therefore (1) \left\{ \begin{array}{l} p = h \sin P, \\ b = h \sin B. \end{array} \right\} \qquad \left\{ \begin{array}{l} \sin P = \frac{p}{h}, \\ \sin B = \frac{b}{h}, \end{array} \right\}$$

- 1. Either side adjacent to the right angle is equal to the sine of the opposite angle multiplied by the hypotenuse.
- 2. The sine of either acute angle is equal to the opposite side divided by the hypotenuse.

Since the angles P and B are complements of each of each of each P and P are B and sin B cos P; ... (1) and P becomes

$$\begin{cases} P = h \cos B, \\ b = h \cos P, \end{cases} \text{ and (b) } \begin{cases} \cos B = \frac{P}{h}, \\ \cos P = \frac{b}{h}, \end{cases}$$

3. Fifter side adjacent to the right angle is equal to the

The comme of either acute angle is equal to the adja-

 $PH : PN :: HB : NL, \text{ or } b : 1 :: p : \tan P.$ $BH : BT :: HP : TQ, \text{ or } p : 1 :: b : \tan B.$

5. Fither side adjacent to the right angle is equal to the true tof the apposite angle multiplied by the other side.

6. The tangent of either acute angle is equal to the appoint side,

Since the angles P and B are complements of each other, $\tan P = \cot B_i$ and $\tan B = \cot P_i$. 5 and 6 become,

$$\begin{cases}
p = b \cot B, \\
b + p \cot P,
\end{cases}$$
 and (8)
$$\begin{cases}
\cot B = \frac{p}{b}, \\
\cot P = \frac{h}{b},
\end{cases}$$

7. Either ode adjacent to the right angle is equal to the entargent of the adjacent acute angle multiplied by the other side

8. The co-tangent of either acute angle is equal to the adjacent side divided by the opposite side.

BH : BT :: BP : BQ, or p : 1 :: h : sec B.

 $PH : PN :: PB : PL_t \text{ or } b : 1 :: h : sec P_t$

(9)
$$\left\{ \begin{array}{cc} P & \frac{h}{\sec B} \\ h & \frac{h}{\sec P} \end{array} \right\} \quad (10) \left\{ \begin{array}{c} \sec B = \frac{h}{P} \\ \sec P - \frac{h}{b} \end{array} \right\}$$

The hypotenias divided by the secont of the adjacent acute angle.

10 The second of there a not a cale is equal to the hopotenuse divided by the adjacent wide.

Since the angles B and P are complements of each other set B cosec P_t set P cosec B; A: 9) and (10) becomes

(11)
$$\left\{ \begin{array}{cc} F & & \\ P & \text{cosec } P \\ & h \\ & \text{cosec } B \end{array} \right\} \text{ and (12)} \left\{ \begin{array}{cc} \cos c e & P & h \\ & P & \\ & & h \\ & & b \end{array} \right\}$$

11 I'm sale adjace to the could angle is equal to the happetennise directed by the expected the angle of poster that sale

12 The ensure that other write angle is equal to the hypotenion divided by the side opposite that angle.

Scholium. By some authors, principles 2, 4, 6, 8, 10, and 12, have been given in the form of definitions.

Introducing radius into these formulas, by substituting for any function to the radius 1, the corresponding function to the radius R divided by R, and reducing, we have:

$$\begin{pmatrix}
1 & \begin{cases}
h & \sin P \\
P & R
\end{cases}
\end{pmatrix}$$

$$\begin{pmatrix}
1 & \begin{cases}
h & \sin P \\
h & R
\end{cases}
\end{pmatrix}$$

$$\begin{pmatrix}
1 & \begin{cases}
h & \sin P \\
h & R
\end{cases}
\end{pmatrix}$$

$$\begin{pmatrix}
1 & \begin{cases}
h & \sin P \\
h & R
\end{cases}
\end{pmatrix}$$

$$\begin{pmatrix}
1 & \begin{cases}
h & \sin P \\
h & R
\end{cases}
\end{pmatrix}$$

$$\left\{
\begin{array}{c}
h \cos B \\
P - R \\
h \cos P
\end{array}
\right\} = \left\{
\begin{array}{c}
\cos B - \frac{Rp}{h} \\
h \cos P
\end{array}
\right\}$$

$$\left\{
\begin{array}{c}
\cos B - \frac{Rp}{h} \\
h \cos P
\end{array}
\right\}$$

$$\left\{
\begin{array}{l}
b \tan P \\
P - R \\
h - P \tan B \\
R
\end{array}
\right\} = \left\{
\begin{array}{l}
\tan P = \frac{Rp}{b} \\
\tan B - \frac{Rh}{P}
\end{array}
\right\}$$

$$\begin{cases}
p \to \frac{b \cot B}{R}, \\
b \to \frac{p \cot P}{R},
\end{cases} (8) \begin{cases}
\cot B = \frac{R\rho}{b}, \\
\cot P = \frac{R^{\mu}}{P},
\end{cases}$$

$$\left\{
\begin{array}{ll}
p & \frac{Rh}{\sec B}, \\
b & \frac{Rh}{\sec P},
\end{array}
\right\}, (10) \left\{
\begin{array}{ll}
\sec B & \frac{Pl}{r}, \\
\frac{P}{h}, \\
\sec P & \frac{Rh}{h},
\end{array}
\right\}$$

$$\begin{cases}
p - \frac{Rh}{\text{cosec } P}, \\
b = \frac{Rh}{\text{cosec } B},
\end{cases}$$
(12)
$$\begin{cases}
\cos e P - \frac{Rh}{P}, \\
cosec B - \frac{Rh}{h},
\end{cases}$$

Applying logarithms to these formulas, we have.

(1.
$$\begin{cases} \log p \cdot \log h + \log \sin P - 10 \\ \log b = \log h + \log \sin B - 10. \end{cases}$$

12.
$$\begin{cases} \log \sin P = 10 \cdot \log p - \log h, \\ \log \sin B = 10 \cdot \log h - \log h. \end{cases}$$

(3)
$$\left\{ \begin{array}{ll} \log p & \log h + \log \cos B - 10, \\ \log h = \log h + \log \cos P - 10. \end{array} \right\}$$

(4)
$$\left\{ \begin{array}{ll} \log \cos B & 10 + \log p - \log h, \\ \log \cos P \dots 10 + \log b & \log h. \end{array} \right\}$$

(6)
$$\begin{cases} \log \tan P = 10 + \log p - \log b, \\ \log \tan B = 10 + \log b - \log p, \end{cases}$$

(7)
$$\left\{ \begin{array}{ll} \log p = \log b + \log \cot B - 10, \\ \log b = \log p + \log \cot P - 10. \end{array} \right\}$$

$$\left\{\begin{array}{ll} \log \cot B = 10 + \log p - \log b, \\ \log \cot P = 10 + \log b - \log p. \end{array}\right\}$$

(9)
$$\begin{cases} \log p = 10 + \log h - \log \sec B, \\ \log b = 10 + \log h - \log \sec P, \end{cases}$$

{10,
$$\begin{cases} \log \sec B = 10 + \log h - \log p, \\ \log \sec P = 10 + \log h - \log b. \end{cases}$$
}

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$$\begin{cases} \log p = 10 + \log h - \log \csc P. \end{cases}$$

 $\log b = 10 + \log h - \log \csc B. \end{cases}$

$$\begin{cases} \log \operatorname{cosec} P = 10 + \log h - \log p, \\ \log \operatorname{cosec} B = 10 + \log h - \log b. \end{cases}$$

65. Case I.

Given the hypotenuse and one acute angle, required the remaining parts.

1. Given
$$\begin{cases} h = 365, \\ P = 33^{\circ} \ 12', \end{cases}$$
 Requir. $\begin{cases} B, \\ p, \\ b, \end{cases}$ Req

Either side adjacent to the right on the is equal to the sine of the appoint angle, multiplied by the topoleruse

$$p = h \sin P$$
.

Latroducing radius, we have, $p = \frac{\hbar \sin P}{R}$.

Applying logarithms, we have,

 $\log p - \log h + \log \sin P = 10.$

log h (365) 2,56229

log sin P (33° 12') 9 73843

 $\log p = -230072 \dots p = 199.85$

In like manner, from either formula, $b = h \sin B$ or $b = h \cos P$, we find b = 305.41.

2. Given
$$\left\{ \begin{array}{l} b = 73.26, \\ B = 49^{\circ} \ 12^{\circ} 20^{\circ}, \end{array} \right\}$$
 Requir. $\left\{ \begin{array}{l} P = 40^{\circ} \ 47^{\circ} \ 10^{\circ} \\ b = 55^{\circ} \ 1625, \\ P = 47^{\circ} 8644, \end{array} \right.$

3. Given
$$\left\{ \begin{array}{l} h = 2195, \\ P = 27^{\circ} 38' 50', \end{array} \right\}$$
 Requir. $\left\{ \begin{array}{l} B = (2-21'10'') \\ p = 1018.512 \\ b = 1944.364. \end{array} \right.$

66. Case II.

Given the hypotenuse and one side adjacent to the right angle, required the remaining parts.

1. Given
$$\left\{\begin{array}{l} h=112\\ p=97. \end{array}\right\}$$
 Required $\left\{\begin{array}{l} P_{i}\\ *B_{i}\\ b_{i} \end{array}\right\}$

The sine of either neute angle is equal to the opposite

$$\therefore \sin P = \frac{p}{h}$$
.

Introducing radius, and multiplying by R, we have.

$$\sin P = \frac{Rp}{\hbar}$$

Applying logarithms, we have,

 $\log \sin P = 10 + \log p - \log h$.

 $\log p$ (97) 1.98677

 $\log |h|(112) = 2.04922$

log sin P 9.93755 ... P 60° 00′ 17″.

 $B = 90^{\circ} - P - 90^{\circ} = 60^{\circ} \cdot 00' \cdot 17'' - 29^{\circ} \cdot 59' \cdot 43''$.

 $b = h \sin B$, or $b = h \cos P$, $\therefore b = 55.991$.

We can also find b as follows:

$$h=1/h^2-p^2=1/(\overline{h}+p)/(\overline{h-p}).$$

 $\log b = \frac{1}{2} [\log (h + p) + \log (h - p)].$

2. Given
$$\begin{cases} h = 7269 \\ h = 3162 \end{cases}$$
 Required $\begin{cases} B = 25^{\circ} 47' \ 07'', \\ P = 64^{\circ} 12' \ 53'', \\ p = 6545, \end{cases}$

Given
$$\{h = 1114\}$$
 Required $\{P = 19^{\circ} 43' \ 36'', B = 70^{\circ} \ 16' \ 24'', b = 418.33,$

67. Case III.

that a rive rate adjust not to the right angle and one arms angle, required the removeding parts.

1 Given
$$\left\{ \frac{h - 152.87}{P - 50^{\circ} 18'32''} \right\}$$
 Requir. $\left\{ \frac{B_{c}}{p_{c}} \right\}_{p_{c}}$

Either side adjurnt to the right angle is equal to the tangent of the opposite a site multiplied by the other side.

$$\varphi_{+} = p - b \tan P_{+}$$

Introducing radius and applying logarithms, as it the preceding cases, we find p 183.95.

Eaker side adjacent to the right angle is equal to the co-sine of the adjuscent acute angle multiplied by the hypotenuse.

$$\therefore b + h \cos P; \quad \therefore h = \frac{b}{\cos P}.$$

Introducing radius and applying logarithms, as above, we shall find h 239,05.

2. Given $\begin{cases} p = 3963.35 \text{ miles} = \text{the earth's radius.} \\ P = 57' 2.3'' = \text{the moon's horizontal parallax} \end{cases}$

Required h, the distance of the moon from the earth Ans. h = 235589 miles

3. Given $\begin{cases} p = 3963.35 \text{ miles} - \text{ the earth's tracus.} \\ P = 8.97 \text{ the sun's horizontal parallax.} \end{cases}$

Required h, the distance of the sun from the earth. Ans. h 91852000 miles

Scholium. Sin $8.9'' = \sin 1' \times \frac{8.9}{66}$.

• $\log \sin 8.9'' = \log \sin 1' + \log 8.9 + a.c. \log 60 - 10$

68. Case IV.

Given the two sides adjacent to the right angle, required the remaining parts.

1. Given
$$\{p = 29.37, b = 37.29, \}$$
 Requir. $\{P, B, B, b = 10.37.29, b$

The tangent of either acute angle is equal to the opposite eide dividal by the adjacent side.

$$\therefore$$
 tan $P = \frac{P}{b}$

Introducing radius and applying logarithms, we shall find that P = 38° 13' 28".

$$B = 90^{\circ} - P = 90^{\circ} - 38^{\circ} 13' 28'' 51^{\circ} 46' 32''$$

Either side adjacent to the right angle is equal to the sine of the opposite angle multiplied by the hypotenuse.

...
$$p = h \sin P$$
. ... $h = \frac{P}{\sin P}$.

Introducing radius and applying logarithms, we find h = 47.466.

2. Given
$$\begin{cases} p = 694.73. \\ b = 8372.1. \end{cases}$$
 Required $\begin{cases} P = 4^{\circ} 44' \ 37''. \\ B = 85^{\circ} \ 15' \ 23''. \\ h = 8401. \end{cases}$

3. Given
$$\left\{ \begin{array}{l} p = 101, \\ b = 103 \end{array} \right\}$$
 Required $\left\{ \begin{array}{l} P = 44^{\circ} \ 26' \ 17'', \\ B = 45^{\circ} \ 33' \ 43'', \\ h = 111 \ 253 \end{array} \right.$

4. Given
$$\left\{ \begin{array}{ll} p = 1728, \\ b = 1575. \end{array} \right\}$$
 Required $\left\{ \begin{array}{ll} P = 47^{\circ} \; 39^{\circ} \; 07^{\prime\prime}, \\ B = 42^{\circ} \; 20^{\circ} \; 53^{\prime\prime}, \\ h = 2338.1. \end{array} \right.$

OBLIQUE TRIANGLES.

69. Case I.

Given one side and two angles, required the remaining Jairia.

Let ABC be an oblique triangle, and let the sides opposite the angles A, B, and C be denoted respectively by a, b and c.



Let the angles A and B and the side a be given, and the angle C and the sides b and c be required.

We find C from the formula,

$$C = 180^{\circ} - (A + B).$$

Draw the perpendicular p from the vertex C to the sale c, thus forming two right triangles. There are two cases:

14. When the perpendicular falls on the side c.

From the principles of the right triangle we have,

 $p = b \sin A$ and $p = a \sin B$.

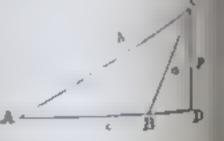
... $b \sin A = a \sin B$.

... (1) $\sin A : \sin B :: a : b$.

2!. When the perpendicular falls on e produced.

$$p = b \sin A$$
 and $p = a \sin CBD$.

But CBD is the supplement of CBA, or B of the triangle. Since the sine of an angle is equal to the sine of its supplement,



sin $CBD = \sin B$; ... $p = a \sin B$ $b \sin A = a \sin B$.

.. (1) sin A: sin B:: a: b.

In like manner we may find,

(2) sin A : sin C :: a : c.

Hence, The sine of the angle opposite the given side is to the rine of the angle opposite the required side as the given side is to the required side.

Introducing radius by substituting for the function to the radius I, the corresponding function to the

radius R divided by R, and reducing, the proportions (1) and (2) will be of the same form as before substitution, and hence are true for any radius.

From proportions (1) and (2), we find,

(3)
$$b = \frac{a \sin B}{\sin A}$$
, (4) $c = \frac{a \sin C}{\sin A}$.

Applying logarithms to (3) and (4), we have,

- (5) $\log b = \log a + \log \sin B + a$. c. $\log \sin A 10$.
- (6) $\log c = \log a + \log \sin C + a$. c. $\log \sin A 10$.

70. Examples.

1. Given
$$\begin{cases} A = 35^{\circ} 45', \\ B = 45^{\circ} 25', \\ a = 7985, \end{cases}$$
 Req. $\begin{cases} C, \\ b, \\ c, \end{cases}$

$$C = 180^{\circ} - (A + B) = 180^{\circ} - 81^{\circ} \cdot 18' = 98^{\circ} \cdot 47'$$

Since the sine of the angle opposite the given side is to the sine of the angle opposite the required side as the given side is to the required side, we have the proportion,

$$\sin A : \sin B :: a : b, \dots b = \frac{a \sin B}{\sin A}.$$

 $\log b = \log a + \log \sin B + a \cdot c \cdot \log \sin A - 10.$

 $\log a \ (7985) = 3.90227$ $\log \sin B \ (45^{\circ} \ 28') = 9.85299$

a.c. $\log \sin A (35^{\circ} 45') = 0.23340$ $\log b = 3.98866$... b = 9742.25.

In like manner we have the proportion,

sin Arsin Crara range a sin C sin A

... log c log a + log sin C + a. c. log sin A - 10

log a (79×5) 8,90227 log sin C (98° 47°) 9,99488

a c log sin A (35° 45') = 0.23340 log c 4 13055 ... c = 13506,88,

In finding log sin 98° 47', take the supplement of 47, which is 81° 13', and find log sin 81° 13'.

2. Given
$$\begin{cases} A = 50^{\circ} \ 30' \ 40'', \\ B = 70^{\circ} \ 45' \ 30'', \\ a = 478.35 \ yd. \end{cases}$$
 Req.
$$\begin{cases} C = 58^{\circ} \ 43' \ 50' \\ b = 585.2 \ yd. \\ c = 529.8 \ yd. \end{cases}$$
 3. Given
$$\begin{cases} B = 65^{\circ} \ 25' \ 35'', \\ C = 60^{\circ} \ 28' \ 34'', \\ b = 12.25 \ \text{miles}, \end{cases}$$
 Req.
$$\begin{cases} A = 54^{\circ} \ 05' \ 51'', \\ C = 10.91 \ \text{miles}, \end{cases}$$

71. Case II.

Given two sides and an angle opposite one of them, required the remaining parts.

1. WHEN THE GIVEN ANGLE IS ACUTE.

Let the sides a and b and the angle A be given, and the remaining parts be required.

Let the perpendicular p be drawn from C to the opposite side. Then we shall have,



$$p = b \sin A$$
.

14. If a > p and a < b, there will be two solutions

For, if with C as a center and a as radius a circumference be described, it will intersect the side opposite C in two points, B and B, and either triangle, ABC of ABC will fulfill the conditions of the problem, since

it will have two sides and an angle opposite one of them the same as those given. Hence, there will be two solutions if a has any value between the limits p and b.

2d. If a = p, there will be but one solution.

For, as a diminishes and approaches A C B p, the two points B and B' approach; and if a = p, B and B' will unite, the arc will be tangent to c, and the two triangles will become one, and there will be one solution.

3d. If a = b, there will be but one solution.

For, as a increases and approaches

b, the points B and B separate, the

triangle ABC increases, and the triangle ABC decreases;
and when a becomes equal to b, the triangle ABC vanishes, and there remains but one triangle, or there is but one solution.

4th. If a > b, there will be but one solution.

For, although there are two triangles ABC and ABC, the latter is
excluded by the condition that the given angle A is
acute, since CAB' is obtuse, and there remains but
one triangle ABC which satisfies the conditions, or
there is but one solution.

5th. If a < p, there will be no solution.

For the arc described with C as center and a as radius will neither intersect the opposite side nor be tangent to it. The triangle can not be constructed, or there will be no solution.

2. WHEN THE GIVEN ANGLE IS OBTUSE.

14. If a > b there will be but one solution.

For, although there are two triangles ABC and ABC, the latter is excluded by the conditions of the problem, since the angle (C.1B' is acute while the given angle is obtuse. There remains but one triangle, ABC, which satisfies all the conditions of the problem, or there is but one possible solution.

22. If a = b there will be no solution.

For as a diminishes and approaches b_i B will approach A_i ; and when a becomes equal to b_i B will unite with A_i and the triangle ABC will vanish. The triangle ABC will remain, but will be excluded by the conditions of the problem, since the angle CAB is acute while the given angle is obtuse.

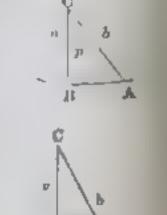
3d. If a < b there will be no solution, for then,

If a > p there will be two triangles, ABC and AB^*C , but both are excluded by the condition that the given angle is obtuse.

If a = p the two triangles reduce to one, right-angled at B, which is excluded by the condition that the given angle is obtuse.

If a < p no triangle can be constructed with the given parts, and there will be no solution





72. Summary of Results.

When A < 90°.

Two Solutions, If a > p and a < b.

One Solution, $\begin{cases} 1st, & \text{If } a = p, \\ 2d, & \text{If } a = b, \\ 3d, & \text{If } a > b. \end{cases}$

No Solution, If a < p

2. When A > 90°.

One Solution, If a > b.

No Solution, $\begin{cases} 1st. & \text{If } a = b, \\ 2t & \text{If } a < b, \end{cases}$

73. Method of Computation.

Reversing the order of the couplets of the proportion in Case I, we have

$$-1$$
) $a:b+\sin A$, $\sin B$

Hence, The side opposite the given angle is to the side opposite the equival angle, as the sine of the given angle is to the sur of the required a given

(1) gives
$$2 \sin B = \frac{h \sin A}{a}$$
.

(3) $\log \sin B = \log b + \log \sin A + a$. c. $\log a - 10$.

If there is but one solution, take from the table the angle B corresponding to $\log \sin B$, if there are two solutions, take B and its supplement B', for both correspond to $\log \sin B$.

We find C from the formula,

C
$$180^{\circ}$$
 (A + B or C 180° - (A + B').

We find a from the proportion,

$$\sin A : \sin C :: a : c, \therefore c = \frac{a \sin C}{\sin A}$$

... $\log c = \log a + \log \sin C + a$. c. $\log \sin A = 10$

74. Examples.

1. Giv.
$$\begin{cases} a = 9.25, \\ b = 12.56, \\ A = 30^{\circ} 25', \end{cases} \text{ Req. } \begin{cases} B, \\ C, \\ c, \end{cases}$$

 $p = b \sin A$.

Introducing R and applying logarithms, we have

$$\log p = \log b + \log \sin A - 10.$$

$$\begin{array}{ll} \log b \ (12.56) &= 1.09899 \\ \log \sin A \ (30^{\circ} \ 25') = 9.70439 \\ \log p &= 0.80338 \\ \end{array}$$

Since a > p and a < b, there are two solutions.

Since the side opposite the given angle is to the side opposite the required angle as the sine of the given angle is to the sine of the required angle, we have the proportion,

$$a:b::\sin A:\sin B$$
, $\sin B=\frac{b\sin A}{a}$.

 $\log \sin B - \log b + \log \sin A + a$, c, $\log a - 10$.

$$C = 180^{\circ} - (A + B) - 106^{\circ} 9' 19'',$$

$$C' = 180^{\circ} - (A + B') - 13^{\circ} 0' 41'',$$

$$\sin A : \sin C :: a : c \longrightarrow c \longrightarrow \sin C$$

 $\log c - \log a + \log \sin C + a.c. \log \sin A - 10.$

Taking the value of C, we have,

log a (9.25)
$$0.96614$$

log sin C (106° 9' 19") = 9.98250
a.e. log sin A (30° 25') = 0.29561
log c = 1.24425 .*. $c = 17.549$.

Taking the value of C', we have,

2. Given
$$\begin{cases} a = 20.35, \\ b = 20.35, \\ d = 7.2^{\circ} .35' .27'', \end{cases}$$
 Req.
$$\begin{cases} J^{2} = 7.2^{\circ} .35' .27', \\ C = 7.4^{\circ} .40 .06'', \\ c = 24.725 \end{cases}$$

3. Given
$$\begin{cases} a = 645.8, \\ b = 234.5, \\ A = 48^{\circ} 35', \end{cases}$$
 Req.
$$\begin{cases} R = 15^{\circ} 48'.04'', \\ C = 115^{\circ} 30'.56'', \\ C = 770.53, \end{cases}$$

1 Given
$$\left\{ \begin{array}{l} a = 17, \\ b = 40.25 \\ A = 27^{\circ} 43' 15'', \end{array} \right\}$$
 Req. $\left\{ \begin{array}{l} B \\ C \\ c, \end{array} \right\}$ No Solution.

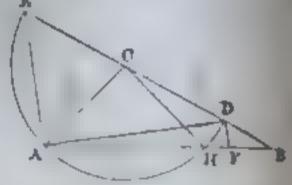
5. Given
$$\begin{cases} a = 94.26, \\ b = 126.72, \\ A = 27^{\circ}.50'. \end{cases}$$
 Req.
$$\begin{cases} B \approx \begin{cases} 38^{\circ}.52'.46'', 2' \\ 141^{\circ}.7'.14'', \\ 11^{\circ}.2'.46'' \\ c = \begin{cases} 185.1.30 \\ 38.682, \end{cases} \end{cases}$$

6. Given
$$\begin{cases} a = 1800, \\ b = 2000, \\ B = 111^{\circ} 15', \end{cases}$$
 Req. $\begin{cases} A = 57^{\circ} \quad 0' \quad 50'', \\ C = 11^{\circ} \quad 44' \quad 10'', \\ c = 436.49, \end{cases}$

75. Case III.

re airding parts.

Let ABC be a triangle, and let the sides opposite the angles A, B, C, be denoted, respectively, by a, b, c. Let a and b, and their



included angle C_i be given, and the remaining parts, A_i , B_i , and c_i required.

The sum of the angles A and B is found from the formula,

 $A + B = 180^{\circ} - C$.

With C as a center, and b, the shorter of the two given sides, as a radius, describe a circumference out ting a in D, a produced in E, and c in H. Draw AE. 1D, CH, and DF parallel to AE. The angle DAE is a right angle, since it is inscribed in a semi-circle; hence, its alternate angle, ADF, is also a right angle.

The angle ACE being exterior to the triangle ABC, is equal to A + B. But ACE having its vertex at the center, is measured by the intercepted are AF. The inscribed angle ADE is measured by one-half the are AE; hence, $ADE = \frac{1}{2}ACE = \frac{1}{2}(A + B)$.

CH · CA, since they are radii of the same circle; hence, the angle CHA = A. The angle CHA being exterior to the triangle CHB is equal to HCB + B; hence,

HCB + B = A. ... HCB = A - B.

But HCB, having its vertex at the center, is measured by the intercepted are DH_J and DAF, being an inscribed angle, is measured by one-half the arc DH_J ; hence, $DAF = \frac{1}{2}HCB := \frac{1}{2}(A - B_J)$.

In the right triangles ADE and ADF we have

$$AE = AD \tan ADE = AD \tan \frac{1}{2}(A + B).$$

 $DF = AD \tan DAF = AD \tan \frac{1}{2}(A - B).$

From the similar triangles, ABE and FBD, we have

Since CE = CA, BE = BC + CA = a + b. Since CD = CA, BD = BC - CA = a - b.

Substituting the values of BE, BD, AE, and DF in the above proportion, and omitting the common factor AD in the second couplet, we have

$$a+b:a-b::\tan \frac{1}{2}(A+B):\tan \frac{1}{2}(A-B).$$

Hence, In any plane triangle, the sum of the sides including are angle or to their difference as the tangent of half the same of the other two angles is to the tangent of half their difference.

We find from the proportion, the equation

$$\tan \frac{1}{2}(A-B) = \frac{(a-b)\tan \frac{1}{2}(A+B)}{a+b}$$
.

$$\frac{1}{1600} \log \tan \frac{1}{2} (A - B) = \log (a - b) + \log \tan \frac{1}{2} (A + B) + a c \log (a + b) - 10.$$

We have now found $\frac{1}{2}(A+B)$ and $\frac{1}{2}(A-B)$.

$$A = \frac{1}{2}(A + B) + \frac{1}{2}(A - B), \quad B = \frac{1}{2}(A + B) - \frac{1}{2}(A - B),$$

$$\sin A : \sin C : \text{ as in } C$$

$$\frac{\sin A : \sin C :: a : c_{i-1}, c_{i-2}}{\sin A}$$

 $\log c = \log a + \log \sin C + a$, c. $\log \sin A - 10$, 8, N, 6,

76. Examples.

1. Given
$$\begin{cases} a = 37.50 \\ b = 23.75 \\ C = 68^{\circ} \cdot 25' \end{cases}$$
 Req. $\begin{cases} A \\ B \\ c \end{cases}$

$$A + B = 180^{\circ} - C = 111^{\circ} 35'$$
.

$$a+b:a-b::\tan \frac{1}{2}(A+B):\tan \frac{1}{2}(A-B).$$

...
$$\tan \frac{1}{2}(A-B) = \frac{(a-b)\tan \frac{1}{2}(A+B)}{a+b}$$
.

...
$$\log \tan \frac{1}{2}(A-B) = \log (a-b) + \log \tan \frac{1}{2}(A+B) + a.c. \log (a+b) - 10$$

$$\log (a-b) (13.81) = 1.14019$$

 $\log \tan \frac{1}{4} (A+B) (55^{\circ} 47' 30'') = 10.16761$

$$\log \tan \frac{1}{2}(A-B)$$
 9.52027
 $\therefore \frac{1}{2}(A-B) = 18^{\circ} \cdot 19' \cdot 55'$

$$A = \frac{1}{4}(A+B) + \frac{1}{4}(A+B) = 74^{\circ} - 7' \cdot 25''$$

$$B = \frac{1}{4}(A+B) - \frac{1}{4}(A-B) = 37^{\circ} 27' 35''$$

$$\sin A : \sin C :: a : c, \dots c - \frac{a \sin C}{\sin A}$$

 $\log c = \log a + \log \sin C + a$, c. $\log \sin A = 10$.

$$2. \ \mbox{Given} \left\{ \begin{array}{ll} a = -996.63, \\ b = 712.83, \\ C = 72^{\circ}.29'.48'', \end{array} \right\} = \mbox{Req.} \left\{ \begin{array}{ll} A = -66^{\circ}.30'.37'' \\ B = -40^{\circ}.59'.35'', \\ c = -1036.35, \end{array} \right.$$

3. Given
$$\begin{cases} b = 776.525. \\ c = 234.5. \\ A = 48^{\circ} | 35'. \end{cases}$$
 Req.
$$\begin{cases} B = 115^{\circ} | 36' | 56''. \\ C = 15^{\circ} | 48' | 04''. \end{cases}$$

$$(a = 11.7209.)$$

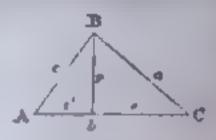
$$(A = 60^{\circ} | 25' | 34''.)$$

4. Given
$$\begin{cases} a = 11.7209, \\ c = 10.9232, \\ B = 65^{\circ} \cdot 25' \cdot 35'', \end{cases} \text{Req.} \begin{cases} A = 60^{\circ} \cdot 25' \cdot 34'', \\ C = 54^{\circ} \cdot 08' \cdot 51'', \\ b = 12.256, \end{cases}$$

77. Case IV.

Given the three sides of a triangle, required the angles.

Let ABC be a triangle, take the longest side for the base, and draw the perpendicular p from the vertex B to the base.



Denote the segments of the base by s and d' respectively.

Then, (1)
$$e^2-s'^2=p^2$$
, and (2) $a^2-s^2=p^2$.

. (3)
$$c^2 - a'^2 = a^2 - a^2$$
, . (4) $a^2 - a'^3 = a^2 - c^3$.

'. (5)
$$(s+s')$$
 $(s-s') = (a+c)$ $(a-c)$.

Hence, The same of the sagrents of the base is to the same of the other sides as the difference of those sides is to the difference of the segments.

(6) gives (7)
$$a-c=\frac{(a+c)(a-c)}{a+a'}$$
.

... (8)
$$\log (a-s') = \log (a+s) + \log (a-s') + a. c. \log (s+s') - 10.$$

In case the sides of the triangle are small, find s s' from (7); otherwise, it will be more convenient to employ (8).

HEIGHTS AND DISTANCES.

69

Having s - s' and s-- s', we find s and s' thus,

$$(9 \times \frac{1}{2} \times \frac{1}{4}) + \frac{1}{2} \times \frac{1}{4}, \quad (10) \times \frac{1}{2} (8 + 8') - \frac{1}{2} 8 - k'$$

$$(11) \cos A = \frac{1}{4}, \quad (12) \cos C = \frac{8}{4}.$$

Introducing R, reducing, and applying logarithms,

(13)
$$\log \cos A = 10 + \log s' - \log c$$
.

(11)
$$\log \cos C = 10 + \log s - \log a$$
.

From which we find A and C.

Then, (15)
$$B = 180^{\circ} - (A + C)$$
.

78. Examples.

1. Given
$$\begin{cases} a = 125, \\ b = 150, \\ c = 100, \end{cases}$$
 Req. $\begin{cases} A, \\ B, \\ C, \end{cases}$

...
$$s = s' - \frac{(a+c)(a-c)}{s+s'} - \frac{225 + 25}{150} - \frac{275}{375}$$
.

 $s = \frac{1}{4}(s+s') + \frac{1}{4}(s-s') - 75 + 18.75 - 93.75$.

 $s' - \frac{1}{4}(s+s') - \frac{1}{4}(s-s') - 75 - 18.75 - 56.25$.

 $\cos A = \frac{s'}{c}$, or introducing R , $\cos A = \frac{Rs'}{c}$.

... $\log \cos A = 10 + \log s' - \log c$.

cos $C = \frac{8}{a}$, or introducing $R_1 \cos C = \frac{R^2}{a}$.

... $\log \cos C = 10 + \log s - \log a$.

$$\log s (93.75)$$
 1.97197
 $\log a (125) = 2.09691$
 $\log \cos C = 9.87506$... $C = 41^{\circ} 21' 31''$.

$$B = 180 - (A + C) = 82^{\circ} 49' 08''$$

2. Given
$$\begin{cases} a = 832.21. \\ b = 345.46. \\ c = 237.61. \end{cases}$$
 Required $\begin{cases} A = 66^{\circ} 30' 35''. \\ B = 72^{\circ} 29' 53''. \\ C = 40^{\circ} 59' 32''. \end{cases}$

3. Given
$$\begin{cases} a = 561 \\ b = 1308 \\ c = 1086, \end{cases}$$
 Required $\begin{cases} A = 41^{\circ} 00' 38'', \\ B = 83^{\circ} 25' 14'', \\ C = 55^{\circ} 34' 08'', \end{cases}$

4. Given
$$\begin{cases} a = 251.25 \\ b = 302.5, \\ c = 342 \end{cases}$$
 Required
$$\begin{cases} .1 = 45^{\circ}.22'.44'', \\ B = 58^{\circ}.58'.20'', \\ c' = 75^{\circ}.38'.59'', \end{cases}$$

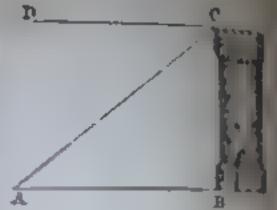
APPLICATION TO HEIGHTS AND DISTANCES

79. Definitions.

- 1. A horizontal plane is a plane parallel to the horizon.
- 2. A vertical plane is a plane perpendicular to a horizontal plane.
- 3. A horizontal line is a line parallel to a horizontal plane.
- 4. A vertical line is a line perpendicular to a horizontal plane.
- 5. A horizontal angle is an angle whose plane is

et A vertical angle is an angle whose plane be vertical.

7. An angle of elevation is a verticle angle, one of whose sides is horizontal, and the nuclined side above the horizontal side. Thus, BAC.



S. An angle of depression is a vertical angle, one is whose sides is horizontal, and the inclined side belt the horizontal side. Thus, DCA.

80. Problems.

1. Wishing to know the height of a tree stands.

on a horizontal plane, I measured from the tree the horizontal line BA, 150 ft., and found the angle of elevation, BAC, to the top of the tree to be 35° 20°. Required the height of the tree.



Ans. 106,335 ft.

2 In surveying a tract of land, I found it impract able to measure the side AB

on account of thick brushwood lying between A and B. I therefore measured AE, 7.50 ch., and EB, 8.70 ch., and



found the angle $AEB = 38^{\circ}$ 46'. Required AB_{\circ}

3. One side of a triangular field is double another their included angle is 60°, and the third side is 1 ch. Required the longest side.

Ans. 17.32 cl.

4. Wishing to know the width of a river, from the

point C on the other bank, I measure the distance AB, 75 yd., and find the angle $BAC = 87^{\circ}$ 28' 30", and the angle $ABC = 47^{\circ}$ 38' 25". Required AC, the width of the river. Ans. 78.53 yd.



5. I find the angle of elevation, BAC, from the foot of a hill to the top to be 46° 25′ 30″. Measuring back

from the hill, AD = 500 ft., I find the angle of elevation $ADC = 25^{\circ}$ 38' 40'. Required BC, the vertical height of the hill. Ans. 441.87 ft.



6. From the foot of a tower standing at the top of a

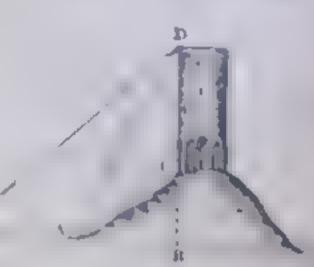
declivity, I measured AB = 45 ft., and the angle $ABD \approx 50^{\circ}$ 15'. I also measured, in a straight line with AB, BC = 68 ft., and the angle $BCD = 30^{\circ}$ 45'. Required AD, the height of the tower.



height of the tower. Ans. 82.94 ft.

7. Wishing to know the height of a tower standing

angle of elevation, BAC, to the top of the hill to be 44° 35′, and the angle of elevation to the top of the tower to be 59° 48′. Measuring the



horizontal line AE, 275 ft., I find the angle of eleva-

tion to the top of the tower to be 46° 25'. Required the height of the tower.

Ans. 317.143 ft.

$$\begin{array}{cccc} & DC & 24 \text{ el} \\ CDB & 45^{\circ}, \\ 8. \text{ Given} & BDA & 50^{\circ}, \\ DCA & 48^{\circ}, \\ ACB & 60^{\circ}, \end{array}$$

Required AB = 38.61 ch.

9. Given AB 800 yd., AC 600 yd., BC - 400 jd., ADC 33° 45′, BDC 22° 30′. Res c quired DA, DC, DB.

 $A_{DA} DA = 710.15 \text{ yd., } DC = 1042.5 \text{ yd., } DB = 934.28 \text{ yd.}$

RELATIONS OF CIRCULAR FUNCTIONS.

St. Fundamental Formulas.

Let a the angle OCT the arc OT, and CO CI -1. Then, we have $MT = CN = \sin a$, NT $CM = \cos a$, MO = vers a, NO covers a, OR tan a, OS -1 -1 By articles 30-46, $\sin (90^{\circ}-a)$

By articles 39-46, $\sin (90^{\circ}-a) =$ $\cos a$, $\cos (90^{\circ}-a) = \sin a$, etc.

From the diagram we have

$$MT^2 + CM^2 - CT^2$$

Substituting the values of MT, CM, and CT, we have

(1)
$$\sin^2 a + \cos^2 a = 1$$
.

Hence, The square of the sine of any are plus the square of its co-sine is apail to 1.

From (1) we have, by transposition,

- (2) $\sin^2 a = 1 \cos^2 a_i$
- (3) $\cos^2 a = 1 \sin^2 a$. Hence,
- 1. The square of the sine of any are is equal to 1 minus the square of its co-sine.
- 2 The square of the co-sine of any are is equal to 1 minus the square of its sine.

From the diagram we have

$$MO = CO - CM$$
.

Substituting the values of MO, CO, and CM, we have

(4) vers
$$a=1-\cos a$$
.

Hence, The versed-sine of any are is equal to 1 minus

From
$$\cos (90)^2 - a$$
 1 $\cos (90)^2 - a$ 2. From a 1 $-\sin a$.

Hence, The co-versal-sine of any a c is open to 1 menus

From the diagram we have

-'- (6)
$$\tan a = \frac{\sin a}{\cos a}$$
.

Hence, The tangent of any are is equal to its one

B. N. 7

CHICULAR FUNCTIONS.

75

$$\frac{\sin (90^{\circ} - a)}{\cos (90^{\circ} - a)} = \frac{\sin (90^{\circ} - a)}{\cos (90^{\circ} - a)}.$$

$$\frac{\cos a}{\sin a}.$$

Honce. The co-tangent of any arc is equal to its co-si e

$$(6) \times (7) = (8)$$
 tan a cot a 1.

Hence, The tangent of any are into its co-tangent a equal to 1.

(8)
$$\div \cot a$$
 (9) $\tan a = \frac{1}{\cot a}$.

Hence, The tangent of any are is equal to the recipest

(8) + tan
$$a = (10)$$
 cot $a = \frac{1}{\tan a}$.

Hence, The co-tangent of any arc is equal to the rest

$$CM : CO :: CT : CR$$
, or $\cos a : 1 :: 1 : \sec a$.
.:. (11) $\sec a = \frac{1}{\cos a}$.

Hence, The secant of any are is equal to the recipies

... sec
$$(90^{\circ} - a) = \frac{1}{\cos (90^{\circ} - a)}$$
.

... $(12) \ \text{cosec} \ a = \frac{1}{\sin a}$.

Hence, The co-secant of any arc is equal to the record of its sine.

$$\widehat{CR}^2 = \widehat{CO}^2 + \widehat{OR}^2,$$

$$\therefore (13) \sec^2 a = 1 + \tan^2 a.$$

Hence, The square of the secant of any are is equal to 1, plus the square of its tangent.

...
$$\sec^2 (90^{\circ} - a) = 1 + \tan^2 (90^{\circ} - a)$$
.
... $(14) - \csc^2 a = 1 + \cot^2 a$.

Hence, The square of the co-secunt is equal to 1, plus the square of the co-tangent.

82. Summary of Fundamental Formulas.

1. $\sin^3 a + \cos^2 a = 1$.	9. $\tan a = \frac{1}{\cot a}$
2, $\sin^2 a = 1 - \cos^2 a$.	10. cot a 1 1 -
3. $\cos^2 a = 1 - \sin^2 a$.	1
4. vers a = 1 - cos a.	11. $\sec a = \frac{1}{\cos a}$
5. covers $a = 1 - \sin a$.	12 cosec a $\frac{1}{\sin a}$.
6. $\tan a = \frac{\sin a}{\cos a}$.	1
7. $\cot a = \frac{\cos a}{\sin a}$	13. $\sec^2 a = 1 + \tan^2 a$.
8. $\tan a \cot a = 1$.	14. $\csc^2 a = 1 + \cot^2 a$.

83. Problems.

- 1. Prove that the above formulas become homogeneous by the introduction of R.
- 2. Deduce formulas (5), (7), (12) and (14) from the diagram.
- 3. Prove that the above formulas are true if a is in the second, third, or fourth quadrant.

84. Each Function in Terms of the Others,

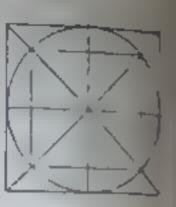
sin a 1/1 costa.	vers $a = 1 - \nu + \frac{1}{\sin^2 a}$
sm a 1 2 vers a -vers a.	vers $a = 1 - \cos a$.
sin a = 1 covers a.	vers a - 1 - V 2 evs a - evs a
$\frac{\tan a}{1 + \tan^2 a}$	vers $a = 1 - \frac{1}{1 + \tan^2 a}$
$\sin a = \frac{1}{\sqrt{1 + \cot^2 a}}.$	$\operatorname{vers} a = 1 - \frac{\cot a}{1 + \cot^2 a}$
$\sin a = \frac{1}{\sec^2 a - 1}$	$\text{vers } a = \frac{\sin a - 1}{\sec a}.$
$\sin a = \frac{1}{\cos e c}$	vers a = 1 cosec a cosec a
$\cos a = 1 \cdot 1 - \sin^2 a.$	covers a 1 — sin a.
cos a 1 - vers a.	covers $a=1-1$ $1-\cos^2 a$.
ers a - 1 2 evs a - evs2 a.	eve a = 1 1 2 vs a vs a
$\cos a = \frac{1}{1 + \tan^2 a}.$	covers a 1- tan a 1 1 + tan a
$\cos a = \frac{\cot a}{1 + \cot^2 a}.$	covers $a=1-\frac{1}{1+\cot^2\theta}$
$\cos a - \frac{1}{\sec a}$.	covers $a = 1 - \frac{1}{\sec^3 a} = \frac{1}{\sec^3 a}$
$\cos a = \frac{1 \cos ee^2 a - 1}{\cos ee}.$	covers $a = \frac{\cos e c}{\cos e c} \frac{a - 1}{a}$.

84. Each Function in Terms of the Others.

tan a sin a	$\sec a = \frac{1}{1 - \sin^2 a}.$
$\tan a = \frac{1}{\cos a}$	$\frac{2a}{\cos a}.$
$\tan a = \frac{\sqrt{2} \text{ vs } a}{1 - \sqrt{2} \text{ vs } a}$	$\frac{-vs^2 a}{sa} \sec a = \frac{1}{1 - vers a}.$
$\tan a = \frac{1 - \epsilon}{\sqrt{2 \cos a}}$	$\frac{1}{-\cos^2 a} \sec a = \frac{1}{\sqrt{2\cos a - \cos^2 a}}$
	sec a = 1 1 - tan2 a.
$\tan a = \frac{1}{\cot a}$.	1 1 -h cot 3 a
$\tan a = V \sec^2 a$	$-1. \qquad \frac{1 + \cot^2 a}{\cot a}.$
$\tan a = \frac{1}{1 \text{ cosec}^2}$	$\frac{a-1}{a-1} \cdot \frac{\sec a - \frac{\csc a}{1 \cdot \csc^2 a - 1}}{1 \cdot \csc^2 a - 1}.$
cot a 1 1 sin a	- (11500 4
$\cot a = \frac{\cos a}{1 \cdot 1 - \cos a}$	$\frac{1}{\sqrt{a}}$ cosec $a = \frac{1}{1 + \cos^2 a}$
$\cot a = \frac{1 - \sqrt{2}}{\sqrt{2} \operatorname{vs} a}$	
cot a 1 2 cvs a	
$\cot a = \frac{1}{\tan a}.$	cosec a VI + tan*a tan a
$\cot a = \frac{1}{1 \sin^2 a}$	$-1 \qquad \begin{array}{ c c c c c c c c c c c c c c c c c c c$
cot a = 1 cosec 2	a = 1. cosec a see a 1 see $a = 1$

85. Functions of Negative Arcs.

We first find the sine and co-sine of — a, in terms of the functions of a from the diagram. Then, dividing the sine by the co-sine, the cosine by the sine, taking the reciprocal of the co-sine and the reciprocal of the sine, we have



$$\sin (-a)$$
 $\sin a$, $\cos (-a)$ $\cos a$,
 $\tan (-a) - \tan a$, $\cot (-a)$ $-\cot a$,
 $\sec (-a)$ $\sec a$, $\csc (-a)$ $-\csc a$

86. Functions of $(n 90^{\circ} \mp a)$.

1. Let n be 1 and a be negative.

From the figure of the last article, and by similar

$$\sin (90^{\circ} - a) = \cos a$$
, $\cos (90^{\circ} - a) = \sin a$,
 $\tan (90^{\circ} - a) = \cot a$, $\cot (90^{\circ} - a) = \tan a$,
 $\sec (90^{\circ} - a) = \csc a$, $\csc (90^{\circ} - a) = \sec a$.

These relations have already been found, articles 89-46.

2. Let n be 1 and a be positive.

$$\sin (90^{\circ} + a) = \cos a$$
, $\cos (90^{\circ} + a) = -\sin a$, $\tan (90^{\circ} + a) = -\cot a$, $\cot (90^{\circ} + a) = -\tan a$, $\sec (90^{\circ} + a) = -\cot a$, $\csc (90^{\circ} + a) = -\cot a$

3. Let n be 2, and a be negative.

$$\sin (180^{\circ} - a) = \sin a$$
, $\cos (180^{\circ} - a) = -\cos a$,
 $\tan (180^{\circ} - a) = -\tan a$, $\cot (180^{\circ} - a) = -\cot a$,
 $\sec (180^{\circ} - a) = -\sec a$, $\csc (180^{\circ} - a) = -\cot a$

4. Let n be 2, and a be positive.

$$\sin (180^{\circ} + a) = -\sin a$$
, $\cos (180^{\circ} + a) = -\cos a$,
 $\tan (180^{\circ} + a) = \tan a$, $\cot (180^{\circ} + a) = \cot a$,
 $\sec (180^{\circ} + a) = -\sec a$, $\csc (180^{\circ} + a) = -\csc a$.

5. Let n be 3, and a be negative.

$$\sin (270^{\circ}-a)$$
 · $\cos a$, $\cos (270^{\circ}-a) = -\sin a$.
 $\tan (270^{\circ}-a)$ · $\cot a$, $\cot (270^{\circ}-a) = \tan a$,
 $\sec (270^{\circ}-a)$ · $-\csc a$, $\csc (270^{\circ}-a) = -\sec a$.

6. Let n be 3, and a be positive.

$$\sin (270^{\circ} + a) = -\cos a$$
, $\cos (270^{\circ} + a) = \sin a$, $\tan (270^{\circ} + a) = \cot a$, $\cot (270^{\circ} + a) = \tan a$, $\sec (270^{\circ} + a) = \csc a$, $\csc (270^{\circ} + a) = -\sec a$.

7. Let n be 4, and a be negative.

$$\sin (360^{\circ} - a) = -\sin a$$
, $\cos (360^{\circ} - a) = \cos a$,
 $\tan (360^{\circ} - a) = -\tan a$, $\cot (360^{\circ} - a) = -\cot a$,
 $\sec (360^{\circ} - a) = \sec a$, $\csc (360^{\circ} - a) = -\cot a$,

8. Let n be 4, and a be positive.

sec
$$(360^{\circ} + a) = \sin a$$
, $\cos (360^{\circ} + a) = \cos a$, $\tan 360^{\circ} + a) = \tan a$, $\cot (360^{\circ} + a) = \cot a$, $\cot (360^{\circ} + a) = \cot a$, $\cot (360^{\circ} + a) = \cot a$,

It will be observed that when n is even, the functions in the two members of the equations have the same name; and that when n is odd, they have contrary names. The algebraic sign attributed to the second member is determined by the quadrant in which the are is situated.

Let this article be reviewed, and these principles applied in determining the names and algebraic signs of the second members.

Hence, functions of ares greater than 90° can be found in terms of functions of ares less than 90°. Thus,

- 1 sin 12^{10} sin $(90^{\circ} + 30^{\circ}) = \cos 30^{\circ}$.
- 2. ew 200° ers (270° + 20°) sin 20°,
- 3. tan 165° tan (180° -- 15°) -- tan 15°.

It's is integral and positive, prove the following:

- 4. $\sin \left[a \, 180^{\circ} + (-1)^{\circ} \, a \right] = \sin a$.
- 5 cos (n 360° ± a) cos a,
- ti, tan (n 180° + a) : tan a.
- Any function of $(n 360^{\circ} + a)$ the same function of a, whatever be the value of a.

87. Values of Functions of Particular Arcs.

1. To find the functions of 30°.

Since 60° is our-sixth of the circumference, the chord of 60° is equal to one side of a regular inscribed hexarm, which is equal to the radius or 1. But the sine of 30° is equal to one-half the chord of 60°.

... (1) $\sin 30^{\circ}$ ½ ... (2) $\cos 30^{\circ} = \sqrt{1 - \frac{1}{4}} = \frac{1}{4}\sqrt{3}$

Dividing (1) by (2), then (2) by (1), taking the reciprocals of (2) and (1), we have

- (3) $\tan 30^{\circ} = \frac{1}{1-\tilde{3}}$, (4) $\cot 30^{\circ} = 1-\tilde{3}$.
- (5) see $30^{\circ} = \frac{2}{1 \cdot 3}$, (6) cosec $30^{\circ} \cdot 2$.

2. To find the functions of 60°.

From article 40, sin 60° = sin (90° - 30°) = cos 30°, cos 60° -- $\cos (90^{\circ} - 30^{\circ}) = \sin 30^{\circ}$. Hence,

(1)
$$\sin 60^\circ - \frac{1}{2} \cdot 1 \cdot 3$$
, (2) $\cos 60^\circ = \frac{1}{2}$,

(3)
$$\tan 60^{\circ} - 1^{\prime} \overline{3}$$
, (4) $\cot 60^{\circ} \frac{1}{1 \overline{3}}$,

(5)
$$\sec 60^\circ = 2$$
, (6) $\csc 60^\circ = \frac{2}{1-3}$.

3. To find the functions of 45°.

From Art. 40, $\sin 45^\circ = \sin (90^\circ - 45^\circ) = \cos 45^\circ$; but $\sin^2 45^\circ + \cos^2 45^\circ = 1$,

...
$$2 \sin^2 45^\circ = 1$$
, ... $\sin^2 45^\circ = \frac{1}{2}$. Hence,

- (1) $\sin 45^\circ = \frac{1}{2}\sqrt{2}$, (2) $\cos 45^\circ = \frac{1}{2}\sqrt{2}$,
- (3) $\tan 45^{\circ} = 1$, (4) $\cot 45^{\circ} = 1$,
- (5) $\sec 45^{\circ} = V_{2}$ (6) $\csc 45^{\circ} = V_{2}$

Prove the following:

- 1. $\sec 120^\circ = -2$. | 5. $\csc 210^\circ = -2$.
- 2. $\cos 135^\circ = -\frac{1}{2}\sqrt{2}$. 6. $\cot 240^\circ = \frac{1}{1 \cdot 3}$.
- 3. $\sin 300^\circ = -\frac{1}{4} \cdot \overline{3}$. 7. $\sin 300^\circ = \frac{1}{4}$.
- 4. $tan 225^\circ = 1$. 8. $cos (-120^\circ) = -1$.
- 9. Construct an angle whose tangent is -1.
- 10. Construct an angle whose sine is -- 1.
- 11. Find all the functions of 150°.

88. Inverse Trigonometric Functions.

If x sin a, then a is the angle or are whose sine is x, which is written a sun 1 x, and read a equals the are whose sine is x.

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It must not be supposed that "I is an exponent, and that san's this would be a grievous error.

Let the fellowing be read:

$$\sup_{x \in \mathbb{R}^{-1} x} \lim_{x \in \mathbb{R}^{-1} x} \sup_{x \in \mathbb{R}^{-1} x} \sup_{x$$

The above notation is not altogether arbitrary; for Let i r be any function of z, and let f[f(x)], or, more briefly, let $f^2(x)$ be the same function of f(x), which notation denotes, not the square of f(x), that is, not $[f \cap f]^2$, but that the same function is taken of f(x) as of z. Thus, if $f(z) = \sin x$, $f[f(x)] = \sin (\sin x)$, then, in general,

(1)
$$f^n f^n(x) - f^{n+n}(x)$$
.

If n 0, (1) becomes,

(2)
$$f^{\alpha} f^{\delta}(x) = f^{\alpha}(x)$$
.

... (3)
$$f^{\circ}(x) = x$$
,

If m-1, and n=-1, (1) becomes,

(4)
$$ff^{-1}(x) = f^{\bullet}(x) = x$$

Hence, $f^{-1}(x)$ denotes a quantity whose like funct - 11 18 Z

Hence, if $y = \sin^{-1}x$, sin $y = \sin (\sin^{-1}x) = x$; that .w. y or sin 'z is an arc whose sine is z.

It would follow from the above that sin a ought to signify sin sain a), and not (sin a)2; but since we rarely have sin (sin a), it is customary to write sin2 a for sin or has we are thus saved the trouble of writing the parenthesis.

It would not, of course, do to write $\sin a^2$ for $(\sin a)^2$, for then we should have the sine of the square of an are for the square of the sine of an arc.

Let the following equations be proved:

1.
$$\sin^{-1}\frac{1}{2} = \cos^{-1}\frac{1}{2}$$
. 4. $\cos^{-1}\frac{1}{2} - 2\cot^{-1}1$ 3.

2.
$$\sin^{-1}\frac{1}{2} = \frac{1}{2}\tan^{-1}\sqrt{3}$$
. 5. $\sin^{-1}1 = 2\tan^{-1}1$.

3.
$$\tan^{-1} \sqrt{3} = \sec^{-1} 2$$
. 6. $\sec^{-1} 2 = \frac{1}{2} \sec^{-1} (-2)$.

89. Problem.

To find the sine and co-sine of the sum of two angles.

Let a = the angle OCA, and b = the angle ACB. Draw BL perpendicular to CA, BP and LM perpendicular to CO, and LN parallel to CO.

The triangles NBL and MCL are similar, since their sides are respectively

perpend dar; hence, the angle NBL of pesite the side NL equals the angle WCL opposite the homologous side ML. But MCL - a; hence NBL = a.

From the diagram we find the following relations:

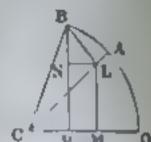
- (1) $LR = \sin b$.
- (2) $CL = \cos b$.
- (3) PB = ML + NB.
- (4) $PB \rightarrow \sin \theta CB = \sin (a + b)$.
- (5) $ML = \sin MCL \times CL = \sin a \cos b$
- (6) $NB = \cos NBL \times LB = \cos a \sin b$.

Substituting the values of PR, ML, and NR, found in (4), (5), and (6), in (3), and denoting the formula by (a), we have

(a) $\sin (a + b) = \sin a \cos b + \cos a \sin b$

Hence. The sine of the sum of two angles is equal to the me i the met is the orisine of the second, plus the consider if the first into the sine of the second.

From the diagram we find the follow-



- (1) CP CM NL
- $CP = \cos \theta CB = \cos (a + b).$
- (3) CM cos MCL × CL = cos a cos b.
- (4) $NL = \sin NBL \times LB = \sin a \sin b$.

Substituting the values of CP, CM, and NL, found in 2, 3, and (4), in (1), we have

(b) $\cos(a+b) = \cos a \cos b - \sin a \sin b$.

Hence, The co-sine of the sum of two angles is equal to the product of their co-sines minus the product of their sines.

90. Problems.

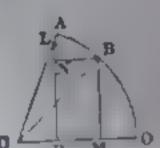
- 1. Prove that formulas (a) and (b) become homogeneous by introducing R.
- 2. Prove that formulas (a) and (b) are true when (a + b) is in the second quadrant.
- 3. Prove that formulas (a) and (b) are true when.
- 4. Prove that formulas (a) and (b) are true when a b, is in the fourth quadrant.
- 5 Deduce formula (b) from formula (a) by substituting 90° — a for a, and — b for b, and reducing by articles 85-86.
 - 6. Develop sin (45° + 30°) by formula (a).
 - 7 Develop cos 105° by formula (b).

91. Problem.

To find the sine and co-sine of the difference of two angles.

Let a = the angle OCA, and b = the angle BCA.

Draw BL perpendicular to CA, LP and BM perpendicular to CO, and BN parallel to CO.



The triangles NLB and PCL are sim- of the sides are respectively perpendicular; hence, the angle NLB, opposite the side NB, equals the angle PCL opposite the homologous side PL. But the angle PCL a, hence, the angle NLB = a. Then we shall have

- (1) $LB = \sin b$.
- (2) $CL = \cos b$.
- (3) MB = PL NL.
- (4) $MB = \sin OCB = \sin (a b)$.
- (5) $PL = \sin PCL \times CL = \sin a \cos b$.
- (6) $NL = \cos NLB \times LB = \cos a \sin b$.

Substituting the values of MB, PL, and NL, found in (4), (5), and (6), in (3), we have

(c) $\sin (a-b) = \sin a \cos b - \cos a \sin b$.

Hence, The sine of the difference of two angles is equal to the sine of the first into the co-sine of the second, minus the co-sine of the first into the sine of the second.

From the diagram we find the following relations:

- (1) CM = CP + NB.
- (2) $CM = \cos OCB = \cos (a b)$.
- (3) $CP = \cos PCL \times CL = \cos a \cos b$.
- (4) $NB = \sin NLB \times LB \Longrightarrow \sin a \sin b$

Substituting in to the values of CM, CP, and NB i vitia (2), (1), and (4), we have

$$i \cos (1-b) \cos a \cos b + \sin a \sin b$$
.

Hone, The cosmic of the difference of two angles is equal to a product of their sines.

92. Problems.

I Prove that formulas (c) and (d) become homogeneous by introducing R.

- 2. Its like formulas (c) and (d) from (a) and (b), respectively, by substituting b for b_i and reducing by article 85.
- Rowe that formulas (c) and (d) are true when
 - Prove that formulas (c) and (d) are true when to be us in the third quadrant.
 - 5 Prove that formulas (c) and (d) are true when b) is in the fourth quadrant.

93. Problem.

To find the tangent and co-tangent of the sum or differ-

Dividing (a) by (b, we have

$$\frac{\sin (a+b)}{\cos (a+b)} = \frac{\sin a \cos b + \cos a \sin b}{\cos a \cos b + \sin a \sin b}.$$

Dividing both terms of the fraction in the second member by cos a cos b, reducing, and recollecting that

the sine of an are divided by its co-sine is equal to its tangent, we have

(c)
$$\tan (a + b) = \frac{\tan a + \tan b}{1 - \tan a \tan b}$$
.

Hence, The tangent of the sum of two angles is equal to the num of their tangents, divided by 1 minus the product of their tangents.

Dividing (b) by (a), and reducing, we have

(f)
$$\cot (a+b) = \frac{\cot a \cot b - 1}{\cot a + \cot b}$$
.

Hence, The co-tangent of the sum of two angles is equal to the product of their co-tangents, minus 1, divided by the sum of their co-tangents.

Dividing (c) by (d), and reducing, we have

(g)
$$\tan (a-b) = \frac{\tan a - \tan b}{1 + \tan a \tan b}$$

Hence, The tangent of the difference of two angles is equal to the tangent of the first minus the tangent of the second, divided by 1 plus the product of their tangents.

Dividing (d) by (c), and reducing, we have

(h) cot (a b)
$$\frac{\cot a \cot b + 1}{\cot b - \cot a}$$

Hence, The co-tangent of the difference of two angles is equal to the product of their co-tangents, plus 1, divided by the co-tangent of the second, minus the co-tangent of the first.

94. Problems.

1. Prove that (e), (f), (g), (h) become homogeneous by introducing R.

- 2. Deduce (g) from (c) by substituting b for b.
- 3. Deslace (b) from (f) by substituting b for b.
- 4. Deduce (f) from (c) by taking the reciprocal of each member, substituting $\frac{1}{\cot a}$ for $\tan a$, $\frac{1}{\cot b}$ for $\tan b$, and reducing.
 - 5. Desluce, in like manner, (h) from (g).
- 6. Find the value of $\sin (a + b + c)$ by substituting b + c for b in (a).
- 7. Find the value of cos (a+b+c) by substituting b+c for b in (b).
- Find the value of tan (a+b+c) by substituting b+c for b in (c).
- 9. Find the value of cot (a+b+c) by substituting b+c for b in (f).

95. Functions of Double and Half Angles.

Making b = a in (a), (b), (e), and (f), we have

- (1) $\sin 2a = 2 \sin a \cos a$.
- (2) $\cos 2a \cos^2 a \sin^2 a$,
- (3) $\tan 2a = \frac{2 \tan a}{1 \tan^2 a}$
- (4) $\cot 2a \frac{\cot^2 a 1}{2 \cot a}$.

Substituting $\frac{1}{2}$ a for a in (1), (2), (3), (4), we have

- (5) $\sin a = 2 \sin \frac{1}{2} a \cos \frac{1}{3} a$.
- (6) $\cos a = \cos^2 \frac{1}{2}a \sin^2 \frac{1}{2}a$.

- (7) $\tan a = \frac{2 \tan \frac{1}{2} a}{1 \tan^2 \frac{1}{2} a}$
- (8) $\cot a = \frac{\cot^2 \frac{1}{2}a 1}{2 \cot \frac{1}{2}a}$

Substituting $1 - \sin^2 \frac{1}{2}a$ for $\cos^2 \frac{1}{2}a$, then $1 - \cos^2 \frac{1}{2}a$ for $\sin^2 \frac{1}{2}a$, in (6), and reducing, we have

- (9) $1 \cos a = 2 \sin^2 \frac{1}{2} a$.
- (10) $1 + \cos a = 2 \cos^2 \frac{1}{2}a$.
- ... (11) $\sin \frac{1}{2}a = \sqrt{\frac{1 \cos a}{2}}$.
- ... (12) $\cos \frac{1}{2}a = \sqrt{\frac{1+\cos a}{2}}$.

Dividing (11) by (12, then (12) by (11), we have

- (13) $\tan \frac{\pi}{a} = \sqrt{\frac{1 \cos a}{1 + \cos a}}$.
- (14) $\cot \frac{1}{2} a = \sqrt{\frac{1 + \cos a}{1 \cos a}}$.

Dividing (5) first by (10), then by (9), and transposing, we have

- $(15) \quad \tan \frac{1}{2}a = \frac{\sin a}{1 + \cos a}.$
- (16) cot 4 a sin a 1 cos a

Taking the reciprocal of (16), then of (15), we have

- (17) $\tan \frac{1}{2}a = \frac{1-\cos a}{\sin a}$.
- (18) $\cot \frac{1}{2} a = \frac{1 + \cos a}{\sin a}$.

Lot the formulas of this article be expressed in words.

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96. Consequences of (a), (b), (c), (d),

Takar the sum and difference of (a) and (c), (d) and ', we have

- (1) $\sin (a + b) + \sin (a b) = 2 \sin a \cos b$.
- $2 \sin (a+b) \sin (a-b) = 2 \cos a \sin b.$
- (i) $\cos(a+b) + \cos(a-b) = 2 \cos a \cos b$.
- $(1 + \cos(a b) \cos(a + b) 2 \sin a \sin b.$

Let
$$\begin{cases} a+b-s, \\ a-b-d, \end{cases}$$
 then $\begin{cases} a-\frac{1}{2}(s+d), \\ b-\frac{1}{2}(s-d), \end{cases}$

Substituting the values of a + b, a - b, a, and b, in (1), (2), (3), and (4), we have

- (5) $\sin s + \sin d = 2 \sin \frac{1}{2} (s + d + -\frac{1}{2} (s d))$.
- (6) sin s sin d 2 cos 1 (s + d m 1 (s d).
- (7) cos s + cos d 2 cos 1 (s + d) c = 1 (s d).
- $cos d cos s + 2 sin \frac{1}{2} (s + d) = \frac{1}{2} (s d)$.

By formula 5) of the preceding artile we have

- $\sin (s + d) 2 \sin \frac{\pi}{2} (s + d) \cos \frac{\pi}{2} + d$.
- (10) $\sin (s d) 2 \sin \frac{1}{2} (s d) \cos \frac{1}{2} (s d)$.

Dividing each of these formulas by each of the following, we have

- 11) $\frac{\sin s + \sin d}{\sin s + \sin d} = \frac{\sin \frac{1}{2}(s+d)\cos \frac{1}{2}(s-d)}{\sin s + \sin d} = \frac{\sin \frac{1}{2}(s+d)\cos \frac{1}{2}(s-d)}{\sin \frac{1}{2}(s-d)} = \frac{\tan \frac{1}{2}(s+d)}{\tan \frac{1}{2}(s-d)}$
- 12 $\frac{\sin s + \sin d}{\cos s + \cos d} = \frac{\sin \frac{1}{2}(s+d)}{\cos \frac{1}{2}(s+d)} = \tan \frac{1}{2}(s+d)$.
- one $d = \cos d = \cos \frac{1}{2}(n-d) = \cot \frac{1}{2}(n-d)$.
- (14) $\sin s = \sin d = \frac{\cos \frac{1}{2}(s-d)}{\cos \frac{1}{2}(s+d)}$.

(15)
$$\frac{\sin s + \sin d}{\sin (s - d)} = \frac{\sin \frac{1}{2}(s + d)}{\sin \frac{1}{2}(s - d)}.$$

(16)
$$\frac{\sin s - \sin d}{\cos s + \cos d} - \frac{\sin \frac{1}{2}(s - d)}{\cos \frac{1}{2}(s - d)} - \tan \frac{1}{2}(s - d).$$

(17)
$$\frac{\sin s - \sin d}{\cos d - \cos s} = \frac{\cos \frac{1}{2}(s+d)}{\sin \frac{1}{2}(s+d)} - \cot \frac{1}{2}(s+d).$$

(18)
$$\frac{\sin s - \sin d}{\sin (s + d)} = \frac{\sin \frac{1}{2}(s - d)}{\sin \frac{1}{2}(s + d)}.$$

(19)
$$\sin s - \sin d = \cos \frac{1}{2} (s + d)$$

 $\sin (s - d) = \cos \frac{1}{2} (s - d)$

(20)
$$\frac{\cos s + \cos d}{\cos d - \cos s} = \frac{\cot \frac{1}{2}(s+d)}{\tan \frac{1}{2}(s-d)}.$$

(21)
$$\frac{\cos s + \cos d}{\sin (s + d)} + \frac{\cos \frac{1}{2} (s - d)}{\sin \frac{1}{2} (s + d)}.$$

(22)
$$\frac{\cos s}{\sin s} \frac{\cos d}{ds} \frac{\cos \frac{1}{2}(s+d)}{\sin \frac{1}{2}(s-d)}$$

(23)
$$\frac{\cos d - \cos s}{\sin (s + d)} = \frac{\sin \frac{\pi}{2} (s - d)}{\cos \frac{\pi}{2} (s + d)}.$$

(24)
$$\frac{\cos d - \cos s}{\sin \left(s - d\right)} \frac{\sin \frac{1}{2} \left(s + d\right)}{\cos \frac{1}{2} \left(s - d\right)}.$$

(25)
$$\frac{\sin((a+d))}{\sin^{\frac{1}{2}}(a+d)} \frac{\sin(\frac{1}{2}(a+d))\cos(\frac{1}{2}(a+d))}{\sin(\frac{1}{2}(a+d))\cos(\frac{1}{2}(a+d))}$$

Formula (11) gives the proportion,

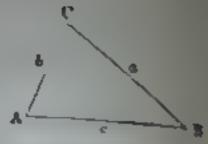
 $\sin s + \sin d : \sin s - \sin d : : \tan \frac{1}{2}(s + d) : \tan \frac{1}{2}(s - d)$

Hence, The sum of the sines of two angles is to their difference as the tangent of one half the som of the angles is to the tangent of one-half their difference.

Let us apply this principle in solving triangles when two sides and their included angle are given Article 75.

a : b : sin A : sin B.

$$a:b:\sin A:\sin B$$
.
 $a:b:a-b:\sin A+\cos B$.
 $\sin B:\sin A:\sin B$.
 $a:A+\sin B:\sin A=\sin B$: $\tan A(A+B):\tanh A$



 $\sin A + \sin B$: $\sin A - \sin B$:: $\tan \frac{1}{2}(A+B)$: $\tan \frac{1}{2}(A-B)$ $a + b : a - b :: \tan \frac{1}{2}(A + B) : \tan \frac{1}{2}(A - B)$.

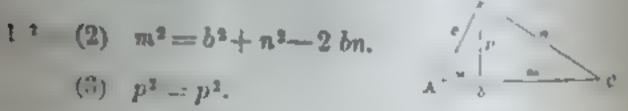
97. Theorem.

The square of any side of a triangle is equal to the mon of the separces of the other sides, minus trene their product rate the cosaine of their included angle.

1st. When the angle is acute.

(1)
$$m=b-n$$
.

$$1^{-2}$$
 (2) $m^2 = b^2 + n^2 - 2bn$.



(3)
$$p^2 = p^2$$
.

2 + 3, (4) $m^2 + p^2 = b^2 + n^2 + p^2 - 2bn$.

Eut $m^2 + p^2 = a^2$ and $n^2 + p^2 = c^2$, ... (4) becomes

(5)
$$a^3 = b^2 + c^2 - 2 bn$$

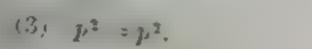
But n = c cos A, which substituted in (5) gives

(6)
$$a^2 = b^2 + c^2 - 2bc \cos A$$

2d. When the angle is obtuse.

(1)
$$m = b + n$$
.

$$(1 = -12)$$
 $m^2 = b^2 + n^2 + 2 bn$





(2) + (3, = .4)
$$m^2 + p^2 = b^2 + n^2 + p^2 + 2bn$$
.

But $m^2 + p^2 = a^2$ and $n^2 + p^2 = c^2$, ... (4) becomes

(5)
$$a^2 = b^2 + c^2 + 2 bn$$
.

But $n = c \cos BAD = -c \cos BAC = -c \cos A$.

... (6)
$$a^2 = b^2 + c^2 - 2bc \cos A$$
.

98. Problem.

To find the angles of a triangle when the wiles are given.

From either formula (6) of the last article we have

(1)
$$\cos A = \frac{b^2 + c^2}{2bc} = \frac{a^2}{2bc}$$
.

Honce, The co-sine of any angle of a triangle is equal to the sum of the squares of the adjacent sides, minus the square of the apposate side, durided by twice the rectangle of the adjacent sides.

Formula (1) gives the natural co-sine of A; hence, A can be found. But it is best to place the formula under such a form as to adapt it to logarithmic computation.

Adding 1 to both members of (1) we have

(2)
$$1 + \cos A = \frac{(b + c^{-2} - a^2)}{2bc} = \frac{(a + b + c)(b + c - a)}{2bc}$$
.

But $1 + \cos A = 2 \cos^2 \frac{1}{4} A$. Article 95, (10).

Let
$$a+b+c=p_c$$
 then $\frac{(a+b+c)(b+c+a)-p(p+2a)}{2bc}$.

Substituting these values in (2), and dividing by 2. we have

(3)
$$\cos^2 \frac{1}{2} A = \frac{\frac{1}{2} p \left(\frac{1}{2} p - a \right)}{bc}$$

$$V(3) = (4) \cos \frac{1}{2} A = \sqrt{\frac{\frac{1}{2}p(\frac{1}{2}p-a)}{bc}}$$

In like manner, (i) $\cos \frac{1}{2}B = \sqrt{\frac{1}{2}p + \frac{1}{2}p + \overline{b}}$.

Also, (6)
$$\cos \frac{1}{2}C = \sqrt{\frac{\frac{1}{2}P}{\frac{1}{2}P} + c}$$
.

Introducing R, applying logarithms, and reducing 4 becomes

 $1 \cdot z \circ w$ $\frac{1}{2} \cdot \frac{1}{2} \cdot \frac$

In like manner introduce R and apply logarithms to 5 and (6),

By subtracting both members of (1) from 1 and reducing we find

(7)
$$\sin \frac{1}{2}A = \sqrt{(\frac{1}{2}P^{-1})^{-1}P^{-1}C}$$
.

(8)
$$\sin \frac{1}{2}B = \sqrt{(\frac{1}{2}p + a + c) + c}$$
.

(9)
$$\sin \frac{1}{2}C = \sqrt{\frac{(\frac{1}{2}P - a_{ab} + b)}{ab}}$$

$$\sqrt{7} = (4) - (10) \quad \tan \frac{1}{2} A = \sqrt{\frac{(\frac{1}{2}p - b)(\frac{1}{2}p - c)}{\frac{1}{2}p(\frac{1}{2}p - a)}}.$$

$$(5) + (5) = (11) \tan \frac{1}{2}B = \sqrt{\frac{(\frac{1}{2}p - a^{-1})}{\frac{1}{2}p - \frac{1}{2}p^{-1}}}$$

$$(9) + (6) = (12) - \tan \frac{1}{2} C - \sqrt{\frac{3}{2}} \frac{p - \sigma}{2} (\frac{3}{2} \frac{p}{p - \epsilon} - b).$$

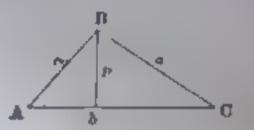
99. Examples.

1. Given
$$\begin{cases} a = 125 \\ b = 150 \end{cases}$$
 Required
$$\begin{cases} A = 55^{\circ} \ 46' \ 18'' \\ B = 82^{\circ} \ 49' \ 08'' \\ C = 41^{\circ} \ 24' \ 34'' \end{cases}$$
2. Given
$$\begin{cases} a = 864, \\ b = 1308 \\ c = 1086, \end{cases}$$
 Required
$$\begin{cases} A = 55^{\circ} \ 46' \ 18'' \\ B = 82^{\circ} \ 49' \ 08'' \\ C = 55^{\circ} \ 34' \ 08'' \end{cases}$$

100. Problem.

To find the area of a triangle when two sides and their included angle are giren.

Let k denote the area of the trib and c and their included angle A



(1) 2 k = bp.

(2) $p = c \sin A$.

... (3) $2 k = bc \sin A$.

Introducing R, and applying logarithms, we have

$$\log (2 k) = \log b + \log c + \log \sin A - 10.$$

101. Examples.

- 1. Two sides of a triangle are 345.6 and 485, respectively, and their included angle is 38° 45' 40"; what is the area? Ans. 52468.
- 2. Two sides of a triangle are 784.25 and 1095.8, respectively, and their in heled angle is \$5° 40' 20"; what is the area. Ans. 428470.

102. Problem.

To find the area of a tria agle when the three sides are given.

By the last problem we find

(1) $k = \frac{1}{2}bc \sin A$,

(2) sin A = 2 sin \(\frac{1}{2}\) A cos \(\frac{1}{2}\) A rticle \(\Omega\)5, (5),

(3) $\sin \frac{1}{2}A = (\frac{1}{2}p + b)(\frac{1}{2}p + c)$. Article 98, (7),

(4)
$$\cos \frac{1}{3}A = \sqrt{\frac{\frac{1}{2}p(\frac{1}{2}p-a)}{bc}}$$
. Article 98, (4).

5) $\sin A = 2 \frac{1}{2} \frac{\frac{1}{2}p(\frac{1}{2}p-a)}{bc} (\frac{1}{2}p-b) (\frac{1}{2}p-c)$.

(6) $k = 1 \frac{1}{2}p(\frac{1}{2}p-a) (\frac{1}{2}p-b) (\frac{1}{2}p-c)$.

103. Examples.

1 The sides of a triangle are 40, 45, 55, required the area.

Ans. 887.412.

2. The sides of a triangle are 467, 845, 756, required the area.

Ans. 175508.

104. Problem.

Given the perimeter and angles of a triangle, required the sudes.

(1)
$$\frac{b}{a} = \frac{\sin B}{\sin A}$$
. (2) $\frac{c}{a} = \frac{\sin C}{\sin A}$.

Adding and reducing by Articles 96, (5) and 95, (5), we have

(3)
$$\frac{b+c}{a} = \frac{\sin \frac{1}{2}(B+C)\cos \frac{1}{2}(B-C)}{\sin \frac{1}{2}A\cos \frac{1}{2}A}$$
.
 $\sin \frac{1}{2}(B+C) = \cos \frac{1}{2}A$, and $\sin \frac{1}{2}A = \cos \frac{1}{2}(B+C)$.

... (3)
$$\frac{b+c}{a} = \frac{\cos \frac{1}{2}(B-C)}{\cos \frac{1}{2}(B+C)}$$
.

Adding 1 to both members, we have

(4)
$$\frac{a+b+c}{a} = \frac{\cos \frac{1}{2}(B+C) + \cos \frac{1}{2}(B-C)}{\cos \frac{1}{2}(B+C)}$$

Let p=a+b+c, and reduce by 96, (7), we have

(5)
$$\frac{P}{a} = \frac{2 \cos \frac{1}{2} R \cos \frac{1}{2} C}{\sin \frac{1}{2} A}$$

Introducing R and applying logarithms, we have

$$\log a = \log \frac{1}{2}p + \log \sin \frac{1}{2}A + a.c. \log \cos \frac{1}{2}C - 10.$$

Similar formulas can be found for b and c. But, after a is found, b and c can be more readily found by article 69.

105. Examples.

1. Given p = 150, $A = 70^{\circ}$, $B = 60^{\circ}$, $C = 50^{\circ}$, required a, b, c.

Ans.
$$a = 54.81$$
, $b = 50.51$, $c = 44.68$.

2. Given p = 31234.36, $A = 35^{\circ} 45'$, $B = 45^{\circ} 28'$, $C = 98^{\circ} 47'$, required a, b, c.

Ans. a = 7985, b = 9742.5, c = 13506.86.

3. Given p = 375, $A = 55^{\circ}$ 46' 18", $B = 82^{\circ}$ 49' 08", $C = 41^{\circ}$ 24' 34", required a, b, c.

Ans. a = 125, b = 150, c = 100.

106. Problem.

Given the three sules of a triangle, to find the radius of the inscribed circle.

(1)
$$BOC + AOC + AOB = ABC$$
.

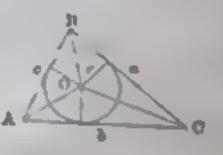
(2) $BOC = \frac{1}{2} ar$.

(3) $AOC = \frac{1}{4}br$.

(4) AOB = \frac{1}{2} cr.

.. (5) $BOC + AOC + AOB = \frac{1}{2}(a+b+c)r = \frac{1}{2}pr$.

But (6) ABC 1 \(\frac{1}{2}\vert \frac{1}{2}\vert p \cdot \alpha \) (\(\frac{1}{2}\vert p \cdot \alpha \cdot \alpha \) (\(\frac{1}{2}\vert p \cdot \alpha \cdot \alph



.. (7)
$$\frac{3}{2}p^{2} = \frac{1}{3}\frac{3}{2}p + \frac{1}{3}p + \frac{1}{6}(\frac{1}{2}p + \frac{7}{6})$$
.

$$\therefore \qquad \qquad \frac{3p-a}{3p}\frac{(3p-b)(3p-c)}{3p} - \frac{k}{3p}.$$

107. Examples.

 The three sides of a triangle are 20, 30, 40, respectively, required the radius of the inscribed circle. Ana, 6.455.

2 The three sides of a triangle are 100, 150, 200, respectively, required the radius of the inscribed circle. Ann. 32 275.

108. Problem.

Given the three sides of a triangle to find the radius of the circumsershal circle

Let O be the center of the circle,

and R the radius.

Let OD be perpendicular to b, then

AD . 5-.

The angle O = the angle B, since each is measured by one-half the arc AC.

(1)
$$AD = \frac{b}{2} = AO \sin O = R \sin B$$
.

... (2)
$$R = \frac{b}{2 \sin B}$$
.

 $\sin B = 2 \sin \frac{1}{2} B \cos \frac{1}{2} B = 2 \frac{1}{2} \frac{\frac{1}{2} p (\frac{1}{2} p - a - \frac{1}{2} p - b) (\frac{1}{2} p - c)}{a}$

.. (3)
$$R = \frac{abc}{4 \nu' \frac{1}{2} p (\frac{1}{2} p - a) (\frac{1}{2} p - b) (\frac{1}{2} p - c)} = \frac{abc}{4 k}$$

Prove that the formula will be the same if the center is without the triangle.

109. Examples.

1 The sides of a triangle are 7, 9, 10, respectively, required the radius of the circumscribed circle. Ans. 5.148.

2. The sides of a triangle are 50, 60, 70, respectively, required the radius of the circumscribed circle. Ans. 35.72.

110, Theorem.

The perpendicular let fall on either side of a triangle from the vertex of the opposite angle is equal to that side into the product of the sines of the adjacent angles divided by the · sine of the sum of those angles.

(1)
$$p = c \sin A$$
.

(2)
$$\sin B : \sin C :: b : c_1 ... c = \frac{b \sin C}{\sin B}$$

$$\therefore (3) \quad p = \frac{b \sin A \sin C}{\sin R}.$$

(4) $\sin B = \sin [180^{\circ} - (A+C)] = \frac{1}{8} \sin (A+C)$

... (5)
$$p = \frac{b \sin A \sin C}{\sin (A+C)}$$

111. Problem.

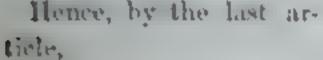
Given the three sides of a triangle to find the radii of the excribed circles.

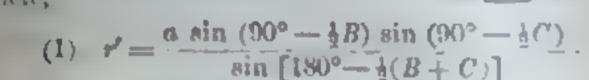
The escribed circles are the three circles external to the triangle, each tangent to one side and to the prolongation of the other sides.

The center of the escribed circles are the points of

intersection of the lines bisecting the external aughs.

The radii r', r'', r''', of the excibed circles, will be the perpendiculars let fall from their centers O', O'', O''', respectively, on the three sides a, b, c.





... (2)
$$r' = \frac{a \cos \frac{1}{2} B \cos \frac{1}{2} C}{\cos \frac{1}{2} A} = \frac{1}{2} p \tan \frac{1}{2} t$$
. Art. 104.

Substituting the value of tan \$A, article 98, we have

(3)
$$r = \sqrt{\frac{\frac{1}{2}p(\frac{1}{2}p - b)(\frac{1}{2}p - c)}{\frac{1}{2}p - a}} - \frac{k}{\frac{1}{2}p - a}.$$

... (4)
$$y'' = \sqrt{\frac{\frac{1}{2}p(\frac{1}{2}p-a)}{\frac{1}{2}p-b}} \frac{\frac{1}{2}p-c)}{\frac{1}{2}p-b} \frac{k}{\frac{1}{2}p-b}$$

.. (5)
$$r'' = \sqrt{\frac{\frac{1}{2}p(\frac{1}{2}p-a)(\frac{1}{2}p-b)}{\frac{1}{2}p-c}} = \frac{k}{\frac{1}{2}p-c}$$

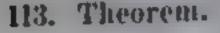
112. Examples.

* 1 Given the sides of a triangle, 6, 9, 11, required the radii of the three escribed circles

Ans. 3.854, 6.745, 13.49.

2. Given p=100, $A=55^{\circ}$, $B=60^{\circ}$, $C=65^{\circ}$, required the radii of the three escribed circles

[See (2), Art. 111.] Ann. 26.028, 28.867, 31.854.



The product of the radius of the inscribed circle and the radii of the three escribed circles is equal to the square of the area of the triangle

The product of (8), article 106, and (3), (4, (5), article 111, gives

$$rr'r''r''' = \frac{k^4}{\frac{1}{2}p\left(\frac{1}{2}p-a\right)\left(\frac{1}{2}p-b\right)\left(\frac{1}{2}p-c\right)} - \frac{k^4}{k^2} - k^3.$$

114. Theorem.

The reciprocal of the radius of the inscribed circle, the sum of the reciprocals of the radii of the escribed circles, and the sum of the racip scale of the perpendiculars let full from the returns of the three angles on the opposite sides of a triangle are equal to each other.

Taking the reciprocal of (8), article 106, we have

Taking the sum of the reciprocals of (3), (4), (5), article 111,

(2)
$$\frac{1}{r} + \frac{1}{r''} + \frac{1}{r'''} = \frac{p-2a}{2k} + \frac{p-2b}{2k} + \frac{p-2c}{2k} - \frac{p}{2k}$$

Let p', p'', p''', respectively, be the perpendiculars let fall from the vertices of the three angles on the sides a, b, and c. Then we have

$$a p' = 2 k$$
. $\cdot \cdot \cdot \frac{1}{p'} = \frac{a}{2k}$.

In like manner, $\frac{1}{p''} = \frac{b}{2k}$. Also, $\frac{1}{p'''} = \frac{c}{2k}$



CIRCULAR FUNCTIONS

103

$$\frac{1}{p} = \frac{1}{p'} + \frac{1}{p''} - \frac{a - b + c}{2k} + \frac{p}{2k}.$$

$$\therefore (1) \quad \frac{1}{r} \quad \frac{1}{r'} + \frac{1}{r''} + \frac{1}{r'''} - \frac{1}{p'} + \frac{1}{p'''} + \frac{1}{p'''}.$$

115. Problem.

To find the distance between the centers of the circum-

Let R and r be the radii, and P and O the centers of the circles, and let D = OP.

Draw PE perpendicular to AC. The AC angle APE = B, since each is meas-

used by one-half the arc AC_I but $PAE = 90^{\circ} - APE_I$. $PAE = 90^{\circ} - B$. $OAC = \frac{1}{2}A$. PAO - PAE = OAC.

•••
$$PAO = 90^{\circ} - B - \frac{1}{2}A = \frac{1}{2}(C - B)$$
. $AO = \frac{r}{\sin \frac{1}{2}A}$

(1)
$$\overline{O}P^2 = AP^2 + \overline{AO}^2 - 2 AP \times AO \cos P.10$$
. Art. 97.

Substituting the values of OP, AP, AO, and PAO, we have

(2)
$$D^2 = R^2 + \frac{r^2}{\sin^2 \frac{1}{2}A} - \frac{2 Rr \cos \frac{1}{2}(C - B)}{\sin \frac{1}{2}A}$$

3)
$$R = \frac{b}{2 \sin B} - \frac{b}{4 \sin \frac{1}{2} B \cos \frac{1}{2} B}$$
. Arts. $\begin{cases} 108, (2) \\ 95, (5) \end{cases}$.

(4)
$$r = \frac{b \sin \frac{1}{2} A \sin \frac{1}{2} C}{\sin \frac{1}{2} A + C} = \frac{b \sin \frac{1}{2} A \sin \frac{1}{2} C}{\cos \frac{1}{2} B}$$
 Art. 110.

... (5)
$$\frac{r^2}{\sin^2 \frac{1}{2}A} = \frac{4 Rr \sin \frac{1}{2} B \sin \frac{1}{2} C}{\sin \frac{1}{2} A}$$
.

Substituting in (2), and reducing by article 91, (d), and 89, (b), we have

(6)
$$D^2 = R^2 - \frac{2 Rr \cos \frac{1}{2}(R + C)}{\sin \frac{1}{2}A} = R^2 - 2 Rr.$$

.. (7)
$$D = V R^2 - 2 Rr$$
.

116. Examples.

1. The sides of a triangle are 12, 13, 15; required the distance between the centers of the circumscribed and inscribed circles

Ans. 1 616.

2. Two sides of a triangle are 35 and 37, and their included angle is 50°; required the distance between the centers of the circumscribed and inscribed circles.

Ans. 3.266.

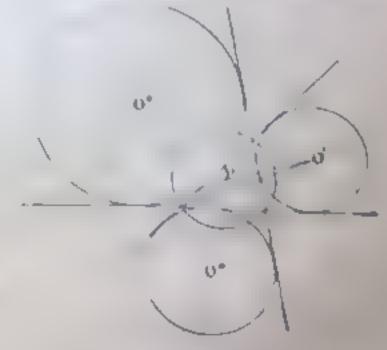
3. The perimeter of a triangle is 120, the angles are 40°, 60°, and 80°, respectively; required the distance between the centers of the circumscribed and inscribed circles.

Ans. 8.353.

117. Problem.

To find the distance between the centers of the circumscribed and excribed circles

Let r', r'', r''' be the radii of the escribed circles, and D', D'', D''', be the distances of their centers, O', O'', O''', respectively, from P, the center of the circumscribed circle, whose radius is R.



As in the last Problem, we find

$$(1-I^{r_2}-R^{\frac{r}{2}}+\frac{r'^{\frac{r}{2}}}{\sin^{\frac{r}{2}}\frac{1}{2}A}+\frac{2-Rr'\cos\frac{1}{2}(C+B)}{\sin\frac{1}{2}A}.$$

2
$$R = \frac{a}{2 \sin A} = \frac{a}{4 \sin \frac{1}{2} A \cos \frac{1}{2} A}$$
 Arts. $\begin{cases} 108, 02 \\ 95, 05 \end{cases}$

(3)
$$r' = \frac{\alpha \cos \frac{1}{2}B \cos \frac{1}{2}C}{\cos \frac{1}{2}A}$$
. Art. 111, (2).

.. if
$$\frac{r'^2}{\sin^2 \frac{1}{2}A} = \frac{4 R r' \cos \frac{1}{2} B \cos \frac{1}{2} C}{\sin \frac{1}{2} A}$$
, by (2) and (3).

Substituting (4) in (1), and reducing by (d) and (b), we have

(5)
$$D'^2 = R^2 + \frac{2 Rr' \cos \frac{1}{2}(B+C)}{\sin \frac{1}{2}A} = R^2 + 2 Rr'$$

... (6)
$$D' = 1/R^2 + 2 Rr'$$
.

.. (7)
$$D^{r}$$
. 1 $R^{2} \sim 2 R$.

... (8)
$$D'' = V \hat{R}^2 + 2 Rr''$$
.

118. Examples.

- 1. The three sides of a triangle are 21, 23, 26; required the distances from the center of the circumscribed circle to the centers of the three escribed circles. Ans. 25.19, 26.64, 29.73.
- 2. The angles of a triangle are 56°, 60°, 64°, the greatest side is 25; required the distances from the center of the circumscribed circle to the centers of the three escribed circles. Ans. 26.96, 27.80, 28.65.
- 3. Given p = 100, $A = 55^{\circ}$, $B = 60^{\circ}$, $C = 65^{\circ}$, required D', D', D', Ans. 37.10, 38.55, 40.01.

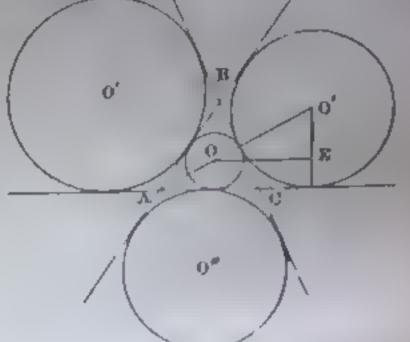
119. Problem.

To find the distance between the centers of the inscribed and carribed circles.

Let D_1 , D_2 , D_3 , be the distances.

In the triangle OO'E, we have

(1)
$$D_1 = \frac{r'-r}{\sin \frac{1}{2}.1}$$



Substituting the values of r', r, and sin 1/1, we have

(2)
$$D_1 = \frac{a}{\sqrt{\frac{\frac{1}{2}p\sqrt{\frac{1}{2}p}}{bc}} \frac{a}{a}}$$
.

(3) $D_2 = \frac{b}{\sqrt{\frac{\frac{1}{2}p\sqrt{\frac{1}{2}p}}{bc}} \frac{a}{a}}$.

(3)
$$D_2 = \frac{b}{\sqrt{\frac{3}{2}p(\frac{3}{2}p - b)}}$$

$$(4) \quad D_3 = \frac{e}{\sqrt{\frac{3}{2}p(\frac{3}{2}p - c)}}.$$

120. Examples.

- 1. The three sides of a triangle are 30, 50, 60; required the distances between the centers of the inseribed and escribed circles Am. 31,05, 56 69, 87 83,
- 2. The sides of a triangle are 500, 600, 700; required the sides of the triangle formed by joining the centers of the inscribed and circumscribed circles and the center of the escribed circle, tangent to the sides (11) and 700 produced. And. 540,08, 104,58, 624 58

121. Miscellaneous Exercises.

1 Prove that sm 15° $\frac{1}{2}$ $\frac{3-1}{2}$, cos 15° $\frac{1}{2}$ $\frac{3+1}{2}$ to 15° $\frac{1}{2}$ $\frac{3}{2}$ $\frac{1}{2}$ $\frac{3}{2}$ $\frac{1}{2}$ $\frac{1}$

2. Find the sine and co-sine of 75°

Ans. sin
$$75^\circ = \frac{1}{2} \cdot \frac{3+1}{2\sqrt{2}}$$
, cos $75^\circ = \frac{1}{2\sqrt{2}} \cdot \frac{3-1}{2\sqrt{2}}$.

- 3. Why is sin 75° = cos 15°, and cos 75° = sin 15°?
- 4. How may the values of tangent, co-tangent, so-cant, and co-secant of 75° be found from the values of the sine and co-sine?
 - 5. Find the functions of 150?,

Ana. sin 150' =
$$\frac{1}{2}$$
, cos 150° $-\frac{1}{2}$, ...

6. Given $\sin a + \cos a = \sqrt{2}$, to find a.

Ans. 45°, or 45° + 360°; or, in general,
$$\frac{\pi}{4}$$
 + 2 πn .

7. Given $\sin 2 a = \cos a$, to find a.

Ans.
$$\frac{\pi}{6} + 2 \pi n$$
, or $\frac{\pi}{6} \pi + 2 \pi n$.

- 8. Prove that the sum of the tangents of the three angles of a plane triangle is equal to their product.
- 9. Prove that the sum of the co-tangents of one-half the angles of a plane triangle is equal to their product.
 - 10. Prove that ABC is isosceles if $\cos A = \frac{\sin B}{2 \sin C}$.
- 11. Prove that the sum of the diameters of the inscribed and circumscribed circles of any plane triangle ABC is

a cot
$$A+b$$
 cot $B+c$ cot C .

12. If b is the base of the triangle ABC, p, the perpendicular to the base from the vertex of the opposite angle, and s, the sum of the sides a and c, prove that

$$\tan \frac{1}{2}B = \frac{2}{(s+b)} \frac{bp}{(s-b)}.$$

13. If b is the base of the triangle ABC, p, the perpendicular to the base from the vertex of the opposite angle, and d, the difference of the sides a and c, prove that

$$\tan \frac{1}{2}B = \frac{(h+d)(h-d)}{2bp}$$
.

14. If a, b, and c be the sides of the triangle ABC, the sum of the sides a and c, and r, the radius of the inscribed circle, prove that

$$\tan \frac{1}{2}B = \frac{2r}{s-b}.$$

122. Computation of Natural Functions.

Dividing the length of the semi-circumference to the adius 1, which is $\pi = 3.141592653589793...$ by 1080, the number of minutes in 180°, the quotient, which is .0002908882..., will be the length of the arc 1', and will differ insensibly from its sine.

- \therefore (1) $\sin 4' = .0002908882$.
- .. (2) $\cos 1' = \sqrt{1 \sin^2 1'} = .99999999577$.

Adding (a) and (c), then (b) and (d), articles 89, 91, and transposing,

- (3) $\sin (a+b) = 2 \sin a \cos b \sin (a-b)$.
- (4) $\cos (a+b) = 2 \cos a \cos b \cos (a-b)$,

If in (3) and (4) b=1, a=1,2,3,..., in succession, we have

$$\sin 2' = 2 \cos 1' \sin 1' + \sin 0' = .0005817764$$
, $\sin 3' = 2 \cos 1' \sin 2' + \sin 1' = .0008726646$, $\sin 4' = 2 \cos 1' \sin 3' + \sin 2' = .0011635526$, $\cos 2' = 2 \cos 1' \cos 1' + \cos 0' = .99999998308$, $\cos 3' = 2 \cos 1' \cos 2' + \cos 1' = .99999996193$.

To facilitate computation, for $2 \cos 1' = 1.9999999154$, use its equal, 2 - .0000000846. Then we have

$$\sin 2' = 2 \sin 1' - .0000000846 \sin 1' - \sin 0'.$$

 $\sin 3' = 2 \sin 2' - .0000000846 \sin 2' - \sin 1'.$

After finding the sines and cosmes the tangents and cosmes the tangents and cosmes the formulas:

(5)
$$\tan a = \frac{\sin a}{\cos a}$$
 (6) $\cot a = \frac{\cos a}{\sin a}$

It is not necessary to carry the computation beyond 45° , since $\sin a = \cos (90^{\circ} - a)$, etc.

The logarithmic functions can be found from the corresponding natural functions by the method of article 60.

SPHERICAL TRIGONOMETRY.

123. Definition and Remarks.

Spherical Trigonometry is that branch of Trigonometry which treats of the solution of spherical triangles

If any three of the six parts of a spherical triangle are given, the remaining parts can be computed.

The radius of the sphere is taken equal to 1, and

each side has the same numerical measure as the subtended angle whose vertex is at the center of the sphere. Thus,

$$a \cdot BOC, b = AOC, c = AOB.$$

An angle of a spherical triangle
is the angle included by the planes of its sides which
is measured by the angle included by two lines, one
line in one plane, the other in the other, both perpendicular to the common intersection of the planes
at the same point.

Thus, if BE, in the plane AOE, is perpendicular to OA, and if ED, in the plane AOC, is perpendicular to OA, then the angle BED will measure the inclination of the planes AOE and AOC, and will be equal to the angle A of the spherical triangle.

RIGHT TRIANGLES.

124. Napier's Circular Parts.

Mapier's circular parts to the two sides soljacent to the right angle, the complements of their opposite angles, and the complement of the hypotenuse.

Thus, if HBP is a spherical triangle, right-angled at H, the circular parts are h, p, $90^{\circ} - B$, $90^{\circ} - P$, and $90^{\circ} - h$.



Adjacent parts are those which

are not separated by an intervening circular part.

Thus, b and $90^{\circ} - P$, $90^{\circ} - P$ and $90^{\circ} - h$, $90^{\circ} - h$ and $90^{\circ} - B$, $90^{\circ} - B$ and p, p and b are adjacent parts.

The right angle H is not regarded as a circular part, nor as separating the parts b and p.

RIGHT TRLANGLES.

Opposite parts are those which are separated by an intervening circular part

Thus, b and 90° -- h, 10° -- P and 90° -B, 90° -- h and p. 90° B and b, p and 90° -P are opposite parts.

Any one of these five circular parts is adjacent to two of the remaining parts, and opposite the other two parts

Of any three circular parts, one part is either adjacent to both the others or opposite both.

A middle part is that which is adjust at to two other parts, or opposite two other parts.

125. Exercises.

Tell which is the middle part, and whether the other parts are adjacent to, or opposite, the middle in the fellowing :

1.
$$90^{\circ}-B_1 \ 90^{\circ}-P_1 \ 90^{\circ}-h_2$$
 | 6. $90^{\circ}-P_1 \ 90^{\circ}-h_1 \ p_2$ | 7. $b_1 \ 90^{\circ}-P_1 \ p_2$ | 8. $90^{\circ}-B_1 \ b_2$ | 9. $90^{\circ}-B_1 \ b_2$ | 10. $90^{\circ}-P_1 \ 90^{\circ}-B_1 \ p_2$

126. Napier's Principles.

1. The sine of the middle part is equal to the product of the tangents of the adjacent parts.

Draw BD and DE, respectively perpendicular to OH and OP, and draw RE. BDE is a right angle, since the plane BOH is perpendicular to the plane POH, and BD is perpendicular to OH. The angle BED is equal to P.

 $\sin h$, $OE = \cos h$, $DB = \sin p$, and $OD = \cos p$. $\frac{ED}{ER} \sim \frac{OE}{ER} \sim \frac{ED}{OE}$, or $\cos P = \cot h \tan b$.

... (1) $\sin (90^{\circ} - P) = \tan (90^{\circ} - h) \tan b$.

 $\frac{ED}{OD} = \frac{DB}{OD} \times \frac{ED}{DB}$, or sin b tan p cot P.

... (2) $\sin b = \tan p \tan (90^{\circ} - P)$.

By changing P, b, p into B, p, b, (1) and (2) become

(3) $\sin (90^{\circ} - B) = \tan (90^{\circ} - h) \tan p$.

 $\sin p = \tan b \tan (90^{\circ} - B)$. (4)

Multiplying (2) by (4), member by member, we have $\sin b \sin p = \tan b \tan p \tan (90^{\circ} - B) \tan (90^{\circ} - P)$.

Dividing by tan b tan p, and reducing, we have $\cos b \cos p \sim \tan (90^{\circ} - B) \tan (90^{\circ} - P)$.

 $\cos b \cos p = \cos EOD \times OD = OE - \cos h = \sin (90^{\circ} - h)$.

. (5) $\sin (90^{\circ} - h) = \tan (90^{\circ} - B) \tan (90^{\circ} - P)$.

2 The more of the modelle part a regard to a product of the co-sines of the opposite parts.

 $OE = \cos EOD \times OD$, or cos $h = \cos b \cos p$.

... (6) $\sin (90^{\circ} - h) = \cos b \cos p$.

 $DB = EB \sin DEB$, or $\sin p = \sin h \sin P$.

... (7) $\sin p = \cos (90^{\circ} - h) \cos (90^{\circ} - P)$,

(3) gives sin (90°-B) - sin (90'-h) sin p cos (90'-h) cos p

This, by substituting $\cos b \cos p$ for $\sin (90^{\circ}-h)$, $\cos (90^{\circ}-h) \cos (90^{\circ}-P)$ for $\sin p$, and reducing, gives

(8) $\sin (90^{\circ} - B) = \cos b \cos (90^{\circ} - P)$.

By changing p, P, B, b into b, B, P, p, (7) and a, become

- (9) $\sin b = \cos (90^{\circ} h) \cos (90^{\circ} B)$.
- (10) $\sin (90^{\circ} P) = \cos p \cos (90^{\circ} B)$.

These ten formulas are thus reduced to two principles, from which the formulas can be written.

The memory will be further aided by observing the common vowel a in the first syllables of the works tangent and adjacent of the first principle, and the common vowel o in the first syllables of the works co-sine and opposite of the second principle; that is we take the product of the tangents of the parts adjacent to the middle, and the product of the co-sine of the parts opposite the middle.

127. Mauduit's Principles.

If we take, as circular parts, the applements of the two sides adjacent to the right and their parts site angles, and the hypoteness we contradity deduce from the diagram, or from Napier possible, the following principles:

- of the co-tangents of the adjacent parts.
- 2. The co-sine of the modelle part in open to the product of the sines of the opposite parts

Let the ten formulas be written and compared with those of the last article.

128. Analogies of Plane and Spherical Triangle-

The formulas which demonstrate Napier's principles may be placed under forms which will exhibit the analogies existing between Plane and Spherical Traangles, as in the subjeited table

Place Right Trungles	Spherost II it True See.
1 $P = \frac{p}{h}$	1 with P with 5 to h
$B = \frac{b}{h}$	2 sin B 5 5
$P = \frac{h}{h}$	3 con P tan 5
1. vos B = P h	4 cce B 2 cr 2
to tan P P	5 ton P
to tra B	K tata B to a
7. sin P cos B.	7. sin P = con b
,	~ ~ 2 2 1 1 c c c y
,	D com & com b com g
1 1 1 2 2	101 com A cont B cont B
	1

-1 .1 .1 here process to the win and

the way I was a second the territory

8 N 10

RIGHT TRIANGLES.

129. Species of the Parts.

Two parts of a spherical triangle are of the same species when both are less than 90° or both greater than 90°.

Two parts of a spherical triangle are of different parts when one part is less than 90° and the other part greater than 90°.

We shall, at present, consider those triangles only whose parts do not exceed 180°.

Let it be remembered that the sine is positive from 0° to 180°, and that the co-sine, the tangent, and the co-tangent are positive from 0° to 100° and megative from 90° to 180°. Hence, if the co-sines, tangents, or co-tangents of two parts have like these parts will be of the same species; if they have allike signs, these parts will be of different species

$$\sin P = \frac{\cos B}{\cos b} \text{ and } \sin B = \frac{\cos P}{\cos p} \quad \text{(et. 128, 7, 8.)}$$

Since neither P nor B exceeds 180° , sin P and sin B are both positive; hence, con B and cos h have like signs, so also have cos P and cos p. Then forc, B and b are of the same species; so also are P and p.

Honco, The sides adjacent to the right angle are of the same species as their opposite angles.

con
$$h = \cos b \cos p$$
. Art. 128, 9

If $h < 90^{\circ}$, cos h is positive; hence, cos h cos p is positive; ... cos b and cos p have like signs; ... b and p are of the same species; ... B and P are of the same species.

Hence, If the hypotenuse is less than 100°, the two sole adjacent to the right angle are of the same species; so also are their opposite angles.

If $h > 90^\circ$, cos h is negative; honce, cos b cos p is negative; it cos b and cos p have unlike signs. . . b and p are of different species; it. B and P are of different species.

Hence, If the hopotenuse is greater than 1812, the to substant to the right angle are of defere to species, we also are there opposite a ights

Let us now investigate the case in which a side adjacent to the right angle and its opposite angle are given.

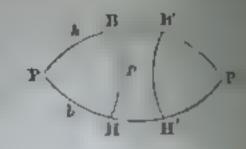


Let p and P be given. Produce the sides PH and PB till they meet in P'. The angles P and P' are equal, since each is the angle included by the plane of the arcs PHP' and PBP'. Take P'H' and PH' and PB''. Take P'H'' and PH'' and PH''' and PH'''' and PH'''' and PH''' and

Since P'H' and PH are equal, and PH' and PH' are supplements of each other, PH and PH' are supplements of each other. In like manner it be shown that PB and PB' are supplements of such other.

When, therefore, a side adjacent to the right and an opposite angle are given, there are apparent at two solutions. The conditions of the problem, have ever, may be such as to render the two series possible, reduce them to one, or render as a series impossible.

Let us now proceed to investigate these conditions



1. When $P < 90^{\circ}$ and p < P.

We have from Napier's principles,

 $\sin b = \tan p \tan (90^{\circ} - P)$, or $\sin b = \tan p \cot P$

Since $P < 90^{\circ}$ and p < P, tan $p < \tan P$; but we have $\tan P \cot P = 1$; ... $\tan p \cot P < 1$; hence, $\sin b < 1$; then $b < 90^{\circ}$ or $b > 90^{\circ}$; hence, b may be either of the supplementary arcs PH or PH' which have the same sine equal to tan p cot P.

If $b < 90^{\circ}$, since $p < 90^{\circ}$, $h < 90^{\circ}$; if $b > 90^{\circ}$. since $p < 90^{\circ}$, $h > 90^{\circ}$. Hence, if $P < 90^{\circ}$ and p < P, either triangle, PHB or PH'B', will satisfy the conditions, and there will be two solutions.

2. When $P < 90^{\circ}$ and p = P.



We have $\sin b = \tan p \cot P_1$ as before.

I: then-Since p = P, tun p cot $P = \tan P$ cot Pfore, $\sin b = 1$; $\therefore b = 90^{\circ}$, or PH = 90

From Napier's principles, we have

 $\sin (90^{\circ} - h) = \cos b \cos p_i$ or $\cos h = \cos b \cos p_i$

Since $b = 90^{\circ}$, $\cos b = 0$; ... $\cos b \cos p = 0$; hence, cos h = 0; ..., $h = 90^{\circ}$, or $PR = 90^{\circ}$.

 $\sin (90^{\circ}-B) = \tan p \tan (90^{\circ}-h)$, which reduces pcos $B = \tan p \cot h$.

Since $h = 90^\circ$, cot h = 0; A = 0.'. com B = 0; .'. B = 90°.

> PH 90°; . PH' = PH. 1115 1800 PB 90°, . . PB

Hence, if $P < 90^{\circ}$ and p = P, $h = 90^{\circ}$, $h = 40^{\circ}$, B - 200°, the two triangles reduce to the bere tangular triangle PHB, and there is but one solution

EIGHT TELANGLES.

When P < 90° and p > P

As before, we have sin b tan p out P. Since p and P are of the same species p (a)2. Then, if p > P, $\tan p > \tan P$; but $\tan P \cot P = 1$ tan p cot P>1; ... sin b>1, wh. h 1 11 11. per. bl.

Hence, if $P < 90^{\circ}$ and p > P, no sci. i. di 1- jens. in

4. When $P > 90^{\circ}$ and p > P.

We have $\sin b = \tan p \cot P$, b - Las before tan p and cot P are both negative, and tan p < tan P, numer, and to tan P cot P = 1; ... tan p cot P < 1win b < 1: '. $b < 90^\circ$, or $b > 90^\circ$; leads be either of the supplementary ares PH or PH wh. have the common sine equal to tan p cot P.

If $b < 90^{\circ}$, since $p > 90^{\circ}$, $h > 90^{\circ}$; if $b = 10^{\circ}$ kinee 71 41°, h < 90°

Hence, if $P > 90^{\circ}$ and p > P, either that P > P = Por PH'B' will satisfy the conditions, and there was betwo solutions

5. When $P > 90^\circ$ and p = P.



We have sin b = tan p cot P, as before

sin b = tan P cot P = 1; . . b = 'st com h O; , ', com h on com h com p = 15 . cut A = 0; . . con R - tan p cut A = 0;

Hence, if $P > 90^\circ$ and p = P, $b = 90^\circ$, $h = 90^\circ$, $R = 90^\circ$, the two triangles reduce to the bi-rectangular PBB, and there is but one solution.

6. When $P > 90^{\circ}$ and p < P.

As before, we have $\sin b = \tan p \cot P$.

Since p and P are of the same species, and since $P \to (a)^{\circ}$, $p > 90^{\circ}$; hence, $\tan p$, $\cot P$ are both negative, and $\tan p > \tan P$, numerically; but since $\tan P$ cot P = 1, $\tan p$ cot P > 1; ... $\sin b > 1$, which is impossible

H nee, if $P > 90^{\circ}$ and p < P, there is no solution.

7. When P 90°.

$$\tan p = \frac{\sin b}{\cot p} = \frac{\sin b}{0} = \pi; \quad i = 90^\circ.$$

... $\cos p = 0$; ... $\cos h = \cos b \cos p = 0$; ... $h = 90^\circ$.

 $\sin b = \tan p \cot P = \infty + 0$; ... $\sin b$ is indeterminate.

$$\sin B = \frac{\cos P}{\cos p} = \frac{0}{0}$$
; ... sin B is indeterminate.

Hence, if P = 90, then $p = 90^{\circ}$, $h = 90^{\circ}$, h and B are inslite rminate; the triangle is hi-rectangular, and there is an infinite number of solutions.

Hence, the following results:

$$P < 90^{\circ}$$
 and $\begin{cases} P < P, & \text{Two solutions,} \\ P - P, & \text{One solution,} \\ P > P, & \text{No solution.} \end{cases}$

$$P > 90^{\circ}$$
 and $\begin{cases} p > P, & \text{Two solutions.} \\ p = P, & \text{One solution.} \\ p < P, & \text{No solution.} \end{cases}$

$$P = 90^{\circ}$$
 then $\left\{ egin{array}{l} p = 90^{\circ}, \\ h = 90^{\circ}, \\ b \text{ indeterminate,} \\ B \text{ indeterminate,} \end{array} \right\}$ Infinite number of solutions

By a comparison of these results, we find,

- 1. If p differs more from 90° than P, there will be two solutions.
- 2. If p = P, and $P < 90^{\circ}$ or $P > 90^{\circ}$, there will be one solution.
- 3. If $p = P = 90^{\circ}$, there will be an infinite number of solutions.
- 4. If p differs less from 90° than P, there will be no solution.

130. Remarks.

- 1. Napier's principles render it unnecessary to divide the subject of right-angled spherical triangles into cases.
 - 2. Two parts will be given, and three required.
- 3. These parts or their complements will be circular parts.
- Take the two given parts, if they are circular parts, otherwise their complements, and any one part required, if it is a circular part, otherwise its complement, and observe which is the middle part, and whether the other parts are adjacent to, or opposite, the middle part: if adjacent, the first of Napier's principles will give the formula; if opposite, the second.
 - 5. Introduce R and apply logarithms.
- 6. Apply the principles which determine the species of the required part,

131. Examples.

1 Giv.
$$\left\{\frac{b}{p} - 50^{\circ} 47.\right\}$$
 Req $\left\{\frac{b}{B}\right\}$

1. To find b.

From the second of Napier's principles, we have $\sin (90^{\circ} - h) = \cos b \cos p$, or $\cos h = \cos b \cos p$. Finding $\cos b$ and introducing R, we have

$$\cos b = \frac{R \cos h}{\cos p}$$

... log cos b = 10 + log cos h = 1 + cos p.

log cos
$$b$$
 (110° 30') = 9.54433
log cos p (50° 45') = 9.80120 · $= 9.74313 - ..., b$ 123° 36' 81".

Since the hypotenuse is greater than 90°, the sides b and p are of different species; but $p < 90^\circ$; ... $b > 90^\circ$. But log cos b corresponds to 56° 23′ 20° , and to its supplement 123° 36′ 31″ which must be taken, since $b > 90^\circ$.

The species of b can also be determined by the formula,

$$\cos h = \frac{\cos h}{\cos p}.$$

Since $h > 90^{\circ}$, cos h is negative, and since $p < \frac{90^{\circ}}{90^{\circ}}$ cos p is positive. . . cos h is negative: . . . $h = 90^{\circ}$. The signs of the functions may be conveniently indicated by placing the signs after their logarithms.

2. To find B.

$$\sin (90^{\circ} - B) = \tan p \tan (90^{\circ} - h),$$

$$\therefore \cos B = \frac{\tan p \cot h}{R}.$$

... $\log \cos B = \log \tan p + \log \cot h - 10$.

$$log tan p (50° 45') = 10.08776 +
log cot h (110° 30') = 9.57274 -
log cos B = 9.66050 - ... B 117° 14'.$$

Since b and B are of the same species, and since $b > 90^{\circ}$, $B > 90^{\circ}$. The species of B can also be determined from the sign of $\cos B$.

3. To find P.

 $\sin p = \cos (90^{\circ} - h) \cos (90^{\circ} - P)$, or $\sin p = \sin h \sin P$.

$$. . \sin P = \frac{R \sin p}{\sin h};$$

 $\log \sin P = 10 + \log \sin p - \log \sin h$.

log sin
$$p$$
 (50° 45') = 9.88896 + log sin h (110° 30') = 9.97159 .

P is of the same species as p, and since $p < 90^\circ$, $P = 10^\circ$. The picces of P can not be determined by the sign of an P, since the sign of $\sin P$ is plus from 0° to 180° .

2. Given
$$\left\{\begin{array}{c} h = 94^{\circ} \ 05', \\ p = 100^{\circ} \ 45', \end{array}\right\}$$
 Req $\left\{\begin{array}{c} h = 67^{\circ} \ 33' \ 27'' \\ B = 67^{\circ} \ 54' \ 47'' \\ P = 99^{\circ} \ 57' \ 35'' \end{array}\right\}$

$$3 \cdot \operatorname{Giv}(\operatorname{n} \frac{\{ |h| - 110^{\circ} | 46^{\circ} | 26^{\circ}, \}}{4 \cdot B} - \operatorname{Req} \left\{ \frac{h}{p} - \frac{67^{\circ} | 06^{\circ} | 44^{\circ}, }{155^{\circ} | 47^{\circ} | 05^{\circ}, } \right\}$$

9 1 11

7 Giv.
$$\begin{cases} P = 75^{\circ} 30'. \end{cases} \Rightarrow \begin{cases} b = 18^{\circ} 07' 02'' \text{ or } 161^{\circ} 52' 58', \\ h = 50^{\circ} 15' \end{cases} \Rightarrow \begin{cases} b = 18^{\circ} 07' 02'' \text{ or } 161^{\circ} 52' 58', \\ h = 52^{\circ} 34' 31'' \text{ or } 127^{\circ} 25' 29', \\ B = 23^{\circ} 03' 06'' \text{ or } 156^{\circ} 56' 54''. \end{cases}$$

S. If a line make an angle of 40° with a fixed plane, and a plane embracing this line be perpendicular to the fixed plane, how many degrees from its first position must the plane embracing the line revolve about it in order that it may make an angle of 45° with the twest plane?

Ans. 67° 22′ 44″ or 112° 37′ 16″.

132. Polar Triangles.

The polar triangle of a given triangle is the triangle formed by the intersection of three area of great circles described about the vertices of the given triangle as poles.

other, the second is the polar of the first.

Thus, if A'B'C' is the polar of the triangle ABC, then ABC is the polar of A'B'C'.

Each angle in one of two polar triangles is the supplement of the side lying opposite to it in the other; and each side is the supplement of the angle lying opposite to it in the other. Thus,

A
$$180^{\circ} - a'$$
, B $180^{\circ} - b'$, C $180^{\circ} - c'$.
a $180^{\circ} - A'$, b $180^{\circ} - B'$, c $180^{\circ} - C'$.
A' $180^{\circ} - a$, B' $180^{\circ} - b$, C' $= 180^{\circ} - c$.
a' $180^{\circ} - A$, b' $180^{\circ} - B$, c' $= 180^{\circ} - C$.

Cor -If a' 90°, A 90°; hence, if one side of a triangle is 90°, one angle of its polar triangle is 90°.

133. Quadrantal Triangles.

A quadrantal triangle is a triangle one side of which is 90°.

By the corollary of the last article, it follows that the polar of a quadrantal triangle is a right-angled triangle.

A quadrantal triangle is solved by passing to its polar triangle, which is solved as a right-angled triangle, then by passing back to the quadrantal triangle, which is the polar of the right-angled triangle.

134. Examples.

1. Given
$$\begin{cases} h' = -90^{\circ}, \\ p' = 129^{\circ} - 15', \\ b' = -62^{\circ} - 46' - 01'', \end{cases}$$
 Req.
$$\begin{cases} H' = -69^{\circ} - 30', \\ R' = -56^{\circ} - 23' - 30', \\ p' = -124^{\circ} - 14' - 03' \end{cases}$$

Passing to the polar triangle, which is right angled, we have

$$G_{\text{IVen}} \begin{cases} H = 90^{\circ}, \\ p = 50^{\circ} 45', \\ B = 117^{\circ} 13' 59'', \end{cases} , , \begin{cases} b = 110^{\circ} 30', \\ b = 123^{\circ} 36' 30'', \\ P = 55^{\circ} 45' 57'', \end{cases}$$

Provide to the qualitate triangle, we find

$$\begin{cases} c & (8) \\ 0 & (8)^2 & (2) \\ b' & (3)^2 & (12 - 23)^2 \end{cases}$$
 Req.
$$\begin{cases} A' = -74^\circ \cdot 26', \\ C'' = 108^\circ \cdot 05' \cdot 26'', \\ b' = -31^\circ \cdot 29' \cdot 14'', \end{cases}$$

OBLIQUE TRIANGLES.

135. Proposition I.

The sines of the sides of a spherical triangle are propor-

Ist ABC be a spherical triat also From C draw p, the are of a great circle perpendicular to the opposite side or to the AC opposite side produced



In the first case we have, by Napier's principles,

$$a = p = \cos (90^{\circ} - a) \cos (90^{\circ} - B) = \sin a \sin B$$
.

**
$$a p = \cos (90^{\circ} - b) \cos (90^{\circ} - A) = \sin b \sin A$$
.

. . ein a sin B sin b sin A.

... $\sin a : \sin b :: \sin A : \sin B$.

In the second case we have, by Napier's principles,

$$\sin p = \cos (90^{\circ} - a) \cos (90^{\circ} - B') = A$$
 $\sin a \sin B' = \sin a \sin B$

 $\sin p = \cos (90^{\circ} - b) \cos (90^{\circ} - A) = \sin b \sin A.$

. . sin a sin $B = \sin b \sin A$.

... $\sin a : \sin b :: \sin A : \sin B$.

In like manner other proportions may be deduced, giving the group,

- (1) $\sin a : \sin b :: \sin A : \sin B$.
- (2) sin a : sin c :: sin A : sin C.
- (3) $\sin b : \sin c :: \sin B : \sin C$.

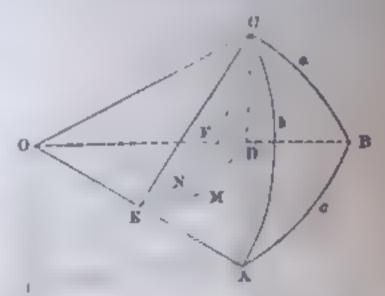
136. Proposition II.

The co-sine of any side of a spherical triangle is equal to the product of the co-sines of the other sides, plus the product of their sines into the co-sine of their included angle.

Let ABC be a spherical triangle, and O the center of the sphere.

Let CM be perpendicular to the plane AOB.

Draw MD and ME, respectively perpendicular to OB and OA, and



draw CD and CE_i which will be respectively perpendicular to OB and OA; hence, the angle CEM = A, and CDM = B. Draw EF perpendicular to OB, and MN perpendicular to EF. Each of the angles MEN and EOF is the complement of OEF; ... MEN = EOF

$$OD = OF + NM$$
.

$$OD = \cos a$$
.

$$OF = OE \cos EOF = \cos b \cos c$$
.

$$NM = EM \sin MEN = \sin b \cos A \sin c$$
.

Substituting the values of OD, OF, and NM, we have

Introduced, giv-

- 1 cer ces boec + sin b sin c cos A.
- 2 cos b cos a cos e sin a sin e cos B.
- C com c = com a com b + sin a sin b com C.

137. Proposition III.

The range of any angle of a spherical triangle is equal to the product of the consumer of the other angles is to the consumer there are placed of the co-since of these are placed.

The formulas for passing to the polar triangle are,

$$a = 180^{\circ} - A', \quad b = 180^{\circ} - B', \quad c = 180^{\circ} - C'.$$

$$A = 180^{\circ} - a', \quad B = 180^{\circ} - b', \quad C = 180^{\circ} - c'.$$

Substituting these values in the formulas of the preceding article and reducing, we have

- -cos $A' = \cos B' \cos C' \sin B' \sin C' \cos a'$. -cos $B' = \cos A' \cos C' - \sin A' \sin C' \cos b'$.
- $-\cos C = \cos A' \cos B \sin A' \sin B' = C.$

Changing the signs and omitting the accents, since the formulas are true for any triangle, we have

- 1 cas A sin B sin C cos a crs B cos C
- 2 $\cos B = \sin A \sin C \cos h + \cos A \cos C$
- (3) $\cos C = \sin A \sin B \cos c \cos A \cos B$.

138. Proposition IV.

The course of our half of any angle of a spherical tri-

dealing the sine of one-half the sum of the sides into the one of one-half the sum minus the side opposite the angle, by the product of the sines of the adjacent sides.

The first formula of article 136 gives

$$\cos A = \frac{\cos a - \cos b \cos c}{\sin b \sin c}.$$

Adding 1 to both members, we have

$$1 + \cos A = \frac{\cos a + \sin b \sin c + \cos b \cos c}{\sin b \sin c}$$

 $1 + \cos A = 2 \cos^2 \frac{1}{2} A$. Article 95, (10).

 $\sin b \sin c - \cos b \cos c = -\cos (b + c)$. Art. 89, (b).

$$2 \cos^2 \frac{1}{2}A = \frac{\cos a - \cos (b+c)}{\sin b \sin c}$$

But by article 96, (8), we have

$$\cos a - \cos (b + c) = 2 \sin \frac{1}{2}(a + b + c) \sin \frac{1}{2}(b + c - a)$$
.

Substituting and dividing by 2, we have

$$\cos^{2} \frac{1}{2}A = \frac{\sin \frac{1}{2}(a + b + c) \sin \frac{1}{2}(b + c - a)}{\sin b \sin c}$$

Let a = a + b + c, then will $\frac{1}{2}a = \frac{1}{2}(a + b + c)$, $\frac{1}{2}a - a = \frac{1}{2}(b + c - a)$.

Substituting in the value of $\cos^2 \frac{1}{2}A$, and in the similar values for $\cos^2 \frac{1}{2}B$ and $\cos^2 \frac{1}{2}C$, and extracting the square root, we have

(1)
$$\cos \frac{1}{2}$$
.1 $\sqrt{\sin \frac{1}{2} \sin (\frac{1}{2} s - a)}$ $\sin b \sin c$

(2)
$$\cos \frac{1}{2}B = \sqrt{\frac{\sin \frac{1}{2}s \sin (\frac{1}{2}s - b)}{\sin a \sin c}}$$

(3)
$$\cos \frac{1}{2}C = \sqrt{\frac{\sin \frac{1}{2}s \sin (\frac{1}{2}s - c)}{\sin a \sin b}}$$

139. Proposition V.

The second of a symbol of a spherical triangle is a specific triangle is a second of the part obtained by dividing the same of the angles into the same is seen if the sum minus the angle opposite the side, and a second of the angles opposite the side,

Tak ug the formulas of the last article, passing to the palar triangle, making S = A' + B' + C', substituting in these formulas, reducing, and omitting the acceptance we have

(1)
$$\sin \frac{1}{2}a = \sqrt{-\cos \frac{1}{2}S \cos (\frac{1}{2}S - A)}$$

 $\sin B \sin C$

(3)
$$\sin \frac{1}{2}c \sqrt{-\cos \frac{1}{2}S\cos ckS}$$
 ($\sin A \sin B$

140. Proposition VI.

The me of one half of any angle of nephro to trade is easi to the square root of the quotient obtained document the one-half the man of the sides minus of adjustent sides when the other adjustent sides to be product of the successful the sum minus the other adjustent sides.

$$\cos A = \frac{\cos a - \cos b \cos c}{\sin b \sin c}$$
. Article 136, (1).

Subtracting both members from 1, we have

$$1 - \cos A = \frac{\cos b \cos c + \sin b \sin c - \cos a}{\sin b \sin c}.$$

$$1 - \cos A = 2 \sin^2 \frac{1}{2}A \quad Article 95, (9).$$

cos b cos c + sin b sin c cos (b - c). Article 91, (d).

$$\therefore 2 \sin^2 \frac{1}{2}A - \frac{\cos (b-c) - \cos a}{\sin b \sin c}.$$

But by article 96, (8), we have

$$\cos (b-c) = \cos a - 2 \sin \frac{1}{2}(a+c-b) \sin \frac{1}{2}(a+b-c)$$
.

Substituting and dividing by 2, we have

$$\sin^{2} \frac{1}{2}A = \frac{\sin \frac{1}{2}(a + c - b) \sin \frac{1}{2}(a + b - c)}{\sin b \sin c}.$$

But
$$\frac{1}{2}(a+c-b) = \frac{1}{2}s-b$$
 and $\frac{1}{2}(a+b-c) = \frac{1}{2}s-c$.

Substituting in the value of $\sin^2 \frac{1}{2}A$, and in the similar values for $\sin^2 \frac{1}{2}B$ and $\sin^2 \frac{1}{2}C$, and extracting the square root, we have

(1)
$$\sin \frac{1}{2}A = \sqrt{\frac{\sin (\frac{1}{2}s - b)}{\sin b} \frac{\sin (\frac{1}{2}s - c)}{\sin b}}$$
.

(2)
$$\sin \frac{\pi}{a}B = \sin \left(\frac{4\pi}{3}a - a\right) \sin \left(\frac{4\pi}{3}a - c\right)$$
.

(3)
$$\sin \frac{1}{2}C = \sqrt{\frac{\sin \frac{1}{2}s - a}{\sin a \sin b}} \cdot \frac{(\frac{1}{2}s - b)}{\sin a \sin b}$$

141. Proposition VII.

The cosme of one half of any side of a spherical triangle is equal to the square root of the quotient obtained by deciding the cosme of one half the sum of the angles minus one adjustent angle into the cosme of half the sum minus the other adjacent angle, by the product of the sines of the adjacent angle,

Taking the 4 mails of the last article, passing to the polar triangle, making S = A' + B' + C', substituting, reducing, and emitting the accents, we have

1)
$$\cos \frac{\pi}{2}$$
: $\cos \left(\frac{\pi}{2}S + B\right) \cos \left(\frac{\pi}{2}S + C\right)$, $\sin B \sin C$

(2)
$$\cos \frac{1}{2}b = \sqrt{\cos \frac{1}{2}S + A)\cos (\frac{1}{2}S + C)}$$
, $\sin A \sin C$

(3)
$$\cos \frac{1}{2}e^{-x}\sqrt{\cos(\frac{1}{2}S + A)\cos(\frac{1}{2}S + B)}$$
, $\sin A \sin B$

142. Proposition VIII.

The tangent of one-half of any angle of a spherical triancie is equal to the square root of the quotient obtained by dirating the sine of one-half the sum of the sides minus one the side into the sine of one-half the sum minus the other absent side, by the sine of one-half the sum of the sides into the sans of one-half the sum minus the opposite side.

Dividing (1), (2), (3), article 140, respectively, by (1), (2), (3), article 138, we have

(1)
$$\tan \frac{1}{2}A = \sqrt{\sin \frac{1}{2}s} = h \sin \frac{1}{4}s = c$$
, $\sin \frac{1}{2}s \sin \frac{1}{2}s \sin \frac{1}{2}s = c$.

(2)
$$\tan \frac{1}{2}B = \sqrt{\frac{\sin(\frac{1}{2}s - a \sin(\frac{1}{2}s - a)}{\sin(\frac{1}{2}s \sin(\frac{1}{2}s - b)}}$$

(3)
$$\tan \frac{1}{2}C = \sqrt{\frac{\sin (\frac{1}{2}s - a) \sin (\frac{1}{2}s - b)}{\sin \frac{1}{2}s \sin (\frac{1}{2}s - c)}}$$

143. Proposition 1X.

The tangent of one half of any side of a spherical triangle is open to the square root of the quotient obtained by dividing

names the co-sine of one-half the sum of the angles into use co-sine of one-half the sum minus the angle opposite the role, by the co-sine of one-half the sum of the angles minus one adjacent angle into the co-sine of one-half the sum minus the other adjacent angle.

Dividing (1), (2), (3), article 139, respectively, by (1), 2, (3), article 141, we have

(1)
$$\tan \frac{1}{2}a = \sqrt{\frac{-\cos \frac{1}{2}S\cos (\frac{1}{2}S - A)}{\cos (\frac{1}{2}S - B)\cos (\frac{1}{2}S - C)}}$$

(2)
$$\tan \frac{1}{2}b = \frac{-\cos \frac{1}{2}S \cos (\frac{1}{2}S - B)}{\cos (\frac{1}{2}S - A) \cos (\frac{1}{2}S - C)}$$

(3)
$$\tan \frac{3}{2}e^{-}$$
 $\cos \frac{3}{4}S \cos (\frac{1}{2}S - C)$ $\cos \frac{3}{4}S - A) \cos (\frac{1}{2}S - B)$

The reciprocals of (1), (2), (3), articles 142, 143, will give formulas for co-tangents, which may be written and expressed in words.

144. Napier's Analogies.

Dividing (1), article 142, by (2), we have

$$\tan \frac{1}{2} \frac{A}{B} = \frac{\sin \left(\frac{1}{2}s - b\right)}{\sin \left(\frac{1}{2}s - a\right)}.$$

This, as a proportion taken by composition and di-

$$\frac{\tan \frac{1}{2}A + \tan \frac{1}{2}B}{\tan \frac{1}{2}A - \tan \frac{1}{2}B} = \frac{\sin (\frac{1}{2}a - b) + \sin (\frac{1}{2}a - a)}{\sin (\frac{1}{2}a - b) - \sin (\frac{1}{2}a - a)}.$$

$$\tan \frac{1}{2}A + \tan \frac{1}{2}B = \frac{\sin \frac{1}{2}A}{\sin \frac{1}{2}A} + \frac{\sin \frac{1}{2}B}{\cos \frac{1}{2}A}$$

$$\tan \frac{1}{2}A + \tan \frac{1}{2}B = \frac{\cos \frac{1}{2}A}{\sin \frac{1}{2}A} + \frac{\sin \frac{1}{2}B}{\cos \frac{1}{2}B}$$

$$\cos \frac{1}{2}A + \cos \frac{1}{2}B$$

Variable not both terms of the second member by $a = 1 + a = \frac{1}{2}B$,

Edung the second member by articles 89, (a), and

$$\tan \frac{1}{2}A + \tan \frac{1}{2}B = \frac{\sin \frac{1}{2}(A+B)}{\sin \frac{1}{2}(A-B)}.$$

$$\frac{\pi}{\sin \frac{1}{2}a-b} + \sin (\frac{1}{2}a-a) = \frac{\tan \frac{1}{2}c}{\tan \frac{1}{2}(a-b)}$$
. Art. 96, (11).

$$\sin \frac{1}{2}(A+B) = \tan \frac{3}{2}c$$

$$\sin \frac{1}{2}(A+B) = \tan \frac{3}{2}ca - b$$

. . (1) $\sin \frac{1}{2}(A+B)$: $\sin \frac{1}{2}(A-B)$:: $\tan \frac{1}{2}c$: $\tan \frac{1}{2}(a-b)$.

The reciprocal of (1) \times (2), article 142, gives

$$\frac{1}{\tan \frac{1}{2}A \tan \frac{1}{2}B} = \frac{\sin \frac{1}{2}s}{\sin (\frac{1}{2}s - c)}.$$

By division and composition, we have

$$\frac{1 - \tan \frac{1}{2}A \tan \frac{1}{2}B}{1 + \tan \frac{1}{2}A \tan \frac{1}{2}B} = \frac{\sin \frac{1}{2}s - \sin (\frac{1}{2}s - c)}{\sin \frac{1}{2}s + \sin (\frac{1}{2}s - c)}.$$

Reducing both members as before, we have

$$\frac{\cos \frac{1}{2} A B}{\cos \frac{1}{2} (A - B)} = \frac{\tan \frac{1}{2}c}{\tan \frac{1}{2} (a + b)}.$$

.". (2)
$$\cos \frac{1}{2}(A+B) : \cos \frac{1}{2}(A-B) : \tan \frac{1}{2}c : \tan \frac{1}{2}(a+b)$$
.

Passing from (1) and (2) to the polar triangle, we have

(3)
$$\sin \frac{1}{2}(a+b) : \sin \frac{1}{2}(a-b) :: \cot \frac{1}{2}C : \tan \frac{1}{2}(A-B)$$
.

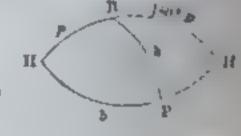
(4
$$\cos \frac{1}{2}(a+b) : \cos \frac{1}{2}(a-b) :: \cot \frac{1}{2}C : \tan \frac{1}{2}(A+B)$$

145. Proposition.

In a right-angled spherical triangle, as b increases from 0° to 90° , from 90° to 180° , from 180° to 270° , and from 270° to 360° , if $p < 90^{\circ}$, h increases from p to 90° , from 90° to $180^{\circ} - p$, decreases from $180^{\circ} - p$ to 90° , and from 90° to p; if $p > 90^{\circ}$, h decreases from p to 90° , from 90° to $180^{\circ} - p$, increases from $180^{\circ} - p$ to 90° , and from 90° to p; if $p = 90^{\circ}$, $h = 90^{\circ}$ for all values of b.

1. $p < 90^{\circ}$; ... cos p is positive.

cos $h = \cos b \cos p$.



If b = 0, $\cos b = 1$; therefore, $\cos b = \cos p$; . . b = p.

As b increases from 0° to 90°, cos b is positive, and diminishes from 1 to 0; ... cos h is positive, and diminishes from cos p to 0; ... h increases from p to 90°.

As b increases from 90° to 180°, cos b is negative, and increases numerically from 0 to -1; ... cos h is negative, and increases numerically from 0 to $-\cos p$; ... h increases from 90° to 180° -p, and the triangle becomes the lune HH'.

As b increases from 180° to 270°, cos b is negative, and decreases numerically from — 1 to 0; ... cos h is negative, and decreases numerically from — cos p to 0; ... h decreases from 180° — p to 90°.

As b increases from 270° to 360°, cos b is positive, and increases from 0 to 1; ... cos h is positive, and increases from 0 to cos p; ... h decreases from 90° to p, and the triangle becomes the hemisphere

2. p + 30 , . . cos p is negative.

to b in bases from 0° to 90°, cos b is positive, and cos a see from 1 to 0; ... cos b is negative, and decreases numerically from cos p to 0; ... h decreases in m p to 90°.

As hear axes from 90° to 180°, cos / is negative, and mercases numerically from 0 to T, cos h is pastive, and increases from 0 to G_{-p} , h despective from 90° to 180°— p_i and the triangle becomes the lune HH'.

As homerouses from 180° to 270°, cos—negative, and decreases numerically from -1 to 0_j —cos h is positive, and decreases from $-\cos p$ to 0; ... h increases from $180^\circ-p$ to 90° .

As h increases from 270° to 360°, cos h a positive, and increases from 0 to 1; ..., cos h as a gatave, and increases numerically from 0 to cos p. h ancreases from 90° to p. and the triangle becomes the hemissiphere

3.
$$p = 90^{\circ}$$
; ... cos $p = 0$.

••• cos $h = \cos b \cos p = 0$; . . $h = (\mu)^{\circ}$.

Cir -Since B and b are of the same species B may

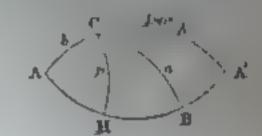
In the application of these principles to the discussion of Case I, in which two sides and an angle opposite one of them are given, a corresponds to h, and HR to h

146. Case I.

Given two sides of a spherical triangle, and the angle oppoute one of them; required the remaining parts.

Let a and b be the given sides and A the given angle.

I.
$$A < 90^{\circ}$$
; $p < 90^{\circ}$.
 $\sin p = \sin b \sin A$.



B coincides with H, and the triangle ABC becomes the right triangle AHC.

2.
$$a < 90^{\circ} \text{ and } a > p$$
.

By the last proposition the point B lies in the first or fourth quadract, estimated from H.

 $HB = 90^{\circ} \text{ or } 270^{\circ}, \text{ and } HCB = 90^{\circ} \text{ or } 270^{\circ}.$

B lies in the second or third quadrant from H.

5.
$$a = 180^{\circ} - p$$
.

HB 180°, and ABC AHC + 1 the hemisphere.

6 a
$$180^{\circ} - b$$
.

HB = HA' or $360^\circ - HA'$, and then the first triangle becomes the lune AA'.

HB = AH or $360^{\circ} - AH$, and the second triangle becomes the hemisphere.

I transle is impossible, since p is the least, and the p is the greatest value of a.

11
$$A > 90^{\circ}$$
; $A > 90^{\circ}$.

sin p sin b sin A.



1. a p.

B committee with H, and ABC becomes AHC.

2.
$$a > 90^{\circ}$$
 and $a < p$.

Blus in the first or fourth quadrant from H.

8.
$$a = 90^{\circ}$$
.

 $HB = 90^{\circ} \text{ or } 270^{\circ}, \text{ and } HCB = 90^{\circ} \text{ or } 270^{\circ}.$

4.
$$a < 90^{\circ}$$
 and $a > 180^{\circ} - 7$

B lies in the second or third quadrant from H.

5.
$$a = 180^{\circ} - p$$
.

 $HB = 180^{\circ}$, and $ABC = AHC + \frac{1}{2}$ the hemisphere.

6.
$$a = 180^\circ - b$$
.

HB = HA' or $360^\circ - HA'$, and the first trangle becomes the lune AA'.

7.
$$a = b$$
.

HB = AH or $360^{\circ} - AH$, and the second triangle becomes the hemisphere.

8.
$$a > p$$
 or $a < 180^{\circ} - p$.

The triangle is impossible, since p is the greatest, and $180^{\circ} - p$ is the least value of a.

The triangle is right-angled, and is solved as in article 131

147. Examples.

1. Given
$$\begin{cases} a = 60^{\circ} 20', \\ b = 80^{\circ} 35', \\ A = 38^{\circ} 25', \end{cases}$$
 Req. $\begin{cases} B, \\ C, \\ c, \end{cases}$

$$A < 90^{\circ}; \quad p < 90^{\circ}.$$

 $\sin p = \sin b \sin A$, ... $p = 37^{\circ} 48' 26''$.

Since a > p and $a < 180^{\circ} - p$, the triangle is possible.

Since a < b and $a < 180^{\circ} - b$, B lies between H and A or H and A'.

 $\sin p = \sin a \sin B$, ... $B = 44^{\circ} 52' .05''$.

cos $HCB = \tan p \cot a$, ... $HCB = 63^{\circ} 46' 18''$.

cos $a = \cos p \cos HB$, ... HB . 51° 12′ 41″.

coe $ACH = \tan p \cot b$, ... $ACH = 82^{\circ} 36' 25''$.

 $\cos b = \cos p \cos AH$, ... $AH = 78^{\circ} 02' 54''$.

 $C = ACH + HCB = 146^{\circ} 22' 43''$ or $18^{\circ} 50' 07''$.

 $c = AH \pm HB = 129^{\circ} 15' 35'' \text{ or } 26^{\circ} 50' 13''.$

In ACB_1 $ABC = 180^{\circ} - HBC = 135^{\circ}$ 07' 55".

We can also find B from the proportion, $\sin a : \sin b :: \sin A : \sin B$.

C and c can be found from the proportions, $\sin \frac{1}{2}(b+a)$: $\sin \frac{1}{2}(b-a)$:: $\cot \frac{1}{2}C$: $\tan \frac{1}{2}(B-A)$. $\sin A$: $\sin C$:: $\sin a$: $\sin c$.

2. Given. Required.

2. Given. Required.

3. Required. Required.

 $\begin{cases} a = 63^{\circ} \ 50', \\ b = 80^{\circ} \ 10', \\ A = 51^{\circ} \ 30', \end{cases} \begin{cases} B = 59^{\circ} \ 16' \ 00'' \text{ or } 120^{\circ} \ 44' \ 00' \\ C = 131^{\circ} \ 29' \ 42'' \text{ or } 24^{\circ} \ 37' \ 30' \\ c = 120^{\circ} \ 47' \ 50'' \text{ or } 28' \ 32' \ 44' \end{cases}$

$$\begin{cases} A & \text{for AC AS".} \\ b & \text{64' 23' 15".} \end{cases} \begin{cases} B & \text{114° 26' 50" or 65° 33' 10"} \\ C = 236° 51' 27" or 97° 27' 13".} \\ c = 236° 01' 51" or 100° 49' 49". \end{cases}$$

5. Green.

Required.

$$\begin{cases}
a = 100^{\circ}. \\
b = 50^{\circ} 47' 41'' \text{ or } 129^{\circ} 12' 19''. \\
C = 156^{\circ} 05' 16' \text{ or } 342^{\circ} 03' 12'. \\
c = 187^{\circ} 50' 09'' \text{ or } 336^{\circ} 39' 45''.
\end{cases}$$

if If $A < 90^\circ$, what is the relation of a to p_i or to 1% — p_i when there is no solution?

7 If $A > 90^{\circ}$, what is the relation of a to p, or to $1^{\circ}1^{\circ} - p$, when there is no solution?

148. Proposition.

Is a right-angled spherical triangle, as B increases from $0.7 \pm 0.90^{\circ}$ from 90° to 1.80° , from 1.80° to 2.70° , and from 2.70° to 360° ; if $p < 90^{\circ}$, P decreases from 90° to p, increases from 90° to $1.80^{\circ} - p$, and decreases from $1.80^{\circ} - p$ to 90° ; if $p > 90^{\circ}$, P increases $p = 1.80^{\circ} - p$, and increases from $p = 1.80^{\circ} - p$ to $p = 1.80^{\circ} - p$, and increases from $p = 1.80^{\circ} - p$ to $p = 1.80^{\circ} - p$, and increases from $p = 1.80^{\circ} - p$ to $p = 1.80^{\circ}$, for all values of B.

I
$$p < 90^{\circ}$$
; ... cos p is positive.

Out $P = \cos p \sin B$.

If $B = 0^{\circ}$, $\sin B = 0$; ... $\cos P = 0$; ... $P = 90^{\circ}$.

As B increases from 0° to 90° , sin B is positive, and increases from 0 to 1; ... cos P is positive, and increases from 0 to cos p_{i} ... P decreases from 90° to p_{i} .

As B increases from 90° to 180°, sin B is positive, and decreases from 1 to 0; ... cos P is positive, and decreases from cos p to 0; ... P increases from p to 90° , and the triangle becomes the lune HH'.

As B increases from 180° to 270°, sin B is negative, and increases numerically from 0 to -1; ... cos P is negative, and increases numerically from 0 to $-\cos p$; ... P increases from 90° to 180° -p.

As B mercus s from 270° to 360° sin B is negative, and decreases numerically from — 1 to 0; ... cos P is negative, and decreases numerically from — cos p to 0; ... P decreases from 180° — p to 90° , and the triangle becomes the hemisphere.

2.
$$p > 90^{\circ}$$
; ... cos p is negative.
 $\cos P = \cos p \sin B$.

If
$$B = 0^{\circ}$$
, $\sin B = 0$; ... $\cos P = 0$; ... $P = 90^{\circ}$.

As B increases from 0° to 90° , sin B is positive, and increases from 0 to 1_{1}° , 1_{1}° , 1_{2}° cos P is negative, and increases numerically from 0 to 1_{2}° , 1_{2}° . P increases from 1_{2}° from 1_{2}° to 1_{2}° .

As B increases from 90° to 180°, sin B is positive, and decreases from 1 to 0; ... cos P is negative, and decreases numerically from cos p to 0; ... P decreases from p to 90°, and the triangle becomes the lune.

As B increases from 180° to 270°, sin B is negative, and increases numerically from 0 to 1; ... cos P is positive, and increases from 0 to $-\cos p$; ... P decreases from 90° to $180^{\circ} - p$.

As R manass from 270 to 360°, $\sin B$ is negative, and decreases numerically from — 1 to 0; ..., $\cos P$ is particle, and decreases numerically from — $\cos p$ to 0,

P reases from 180° — p to 90°, and the triangle becomes the homesphere

.
$$\cos P = \cos p \sin B = 0$$
; . $P = 90^{\circ}$.

Cor - Since b and B are of the same species, b may be substituted for B in the preceding proposition.

149. Case II.

Given two angles of a spherical triangle and the side of punits one of them; required the remaining parts.

Let A and B be the given angles, and b the given side.

I.
$$A < 90^{\circ}$$
; $p < 90^{\circ}$.

Sin $p = \sin b \sin A$.

By the last proposition, the point B lies in the first or second quadrant estimated from H as origin.

The angle $HCB = 90^{\circ}$, and the arc $HB = 90^{\circ}$.

3.
$$B < 180^{\circ} - p$$
, and $B > 90^{\circ}$.

B lies in the third or fourth quadrant from II.

4.
$$B = 180^{\circ} - p$$

The angle $HCB = 270^{\circ}$, and the arc $HB = 270^{\circ}$.

5.
$$B = 90^{\circ}$$
.

 $HB=0^{\circ}$, 180°, or 360°, and the triangle becomes ACH, $ACH+\frac{1}{2}$ of a hemisphere, or a hemisphere + ACH.

B lies in the first or second quadrant from H_1 , and one of the triangles becomes the lune AA'.

7.
$$B = 180^{\circ} - A$$
.

B lies in the third or fourth quadrant from H, and one of the triangles becomes the hemisphere.

8.
$$B 180^{\circ} - p$$
.

The triangle is impossible, since p is the least, and $180^{\circ} - p$ is the greatest value of B.

11
$$1 > 90^{\circ}$$
; $p > 90^{\circ}$.

$$\sin p = \sin b \sin A.$$

1.
$$B < p$$
 and $B > 90^{\circ}$.

B lies in the first or second quadrant from H.

2.
$$B = p$$
.

The angle $HCB = 90^{\circ}$, and the arc $HB = 90^{\circ}$.

3.
$$B > 180^{\circ} - p$$
 and $B < 90^{\circ}$.

B lies in the third or fourth quadrant from H.

4.
$$R = 180^{\circ} - p$$
.

The angle $HCB = 270^{\circ}$, and the arc $HB = 270^{\circ}$.

5.
$$B = 90^{\circ}$$
.

ACH, $ACH + \frac{1}{2}$ of a hemisphere, or a hemisphere + ACH.

6. B A

Bit is in the first or second quadrant from H, and one of the triangles becomes the lune AA'.

E las in the third or fourth quadrant from H, and so of the triangles becomes the hemisphere.

8.
$$B > p$$
 or $B < 180^{\circ} - p$.

The trumple is impossible, since p is the greatest, and 180° p is the least value of B.

HI. A 90°

The triangle is right-angled, and is solved as in art, le 131.

150. Examples.

I Giv
$$\begin{cases} A = 75^{\circ} 30', \\ B = 80^{\circ} 40', \\ b = 70^{\circ} 50', \end{cases}$$
 Req $\begin{cases} a, \\ c, \end{cases}$ A $\begin{cases} 90^{\circ}; & 1$

 $\sin p = \sin b \sin A$; $\therefore p = 66^{\circ} 0.7' 56''$

Since B > p and $< 180^{\circ} - p$, the triangle is possible.

Since $B < 90^{\circ}$ and > p, B lies in the first or second quadrant from H.

".n p sin a sin B, ... a =
$$\begin{cases} 67^{\circ} 56'. \\ 112^{\circ} 04'. \end{cases}$$

The second value of a, the supplement of the first is taken when B lies in the second quadrant from H.

 $cos B = cos p sin HCB, \therefore HCB = \begin{cases} 23^{\circ} 37' 44'', \\ 156^{\circ} 22' 16'', \end{cases}$ $sin HB = tan p cot B, \therefore HB = \begin{cases} 21^{\circ} 48' 19'', \\ 158^{\circ} 11' 41'', \end{cases}$ $cos ACH = tan p cot b, \dots ACH = 38^{\circ} 13' 36', \end{cases}$ $cos b = cos p cos AH, \dots AH = 35^{\circ} 46', \end{cases}$ $C = ACH + HCB = 61^{\circ} 51' 20'' \text{ or } 194^{\circ} 35' 52'', \end{cases}$ $c = AH + HB = 57^{\circ} 84' 19'' \text{ or } 193^{\circ} 57' 41'', \end{cases}$

We can find a, c, and C from the proportions, $\sin B$: $\sin A$:: $\sin b$: $\sin a$.

 $\sin \frac{1}{2}(B + A) :: \tan \frac{1}{2}c : \tan \frac{1}{2}(b - a).$ $\sin b : \sin c :: \sin B : \sin C.$

2. Given.

Required.

80° 03′ 25″ or 99° 56′ 35″.

80° 03′ 25″ or 173° 30′ 52″.

161° 24′ 52″ or 173° 30′ 52″.

145° 03′ 13″ or 168° 18′ 23″.

Required.

Required.

(A 132° 16) (a 65° 16′ 30″ or 114° 43′ 30″.

 $\left\{ \begin{array}{l} A = 132^{\circ} \ 16 \\ B = 139^{\circ} \ 44', \\ b = 127^{\circ} \ 30', \end{array} \right\} \left\{ \begin{array}{l} a = 65^{\circ} \ 16' \ 30'' \ \text{or} \ 114^{\circ} \ 43' \ 30'', \\ C = 165^{\circ} \ 41' \ 46'' \ \text{or} \ 128^{\circ} \ 40' \ 44'', \\ c = 162^{\circ} \ 20' \ 55'' \ \text{or} \ 100^{\circ} \ 07' \ 25'', \end{array} \right.$

4. Given.

Required. $\begin{cases}
A :: 48^{\circ} 50' \\
B = 131^{\circ} 10' \\
b = 75^{\circ} 48'
\end{cases}$ $\begin{cases}
a : 75^{\circ} 48' & \text{or } 104^{\circ} 12' \\
C = 360^{\circ} & \text{or } 328^{\circ} 89' 28'' \\
c = 360^{\circ} & \text{or } 317^{\circ} 56' 42'' .
\end{cases}$

Scholium.—In the two preceding cases some of the parts are found to be greater than 180°; but the corresponding triangles conform to the conditions of the problem, and are therefore true solutions.

When we the principles of the following article will a first true, mg the species of the parts.

The peramption established in Geometry are given we out demonstration.

15L Principles.

- 1 First part of a spherical triangle is less than 180°.
- 2 The greater side is opposite the greater angle, and con-
 - 3. Each side is less than the sum of the other sides.
 - 4 The sum of the sides is less than 360°.
- 5. The sum of the angles is greater than 180°, and less than 540°.
- Each angle is greater than the difference between 180° and the sum of the other angles.

For,
$$A + B + C > 180^{\circ}$$
. Principle 5.
. $A > 180^{\circ} - (B + C)$.

The last formula is always algebraically true; but in case $B + C > 180^{\circ}$, it might be doubted whether it is numerically true.

Passing to the polar triangle, we have, by principle 3.

$$a' < b' + c'$$
.
or $180^{\circ} - A < 180^{\circ} - B + 180^{\circ} - C$.
or $-A < 180^{\circ} - (B + C)$.
 $A > B + C - 180^{\circ}$.

7. A side differing more from 90° than another side is of the same species as its opposite angle.

By article 136, we have

 $\cos a = \cos b \cos c + \sin b \sin c \cos A$.

$$\therefore \cos A = \frac{\cos a - \cos b \cos c}{\sin b \sin c}.$$

But sin b sin c is positive, since b and c are each besthan 180°.

If a differs more from 90° than b or c, then we shall lave

cos $a > \cos b$, or $\cos a > \cos c$, numerically; and since neither $\cos b$ nor $\cos c$ exceeds 1, we have $\cos a > \cos b \cos c$.

are of the same species.

8. An angle differing more from 90° than another angle is of the same species as its opposite side.

By article 137, we have

B. N. 13

$$\cos A = \sin B \sin C \cos a - \cos B \cos C$$

$$\cos A = \cos B \sin C \cos B \cos C$$

$$\sin B \sin C$$

If A differs more from 90° than B or C, then, as before, cos A and cos n have the same sign, or A and 4 are of the same species.

9. Two sides, at least, are of the same species as their opposite angles, and conveniely.

If each of two sides differs more from 90° than the remaining side, they will be of the same species as their opposite angles, as is evident from principle 7.

If the triangle is isosceles, and the equal sides less than 90°, the perpendicular from the vertex to the third side will be less than 90°, since one half the

third side is less than 90°, and the angles opposite this perpendicular will be less than 90°, article 129, or of the same species as their opposite sides.

If the equal sides are greater than 90°, the perpendicular will be greater than 90°, since one half the third side is less than 90°, and the angles opposite the perpendicular will be greater than 90°, article 129, or of the same species as their opposite sides.

If one side exceeds 90° by as much as 90° exceeds another side, and the third side is greater or less than each of the other sides, this third side is of the same species as its opposite angle by principle 7.

If the greater of the two sides is of the same species as its opposite angle, then we shall have two sides of the same species as their opposite angles

If the greater of the two sides is not of the same species as its opposite angle, this angle will be of the same species as the other side, or less than 90°; but the angle opposite this other side is less than the angle opposite the greater side, and hence less than 90°, or of the same species as its opposite side, and again we have two sides of the same species as their opposite angles

10. The sum of two sides in greater than, equal to, or less than, 180°, according as the sum of their eggs an angles is greater than, equal to, or less than, 180°.

 $\tan \frac{1}{2}(a+b) \cos \frac{1}{2}(A+B) = \tan \frac{1}{2}c \cos \frac{1}{2}(A-B)$. Art. 144. But $c < 180^{\circ}$, $\therefore \frac{1}{2}c < 90^{\circ}$, $\tan \frac{1}{2}c > 0$,

and $A-B < 180^{\circ}$, $\therefore \frac{1}{2}(A-B) < 90^{\circ}$, $\cos \frac{1}{2}(A-B) > 0$.

- ... $\tan \frac{1}{2}e \cos \frac{1}{2}(A-B) > 0$, $\tan \frac{1}{2}(a+b) \cos \frac{1}{2}(A+B) > 0$.
- ••• tan $\frac{1}{2}(a+b)$ and cos $\frac{1}{2}(A+B)$ have like signs.

.. If A + B > 0 or $< 180^{\circ}$, $\frac{1}{2} a + b > 0$ or $< 180^{\circ}$, $\frac{1}{2} a + b > 0$ or $< 180^{\circ}$. .. If A + B > 0 or $< 180^{\circ}$, a + b > 0 or $< 180^{\circ}$.

152. Case III.

Given two rules and the medulal unite of a spheroid triangle; required the remains g parts.

1 Given
$$\begin{cases} a = 85 & M \\ b = 65 & 47 \\ C = 95 & 50 \end{cases}$$
 Req
$$\begin{cases} A \\ B \end{cases}$$

We have, article 144,

$$\cos \frac{1}{2}(a+b) : \cos \frac{1}{2}(a-b) : \cot \frac{1}{2}C \quad \cot \frac{1}{2$$

We also have, article 144,

$$\sin \frac{1}{2}(A+B) : \sin \frac{1}{2}(A-B) : : \tan \frac{1}{2}e : \tan \frac{1}{2}(e-b)$$

 $\frac{1}{2}e = 46^{\circ} 43' \ 00'', ... e = 93^{\circ} 26' 14''.$

We can also find e from the proportion,

the proportion employed; for if we take the said ment of 46° 41' OFF, then a would be greater than 180°

Again, all the known terms of the proportion a positive; hence, tan je is positive, ... je < 90°.

153. Case IV.

Given two angles and the included sale of a spherical triangle; required the remaining parts

1. Giv.
$$\begin{cases} A = 62^{\circ} 54', \\ B = 48^{\circ} 30', \\ C = 114^{\circ} 20' 58'', \end{cases} \text{Req.} \begin{cases} a, \\ b, \\ C, \end{cases}$$

We have, article 144,

$$\cos \frac{1}{2}(A + B) + \cos \frac{1}{2}(A + B) + \sin \frac{1}{2}e + \tan \frac{1}{2}e + \tan \frac{1}{2}(a + b),$$

$$\sin \frac{1}{2}(A + B) + \sin \frac{1}{2}(A + B) + \tan \frac{1}{2}e + \tan \frac{1}{2}e + \tan \frac{1}{2}(a + b),$$

$$\int \frac{1}{2}(a + b) + 69^{\circ} 55' 48'',$$

$$\int \frac{1}{2}(a + b) + 13^{\circ} 16' 18'',$$

We also have, article 144.

$$\sin \frac{1}{2}(a+b)$$
; $\sin \frac{1}{2}(a-b)$; $\cot \frac{1}{2}C$ τ 1.1 B.

$$2. \ \text{Given} \left\{ \begin{matrix} A = 126^{\circ} \ 35' \ 02'', \\ B = 61^{\circ} \ 13' \ 58'', \\ c = 57^{\circ} \ 30', \end{matrix} \right\} \ \text{Req} \left\{ \begin{matrix} a = 115^{\circ} \ 19' \ 57'', \\ h = 57' \ 59'', \\ C = 48^{\circ} \ 31' \ 38'', \end{matrix} \right.$$

154. Case V.

the angles.

1. Giv.
$$\begin{cases} a = 100^{\circ} 40^{\circ} 30^{\circ} \\ b = 99^{\circ} 40^{\circ} 45^{\circ} \end{cases} \operatorname{Req} \begin{cases} A, & b \\ B, & f \end{cases}$$

By article 138, we have

Introducing R and applying legarithms, we have

$$\log \cos \frac{1}{2}A = \frac{1}{2} [\log \sin \frac{1}{2}a + \log \sin \frac{1}{2}a - a]$$
$$= a \cdot c \log \sin b + a \cdot c \log \sin c].$$

...
$$\frac{1}{2}A = 48^{\circ} 43^{\circ} 14^{\circ}$$
, ... $A = 97^{\circ} 26^{\circ} 28^{\circ}$
In like manner we find $\begin{cases} B = 95^{\circ} 38^{\circ} 04^{\circ} \\ C = 65^{\circ} 33^{\circ} 04^{\circ} \end{cases}$

2. Given
$$\begin{cases} a = 85^{\circ} 30^{\circ} \\ b = 65^{\circ} 47, \\ c = 93^{\circ} 26^{\circ} 48^{\circ} \end{cases} \xrightarrow{\text{Req}} \begin{cases} A = 83^{\circ} 20^{\circ} (8), \\ B = 65^{\circ} 44^{\circ} 57, \\ C = 95^{\circ} 57^{\circ} 67^{\circ} \end{cases}$$

155. Case VI.

Given the transport of a spherical transport out and the sides.

By article 141, we have

Introducing B and split at Larth square last

MENSURATION.

156. Definition and Classification.

Mensuration is the art of calculating the values of geometrical magnitudes.

Mensuration is divided into two branches — Mensuration of surfaces and Mensuration of volumes.

MENSURATION OF SURFACES.

157. Unit of Superficial Measure.

A unit of superficial measure is a square each side of which is a linear unit.

Thus, according to the object to be accomplished, a square inch, a square foot, a square yard, an acre, etc., is the superficial unit taken.

158. Problem.

To find the area of a rectangle.

Let k denote the area, b the base, and a the altitude of a rectangle.

There are a rows of b superficial units each.

Since there are b superficial units in one row, in a such rows there will be a times b or ab superficial units.

The above demonstration applies only in case the base and altitude are commonsurable, or have a common unit

If the base and altitude are incommensurable, d is to the area by k', the base by b', and the altitude by a'. Then, since by (resometry any two rectangles are to each other as the products of their bases and altitudes, we have

L L' : ab : ab.
But L ab, . . . F ab.

159. Problem.

To find the area of a partire of me.

1. When the base and altitude are given.

Let k denote the area, b the base, and a the altitude of a parallelegram.

Since a parallelogram is equal to a pretangle, having the same base and altitude, and since the area of the rectangle is equal to the product of its base as a lititude.

1 1 k == ab.

2. When two sides and their included angle are given.

a seconda.

(2) 1 = le sin A.

160, Problem.

To find the area of a troungle.

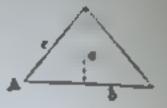
I. When the base and altitude are given.

bince a triangle is one-half the parallelogram having the same base and altitude, we have for the triangle,

(1) h = 1 ah

2 When two sides and their included angle are given.

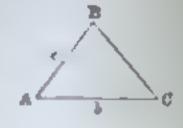
Since a triangle is one-half the parallelogram, having an equal angle and equal adjacent sides, we have for the triangle,



(2)
$$k = \frac{1}{2}bc \sin A$$
.

3. When two angles and a side are given

The third angle is equal to 180° minus the sum of the given angles.



Let, then, the angles and the side be given.

By the last case, we have

$$k = \frac{1}{2}bc \sin A$$
.

But $\sin B : \sin C :: b : c, ..., c = \frac{b \sin C}{\sin B}$.

Substituting this value of c, we have

(3)
$$k = \frac{b^2 \sin A \sin C}{2 \sin B}.$$

4 When two sides and an angle opposite one of them are given.

Let a and s be the given sides, and A the given angle.

In case of one or two solutions determined by article 72, find the value or values of C and B from the formulas,

$$\sin C = \frac{c \sin A}{a}, \text{ and } B = 180^{\circ} - (A + C).$$

Then, by (2), we have

(4)
$$k = \frac{1}{2} ac \sin B$$
.

5 When the three sides are given Let p - the perimeter a - b - c.
Then, by article 102, we have



(5)
$$k = \sqrt{\frac{1}{2}p(\frac{1}{2}p-a-\frac{1}{2}p-b-\frac{1}{2}p} r$$
.

6. When the perimeter and angles are given.

Let p be the perimeter, and A, E, and C the angles.

By article 98, (10), (11), (12),

 $\frac{1}{2}p^2 \tan \frac{1}{2}A \tan \frac{1}{2}B \tan \frac{1}{2}C = \frac{1}{2}p(\frac{1}{2}p - a)(\frac{1}{2}p - a)$

... (6)
$$k = \frac{1}{2}p^2 \tan \frac{1}{2}A \tan \frac{1}{2}B \tan \frac{1}{2}C$$

7. When the perimeter and radius of the carbed circle are given

Let p = a + b + c, and r be the radius of the inscribed circle.

ABC = BOC + AOC + AOB

ABC = k, $BOC = \frac{1}{2}ar$, $AOC = \frac{1}{2}br$, $AOB = \frac{1}{2}cr$.

$$k = \frac{1}{2}(a - b - c - 1)(1 + b - c - p)$$

161. Examples.

1. Find the area of a triangle whose base is 75 and altitude is 24 fb.

Peapertively, and their included angle is 50°; no. 1 and the area.

† 3. In a triangle, b = 100 ft, $A = 50^{\circ}$, $C = 60^{\circ}$; required the area.

Ans. 3529.9 sq. ft.

4. In a triangle, a = 40 yds., c = 50 yds., A = 40°; required the area. Ans. 998.18, or 232 83 sq. yds.

5. In a triangle, a=12 ft., b=15 ft., c=17 ft.; required k.

Ans. 87.75 eq. ft.

6. In a triangle the perimeter is 20 ft., and the angles are 50°, 60°, and 70°, respectively; required the area.

Ann. 18.85 sq. ft.

7. In a triangle the perimeter is 60 ft., and the radius of the inscribed circle is 5 ft.; required the area.

Ann. 150 eq. ft.

162. Problem.

To find the area of a quadrilateral.

I. When two opposite sides and the perpendiculars to these sides from the vertices of the angles at the extremities of a diagonal are given

Let b and b be two opposite sides, and a and a the perpendiculars to these sides from the vertices of the A^{L} angles D and B.

$$ABCD = ABD + DCB$$
.

$$ABCD = k_1 \quad ABD \sim \frac{1}{2} ab_1 \quad DCB = \frac{1}{2} a'b'.$$

• (1)
$$k = \frac{1}{2}ab + \frac{1}{2}ab'$$
.

Corollary 1.—If b' is parallel to b_i the quadrilateral becomes a trapezoid, a' = a, and (1) becomes

(2)
$$k = \frac{1}{2}a(b + b')$$
.

Corollary 2. — If b' = b, the trapezeid becomes a parallelogram, and (2) becomes

Corollary 3.— If b'=0, the trapez id becomes a triangle, and (2) becomes

(4)
$$k = \frac{1}{2}ab$$
.

2. When a diagonal and the perpendiculars to the diagonal from the vertices of the opposite and sixtles are given.

Let d denote the diagonal, and p and p' the perpendiculars.

 $ABCD \leftrightarrow ABC + ADC$.

ABCD = k, $ABC = \frac{1}{2}dp$, $ADC = \frac{1}{2}dp'$. $k = \frac{1}{2}d(p + p')$.

3. When the sides and a diagonal are given.

Let the areas of the triangles be denoted by k' and k", which are found by article 160, (5)



4. When the sides and one angle are given.

angle, and call the areas of the tri-

and the grand the diagonal.



Then, in the other triangle, we have the three sides, from which we find the area.

... (7)
$$k = k' + k''$$
.

5. When the diagonals and their included angle are

Let d and d' denote the diagonals p and q, r and s their segments, and A their included angle.



The angles at A are equal or sup-

$$BCDE = BAC + CAD + DAE + EAB.$$

BCDE = k, $BAC = \frac{1}{2}pa \sin A$, $CAD = \frac{1}{2}qa \sin A$.

 $DAE = \frac{1}{2}qr \sin A$, $EAB = \frac{1}{2}pr \sin A$.

$$\therefore k = \frac{1}{2}(ps + qs + qr + pr) \sin A.$$

But
$$ps + qs + qr + pr = (p + q)(r + s) = dd'$$
.

6. When the angles and two opposite sides are given.

Let a - BC, and b = AD. $E = 180^{\circ} - (B + C).$

The angles at A being supplementary, the same is true of the angles at D.

$$ABCD = BCE - ADE$$
, $ABCD = k$.

$$BCE = \frac{a^2 \sin B \sin C}{2 \sin E}, \quad ADE = \frac{b^2 \sin A \sin D}{2 \sin E}.$$

(9)
$$k = \frac{n^2 \sin B \sin C}{2 \sin E} - \frac{b^2 \sin A \sin D}{2 \sin E}$$

7. When three sides and their included angles are given.

Let a, b, and c be the given sides, and A and B their included angles.

$$ABCD = ABD + DBC$$
.

W B.

$$ABCD = k$$
, $ABD = \frac{1}{2}ab \sin A$.

Find B' and d, B'' = B - B', $DBC = \frac{1}{2}cd \sin B''$ (10) $k = \frac{1}{2}ab \sin A + \frac{1}{2}cd \sin B''$.

8. When the sides of a quadrilateral inscribed in a circle are given.

Let a, b, c, d be the given sides.

$$ACBD = ACB + ADB.$$

$$ACBD = k$$
, $ACB = \frac{1}{2}ab \sin C$.

$$ADB = \frac{1}{2} \alpha l \sin D = \frac{1}{2} \alpha l \sin C_1$$

since $D = 180^{\circ} - C$.

$$\dots k = \frac{1}{2}(ab + cd) \sin C.$$

$$\overline{AB}^2 = a^2 + b^2 - 2$$
 ab cos C, article 97.

$$\overline{AB}^2 = e^2 + d^2 - 2$$
 of $\cos D = e^2 + d^2 + 2$ of $\cos C$.

".
$$c^2 + d^2 + 2$$
 of $\cos C = a^2 + b^2 - 2$ ab $\cos C$.

$$\cos C = \frac{a^2 + b^2 - c^2 - d^2}{2(ab + cd)}.$$

$$\sin C = 1/1 - \cos^2 C$$
, Let $a = a + b + c + d$.

...
$$\sin c = \frac{2}{1} + \frac{1}{2} s = a + (\frac{1}{2} s - b) + (\frac{1}{2} s - c) + (\frac{1}{2} s - d)$$
.

· · (11)
$$k = 1 \left(\frac{1}{2}s - a\right) \left(\frac{1}{2}s - b\right) \left(\frac{1}{2}s - c\right) \left(\frac{1}{2}s - d\right)$$
.

163. Examples.

1. Two opposite sides of a quadrilateral are 35 rds and 25 rds, and the perpendiculars to these s les from the extremities of the diagonal are, respectively, 12 rds, and 16 rds.; required the area.

Ana. 410 sq. rds

2. Find the area of a trapezoid whose bases are 15 rds, and 20 rds, and whose altitude is 18 rds.

Ans. 315 sq. rds.

3. Two adjacent sides of a parallelogram are 30 nls and 40 rds, and their included a gle is 30°; required the area.

Ans. 600 sq. rds.

- 4. The diagonal of a quadricateful is 10 rds, and the two perpendiculars to the diagonal from the vertices of the opposite angles are 10 rds, and 15 rds, respectively; required the area.

 120, 500 eq. rds.
- 50 rds, and 60 rds, and the diagonal drawn from the intersection of the siles who had a side 30 rds, and 40 rds, is 70 rds.; required the area

.Ins. 1874.22 sq. rds.

6. The sides of a quadrilateral are 25 rds, 35 rds, 45 rds, 55 rds, and the angle included by the sides, whose lengths are 35 rds, and 45 rds, is 50°; required the area.

Ana, 927,47 sq. rds.

7. The diagonals of a quadrilateral are 30 rds and 40 rds, and their included angle is 30°; required the area.

Ans. 300 sq. rds.

8. The angles of a quadrilateral are 80°, 110°, 88°, 82°, the side included by the first and second of these angles is 25 rds., and the side included by the third and fourth angles is 45 rds.; required the area.

Ans. 4105,08 sq. rds.

9. Three sides of a quadrilateral are 20 rds., 30 rds., 40 rds., the angle included by the first and second is 60°, and between the second and third, 80°; required the area.

Ann. 593 58 sq. rds.

10. The sides of a quadrilateral inscribed in a circle are 40 rds., 50 rds, 60 rds., 70 rds.; required the area.

Ann. 2898 28 sq. rds.

11. The area of a parallelogram is 47 055 sq. ft, the sides are 6 ft. and 8 ft.; required the diagonal.

.ins. 9 ft., or 10.906 ft.

12. If the adjacent sides of a parallelogram are b and c, and their included angle A, find A and k when k is a maximum.

Ann. A 90°, k bc.

13. The sides and angles being expressed as in the last example, find A and k when k is a minimum.

Ans. $A = 0^{\circ}$ or 180° , k = 0.

14. If only two adjacent sides, b and c, of a parallelogram be given, prove that k is indeterminate between the limits 0 and bc.

15. Prove that the diagonals of a parallelogram divide it into four equal triangles.

164. Problem.

To find the area of an irregular polygon.

1. When the sides and diagonals from the same vertex are given.

2

The diagonals divide the polygon into triangles whose sides are given.

The areas of these triangles, k', k", k", are found by article 160, (5).

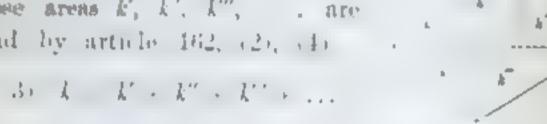
.: (1) $k \cdot k' + k'' + k''' + ...$

2 When the diagonals from the same vertex, and the perpendiculars to these diagonals from the opposite vertices are given.

(2)
$$k = \frac{1}{2}dp + \frac{1}{2}d'p' + \frac{1}{2}d'p'' + .$$

3. When the perpendiculars to a diagonal from the vertices of the opposite angles and the segments of the diagonal made by these perpendiculars are given.

The polygon is divided into right triangles and trapezoids, whose areas k', k', k'', ... are found by article 162, (2), (4)

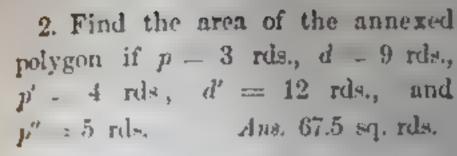


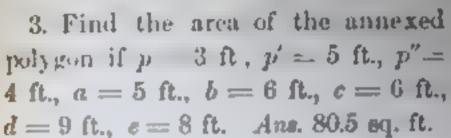
4. When one side of a figure is a straight line, and the opposite side is an irregular curve or broken line.

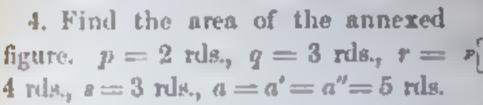
Let the strught line be divided into the parts a a a a a it is a latter perpendicular to part with y be considered trapezoids.

(4)
$$k = \frac{1}{2}a(p+q) + \frac{1}{2}a'(q+r) + \frac{1}{2}a''(r+s)$$
.
If $a' = a$ and $a'' = a$, (4) becomes,
(5) $k = \frac{1}{2}a(p+2q+2r+s)$.

165. Examples.













Ans 475 sq. rds.

166. Problem.

To find the area of a regular polygon.

I. When the perimeter and apothegm are given.

Let p be the perimeter, a the apothern, and s one side of the polygon.

$$k = \frac{1}{2}a_{1}a_{2} + \frac{1}{2}a_{3} + \frac{1}{2}a_{3} + \frac{1}{2}a_{4} + \dots$$

$$k = \frac{1}{2}a_{1}a_{2} + s + s + s + \dots$$

$$\vdots \quad (1) \quad k = \frac{1}{2}a_{1}.$$



2. When the value of each side and the number of sides are given.

Let a be one side, a the number of sides, a the apothem, and p the perimeter.

$$p = i \leftarrow DOB = \frac{360^{\circ}}{2 n} = \frac{180^{\circ}}{n}$$



S N 11

SURFACES.

163

... (2) $k = \frac{1}{2} ns^2 \cot \frac{150^{\circ}}{n}$.

If s 1, then (3) k $\frac{1}{2}n \cot \frac{180^{\circ}}{n}$.

From (3) calculate the areas of the regular polycons each of whose sides is 1, as given in the table subjoined

167. Table.

Triangle = 0.4330127.	Octagon 4.82	84271.
Square $= 1.0000000$	Enneagon 618	18242
Pentagon = 1.7204774.	Decagon 7.69	12088
Hexagon == 2,5980762.	Hendecagon= 9.36	56399,
Heptagon = 3.6339124.	Dodecagon = 11.19	61524.

168. Application of the Table.

Denoting the area of a regular polygon whose side is a by k, and the area of a similar polygon whose side is 1, as given in the table by k, and applying the principle that the areas of similar polygons are to each other as the squares of the homologous sides, we have the proportion,

$$k = k' = -1^2$$
. ... $k = k's^2$.

169. Examples.

1. What is the area of a regular hexagon each of whose sides is 6?

Ana. 93.5307432

2. What is the area of a regular pentagon each of whose sides is 10?

Ans. 172.04774.

3. What is the area of a regular decagon each of whose sides is 20?

Ans. 3077.68352.

4. What is the area of a regular dodecagon each of whose sides is 100?

Ana. 111961.524.

5. What is the area of a regular enneagon each of whose sides is 30?

Ann. 5563 64178.

170. Formulas for the Circle.

Let r be the radius, d the diameter, c the circumference, and k the area of a circle, then, by Geometry, we have

$$d = 2 \tau$$
, $c = \pi d$, $k = \frac{1}{2}\pi c$.

From which verify the following table of formulas:

1. r = 1 d	7.	$c=2$ τr .
2. $r = \frac{r}{2} - \frac{r}{2}$	8.	c = ±d.
3. r 1	9.	$c=21 k \tau$
4. $d = 2\tau$.	10.	$k = \pi r^2$.
5. d = \frac{c}{7}.	11.	$k=\tfrac{1}{4}\pi d^2.$
6. d 2	. 12.	$k = \frac{c^3}{4 \tau}$

171. Examples.

- 1. Given the radius of a circle = 10 rds; required d, c, and k.
- 2. Given the diameter of a circle = 20 rds.; required.

3. Given the circumference of a circle 150 rds; required r, d, and k.

4 Given the area of a circle = 1000 sq rds.; required r, d, and r

5. Find the diameter of a circle whose area is equal to that of a regular decagon, each side of which is 10 ft.

6. The radius of a circle is 10 ft., the diagonals of an equal parallelogram are 24 ft. and 30 ft.; required their included angle.

Ann. 60° 46′ 17″.

7. The radii of two concentric circles are r and r', find the area of the ring included by their circumferences.

1. $\tau(r+r')(r-r')$.

172. Problem.

To find the area of a sector of a circle.

Let a be the are of a sector, d the degrees in the are, the race on l k the area.



By Geometry, (1) $k = \frac{1}{2}m$.

Tr == the semi-circumference,

$$\frac{\pi r}{180}$$
 — the arc of 1°. ... $\frac{d\pi r}{180}$ = the arc of d°.
(2) $a = \frac{d\pi r}{180}$... (3) $k = \frac{d\pi r^4}{360}$

173. Examples.

1. Find the area of a sector whose are is 40° and radius is 10 ft.

Ans. 34.907 sq. ft.

2. Find the area of a sector whose are is 60° 24' 30' and radius is 100 rds.

Ans. 5271.64 sq. rds.

3. The area of a sector is 345 sq. ft, the radius is 20 ft; required the arc. Am. 98° 50′ 06".

The area of a sector is 1000 sq. rds., the are is 30° is; required the radius.

Ans. 61.01 rds.

174. Problem.

To find the area of a segment of a circle.

Let d be the degrees in the arc of the segment, r the radius, and k the arca.



By the last problem,

d=r² the area of the sector.

1 r2 sin d the area of the triangle.

$$k = \frac{d\pi r^2}{360} - \frac{1}{2} r^2 \sin d.$$

If d is great r than 180, sin d is negative, and the second term in the value of k becomes positive, as it should, since, in this case, the segment is equal to the corresponding so for plus the triangle.

175. Examples.

- 1. Find the area of the segment of a circle whose are is 36° and radius 10 ft.

 Ans. 2.027 sq. ft
- 2. Find the area of a segment whose chord is 36 ft.
 and radius 30 ft.

 Ans. 147,30 sq ft.
- 3. Find the area of a segment whose altitude is 36 rds and radius 50 rds.

 Ans 2545 85 sq rds

I The area of a segment is 2545.85 eq. rds, the robus is 50 rds; required the number of degrees in the are.

176. Problem.

To find the arm of an ellipse

Let a be the semi-major axis, and b the semi-minor axis.



Then, Ray's Analytic Geometry, article 446,

k nab.

177. Examples.

- 1. The semi-axes of an ellipse are 10 in, and 7 in; required the area.

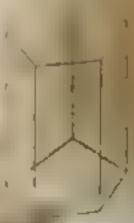
 Ans. 219,912 eq. in
- 2. The area of an ellipse is 125 sq. rds.; find the axes if they are to each other as 3 is to 2.

 Ans. 15.45; 10.30.

178. Problem.

To find the area of the entire surface of a right priest

Let p be the perimeter of the base, a the altitude, a one side of the base, k' the area of a polygon similar to the base, each side of whale is unity, article 167, and k the area of the entire surface.



ap the convex safece

2 Ks2 = the areas of the bases Article 168

$$k = ap + 2 k'a^2.$$

179. Examples.

- 1. What is the entire surface of a right prism whose altitude is 20 ft, and base a regular octagon each side of which is 10 ft.?

 Ans. 2565 68512 sq. ft.
- 2 What is the entire surface of a right hexagonal prism whose altitude is 12 ft., and each side of the base is 6 ft.?

 Ans. 619 0614864 eq. ft.
- 3. What is the entire surface of a right prism whose altitude is 15 in., and base a regular triangle each side of which is 3 in.?

 Ans. 142.7942286 eq. in.

180. Problem.

To find the area of the surface of a regular pyramid.

Let p be the perimeter of the base, a the slant height, s one side of the base, K and k as in the last problem.

> $\frac{1}{2}ap =$ the convex surface. $\mathcal{K}a^2 =$ the area of the base.

> > $k = \frac{1}{2}ap + k's^2.$



181. Examples.

1. What is the entire surface of a regular pyramid whose slant height is 12 ft., and base a regular transle each side of which is 5 ft.?

Ans. 100.82532 sq. ft.

2. What is the entire surface of a right pyramid whose slant height is 100 ft, and base a regular decagon each side of which is 20 ft.?

Ans 13077 68352 sq R.

182. Problem.

To find the entire surprise of a frustum of a right puramot.

Let p be the perimeter of the lower base p' the perimeter of the upper luse, a the slant height, a one side of the lower base, a one side of the upper base, if at i k as in Att 178.



 $\frac{1}{2}a(p + p') =$ the convex surface. $\frac{1}{2}a^2 =$ the area of lower base. $\frac{1}{2}a^2 =$ the area of upper base.

,
$$k = \frac{1}{2}a(p+p') + k'(s^2 + s'^2)$$
.

183. Examples.

- 1. What is the entire sart, e of a frustum of a pyramid whose slant height is 12 ft., and the basis regular decagons whose sides in S ft and 5 ft, respectively?

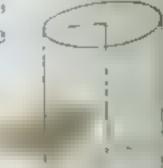
 And 1464.78458 sq. ft.
- 2. What is the entire surface of a frustum of a pyramid whose side at 10 it and 6 ft, reregular hexagons whose side at 10 it and 6 ft, respectively?

 Ans. 1073.338 sq. ft.

184. Problem.

To find the area of the entire surface of a cylinder

Let τ be the radius of the cylinder, a its altitude, and k the area of the entire surface.



$$2 \pi r^2$$
 the area of the bases.

...
$$k = 2 \pi r (a + r)$$
.

185. Examples.

1. What is the entire surface of a cylinder whose altitude is 6 ft. and radius 2 ft.?

Ann. 100 5312 sq. ft.

2 What is the entire surface of a cylinder whose altitude is 100 ft, and radius 20 ft.?

Anot, 15079 68 sq. ft.

186. Problem.

To find the area of the entire surface of a cone

Let r be the radius of the base of the cone, a the slant height, and k the area of the entire surface.

ara = the convex surface.

 $\pi r^2 =$ the area of the base.

 $\therefore k = \pi r (a + r).$



187. Examples.

- 1. What is the entire surface of a cone whose slant height is 10 ft. and radius 5 ft.? Ans. 235.62 sq. ft.
- 2. What is the entire surface of a cone whose altitude is 100 ft. and radius 25 ft.?

Ans. 10059.1675 eq. ft.

188. Problem.

To find the area of the entire surface of the frustum

Let r be the radius of the lower base, r be the 8. N. 15.

SURFACES

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ridans of the upper base, a the slant height, and k

-1 r r r') the convex surface.

Tr2 the area of the lower base.

the area of the upper base.

... $k = \pi[a(r+r') + r^2 + r'^2]$

189. Examples.

1. Find the entire surface of the frustum of a cone of which the radius of the lower base is 10 ft., the radius of the upper base is 6 ft., and slant height is 20 ft.

Ans. 1432.5696 sq. ft.

2. Find the entire surface of the frustum of a cone of which the radius of the lower base is 25 in., the radius of the upper base 12 in., and the slant height 36 in.

Anv. 45.8368 eq. ft.

190. Problem.

To find the area of the surface of a sphere.

Let r be the radius, d the diameter, c the circumference, and k the area. Then, by Geometry,

(1) $k = 4 \pi r^2$. (2) $k = \pi d^2$.

(3) $k = \frac{c^3}{\pi}$ (4) k = cd.

191. Examples.

1. The radius of a sphere is 10 ft; required the area.

2. The diameter of a sphere is 25 ft.; required the area.

Ana 1963 5 eq. ft.

3 The circumference of a sphere is 100 in.; required the area.

Ann. 3183 0914 sq. in.

4 The circumference of a sphere is 62 832, and diameter 20; required the area. Ann. 1256.64.

192. Problem.

To find the area of a zone.

By Geometry, the area of a zone is equal to the circumference of a great circle multiplied by the altitude of the zone.

Let a denote the altitude of the zone, r the radius of the sphere, and k the area of the zone.

k = 2 ma.

193. Examples.

1. What is the area of the torrid zone, calling its width 46° 55 and the earth a perfect sphere whose radius is 395c 5 mi.?

Ana. 783333333, eq. mi.

2. What is the area of the two frigid zones if the polar circles are 23° 28' from the poles?

Ans. 16270370, sq. mi.

3. What is the area of the two temperate zones?

Ans. 102109933, sq. mi.

194. Problem.

To find the area of a spherical triangle.

Let a = A + B + C, and $\frac{1}{2}\pi r^2 =$ the tri-rectangular triangle.

Then, by Geometry,

$$k = \frac{1}{2} \pi r^2 \left(\frac{s}{90^6} - 2 \right)$$



SURFACES.

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In this formula, $\frac{e}{10^{\circ}}$ 2 is to be regarded as an abstract number. Minutes and seconds are to be reduced to the decimal of a degree.

195. Examples.

1. Find the area of the spherical triangle whose angles are 60°, 80°, 100°, and the radius 3956,5 mi.

Ans. 16392592 sq. mi.

2 Find the area of a spherical triangle whose sides are 70°, 90°, 100°, respectively, and radius 100 in.

Ans. 10042 1928 sq. in.

196. Problem.

To find the area of a spherical polygon.

Let s be the sum of the angles s t a number of sides, k the area of the polygon, and the radius of the sphere.

Then, by Geometry,

$$k=\tfrac{1}{2}\pi r^2\left[\frac{s}{90}\circ-2\left(n-2\right)\right]$$

197. Examples.

The sum of the angles of a spherical hexagon is 800°, the radius is 100 ft , required the area

Abs, 43963, sq. ft

2. Each angle of a spherical pentagon is 120°, the radius is 50 ft.; required the area. Ann. 2618. sq. ft.

3. The angles of a spherical polygon are 90°, 100°, 110°, 150°, respectively, the radius is 10 ft; required the area.

100, 157.08 sq. ft.

4. Each angle of a spherical decagon is 150°, the radius is 1 ft.; required the area. Ans. 1.0472 ft.

198. Problem.

To find the area of the surface of a regular polyhedron.

Let e be one edge, n the number of faces, k the area of a polygon whose side is 1, and similar to one face, and k the area of the entire surface.

Re2 = the area of one face. Article 168.

 $k = nke^{1}$.

199. Examples.

- 1. What is the area of the entire surface of a tetrahedron whose dge is 10 ft? Ans. 173,20508 eq. ft.
- 2. What is the area of the entire surface of a hexahedron whose edge is 5 ft.? Ans. 150 sq. ft.
- 3. What is the area of the entire surface of an octahedron whose edge is 20 ft.? Ans. 1385,64064 sq. ft.
- 4. What is the area of the entire surface of a dodec ahedron whose edge is 15 in? Ans. 32 25895 sq A.
- 5. What is the area of the entire surface of an icosahedron whose edge is 100 in.? Ans. 601.4065 sq. ft.

MENSURATION OF VOLUMES.

200. Problem.

To find the volume of a prism.

Let k be the area of the base, a the altitude, and v

v = ak

201. Examples.

1. What is the volume of a regular bexagonal prism whose altitude is 20 ft., and each side of the base 10 ft.?

Ans. 5196,1524 cu. ft.

2. What is the volume of a triangular prism whose altitude is 6 ft., and the sides of its base 3 ft., 4 ft., and 5 ft., respectively?

100, 36 cu. ft.

3. What is the volume of a regular octagonal prism whose altitude is 120 ft., and each so of the base 20 ft.?

Ans. 1.764 5008 cu. ft.

202, Problem.

To find the volume of a pyramid.

Let k be the area of the base, a the dilitude, and v the volume. $v = \frac{1}{2}ak$.

203. Examples.

1. What is the volume of a pyramid whose altitude is 15 ft, and whose base is a regular heptagon each side of which is 5 ft?

Ans. 451 23905 cu ft.

2. What is the volume of a pyramid whose altitude is 21 in., and whose base is a triangle each side of which is 30 in?

Ans. 2727.98 cu. in.

204. Problem.

To find the volume of the frustum of a pyramid.

Let k and k_1 be the areas of the bases, a the altitude, and v the volume. Then, by Geometry,

(1)
$$v = \frac{1}{3} a (k + k_1 + 1 - k k_1).$$

If the bases are regular polygons whose sides are and d, we shall have, by article 168, $k = k'a^2$, and $k_1 = k'd^2$, in which k' is given in the table of article 167, and (1) becomes

(2)
$$v = \frac{1}{3} a (s^2 + s'^2 + ss') k'$$
.

205. Examples.

1. What is the volume of the frustum of a pyramid whose altitude is 9 ft., and whose bases are regular triangles, one side of the lower being 8 ft., and one side of upper, 5 ft.?

Ans. 167.576 cu. ft

2. What is the volume of the frustum of a pyramid whose altitude is 27 in., and the bases regular hexagons, the sides of which are 10 in. and 6 in., respectively?

Ans. 4583.0064 cu. in.

206. Problem.

To find the volume of a cylinder.

Let r represent the radius, a the altitude, and r the volume. $v = a\pi r^2$.

207. Examples.

2. What is the volume of a cylinder whose altitude is 25 ft., and radius 4 ft?

Ana. 1256.64 cu ft.

3 OLUMES.

208. Problem.

To a d the volume of a cone.

let r be the radius of the base, a the altitude, and r the volume.

v = \ \ a7r2.

209, Examples.

1 What is the volume of a cone whose altitude is 21 m, and radius 10 in.? Ans. 2199.12 cu. in.

2 What is the volume of a cone whose altitude is 30 ft, and radius is 10 ft.?

Ans. 31416, cu. ft.

210. Problem.

To find the volume of the frustum of a cone.

Let r and r' be the radii of the bases, a the altitude, and v the volume.

$$v = \frac{1}{3} a \pi (r^2 + r'^2 + rr')$$

211. Examples.

1. What is the volume of the frustum of a cone whose altitude is 15 ft., and the radii of whose bases are 9 ft. and 4 ft, respectively? Ans. 2089.164 cu. ft.

2. How many barrels will that eistern contain whose altitude is 8 ft, the diameter at the bottom 4 ft, and at the top 6 ft?

Ans. 37.8 bbl.

212. Formulas for the Sphere.

Let r be the radius, d the diameter, c the circumference, k the area of the surface, and c the volume of a sphere, then, by Geometry, we have

d = 2r, $c = \pi d$, $k = 4\pi r^2$, $v = \frac{1}{3}rk$.

From which verify the following table of formulas:

1. $r = \frac{1}{2}d$.	11. $c = 1 \overline{nk}$.
2. $r = \frac{c}{2\pi}$.	12. c = 1 6 v = 2.
3. $r = \frac{1}{2} \sqrt{\frac{k}{\pi}}$.	13. k = 4 7r2.
4. $r = \frac{1}{2} \frac{3}{4} \frac{6 \dot{r}}{\pi}$.	14. $k = \pi d^2$.
5. d 2 r	15. $k = \frac{c^2}{\tau}$
6. d ·	16. $k = f^{*} \overline{36 \pi r^{2}}$.
7. $d = \sqrt{\frac{k}{\pi}}$.	17. v 1 1775.
8. d 161.	18. v 1 πd*.
9. c 2 7r.	19. $v = \frac{c^3}{6^{\frac{1}{2}}}$.
10. σ π./	$20. c = \frac{k}{6} \sqrt{\frac{\vec{k}}{\pi}}.$

213. Examples.

1. Calling the diameter of the earth 7913 mi, and the diameter of the sun 856,000, find the ratio of their surfaces, also the ratio of their volumes.

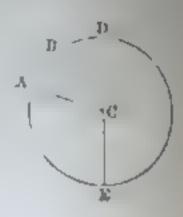
2. What is the volume of the shell of a hollow sphere whose radius is 5 ft. 4 in., and the thickness of the shell 3 ft 6 in?

Ans. 1951 1081 cu ft

214. Problem.

To find the volume of a spherical sector,

A spherical sector is the volume generated by the revolution of any circular sector, ABC, about any diameter, DE. By Geometry, the volume of a spherical sector is equal to the zone which forms its base, unitiplied by one-third of the radius.



Let a be the altitude of the zone, and r the radius.

.
$$v = \frac{a}{3} \pi r^2 a$$
.

215. Examples.

1. The altitude of the zone which forms the base of a sector is 6 ft., the radius is 12 ft; required the Ann. 1809 5616 cu. ft. volume.

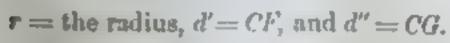
2. The angle BCD, in the diagram of last article, is 20°, ACB is 35° , $\tau = 20$ ft.; required the volume. Aus. 6134.25 cu. ft.

216. Problem.

To find the volume of a upherical segment.

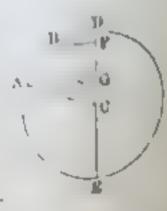
A spherical segment is the portion of a sphere included between two parallel planes.

Let r' = BF perpendicular to DE, and r'' = AG perpendicular to DE.



v = the vol. generated by ABFG.

 $\mathbf{z}'=$ the vol. generated by $ABC=\frac{2}{3}\pi r^2a$.



v''= the vol. generated by $BFC=\frac{1}{3}d'\pi r'^2$. the vol. generated by $AGC = \frac{1}{2}d''\pi r''^2$. v - v' + v'' + v''

The sign of r" is - or + according as AG is on the same or opposite side of the center as BF.

...
$$v = \frac{1}{3}\pi(2 ar^2 + d'r'^2 + d''r''^2)$$
.

217. Examples.

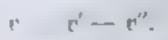
1. r = 12 in , r' = 3 in., r'' = 10 in.; required r.

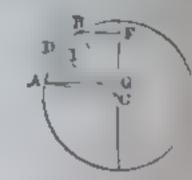
2. Two parallel planes divide a sphere whose diameter is 36 in, into three equal segments; required the altitude of each. zbs. 13.93 in.; 8.14 in ; 13.93 in.

218. Problem.

To find the volume generated by the revolution of a circular segment about a diameter exterior to it.

Let v = vol. generated by ADB. v' = vol. generated by ADBC. v''= vol. generated by ABC.





Let a = FG, c - AB, p = CI, perpendicular to AB. $v' = \frac{2}{3}\pi a v^3, \quad v'' = \frac{2}{3}\pi a p^2.$ • • • • • • • • $\eta \pi a (r^2 - p^2) = \frac{1}{8} \pi a c^2$. $v = \frac{1}{8} \pi a c^2$

219. Examples.

1. a 5 in , c 8 in.; find v. Ans. 167.552 cu. In.

2 A sphere 6 in, in diameter is bored through the center with a 3-meh auger; required the volume rereaining. Ana, 73,457 eu. 10.

Reprove that the volume generated by the segment whose attracte is a and chord case to the sphere whose diameter is case and

4. Prove that if c is parallel to the diameter about which it is revolved, the volume generated by the squart is equal to the volume of a sphere whose diameter is c.

220. Problem.

To find the volume of a waige.

The base is a rectangle, the sides are trapezoids, the ends, triangles.

Let c be the edge, i the length of base, b the breadth of base, and a the altitude.

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Passing planes through the extremities of the edge perpendicular to the base, we have a triangular prism and two pyramids. These pyramids may full within or without the wedge, or one or both of the pyramids may vanish

But in all cases the formula is the same.

 $\frac{1}{2}abc =$ the volume of the prism. $\frac{1}{2}a(l-c)b =$ the volume of the pyramids.

...
$$v = \frac{1}{6} ab (2 l + e).$$

221. Examples.

1. The edge of a wedge is 6 in, the altitude 12 in, the length of base 9 in, and the breadth of base 5 in; what is the volume?

Ans. 240 cu. in.

2. The edge of a wedge is 20 ft, the altitude 24 ft, the length of base 15 ft., the breadth of base 10 ft; what is the volume?

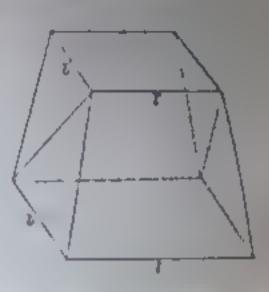
Ann. 2000 cu. ft.

222. Problem.

To find the volume of a rectangular prismoid.

The bases are parallel rectangles, the other faces are trapezoids

Let *l* and *b* be the length and breadth of the lower base, *l* and *b'* the length and breadth of the upper base, and *a* the altitude.



Passing the plane as indicated, the prismoid is divided into two wedges.

 $\frac{1}{2}ab'(2l'+l') =$ the vol. of wedge whose base is bl. $\frac{1}{2}ab'(2l'+l) =$ the vol. of wedge whose base is bl'. $\frac{1}{2}ab'(2l'+l) = \frac{1}{2}a[b(2l+l') + b'(2l'+l)].$

223. Examples.

1. The length and breadth of the lower base of a rectangular prismoid are 25 ft. and 20 ft., the length and breadth of the upper base are 15 ft. and 10 ft. and the altitude is 18 ft.; what is the volume?

Ans. 5550 cu. ft.

2. The length and breadth of the lower base of a rectangular prismoid are 15 yds, and 10 yds, the length and breadth of the upper base are 9 yds and 6 yds, and the altitude is 18 yds.; what is the volume?

Ana 1764 cu yds.

FOLUMES.

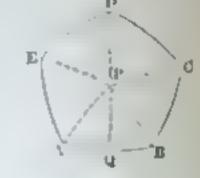
204. Problem.

To but the their angle included by the faces of a

the center shall be at any vertex of the polyhedron

The faces of the polyhedral angle will intersect the strace of the sphere in a regular polygon whose sides in a sure the plane angles that include the polyhedral angle, and whose angles are each equal to the required dihedral angle.

Let ABCD be such a polygon, P the pole of a small circle passing through A, B, C, D, E. Join P with the vertices and with the middle of AB by area of great circles.



Let n denote the number of sides of the polygon, s =one side, and A =a dihedral angle

...
$$APQ = \frac{360^{\circ}}{2 \text{ n}} = \frac{180^{\circ}}{n}$$
, and $AQ = \frac{1}{2} s$.

By Napier's circular parts, we have

$$\sin (90^{\circ} - APQ) = \cos AQ \cos (90^{\circ} - PAQ).$$

or
$$\sin (90^{\circ} - \frac{180^{\circ}}{n}) = \cos \frac{1}{2}s \cos (90^{\circ} - \frac{1}{2}A).$$

or
$$\cos \frac{180^{\circ}}{n} = \cos \frac{1}{2} s \sin \frac{1}{2} A$$
.

$$a \sin \frac{\pi}{2} A = \frac{\cos \frac{1}{n} 180^{\circ}}{\cos \frac{1}{2} \pi}.$$

In the Tetrahedron, n=3, and $s=60^{\circ}$,

...
$$\sin \frac{1}{2}A = \frac{\cos 60^{\circ}}{\cos 30^{\circ}}$$
 ... $A = 70^{\circ} 31' 42''$

In the Hexahedron, n - 3, and s 90°,

$$\sin \frac{1}{2} A = \frac{\cos 60^{\circ}}{\cos 45^{\circ}}$$
. ... A 90°,

In the Octahedron, n=4, and $s=60^{\circ}$.

$$\sin \frac{1}{2} A = \frac{\cos 45^{\circ}}{\cos 30^{\circ}}$$
. ... $A \sim 100^{\circ} 28' 18''$.

In the Dodecahedron, n = 3, and $s = 108^{\circ}$,

..,
$$\sin \frac{1}{2} A = \frac{\cos 60^{\circ}}{\cos 54^{\circ}}$$
. .. $A = 116^{\circ} 33' 54''$.

In the Icosahedron, n = 5, and $s = 60^{\circ}$,

$$\therefore \sin \frac{1}{2} A = \frac{\cos 36^{\circ}}{\cos 30^{\circ}}.$$
 $\therefore A = 135^{\circ} 11' 23''.$

225. Problem.

To find the volume of a regular polyhedron.

If planes be passed through the edges of the polyhedron and the center, they will bisect the dihedral angles and divide the polyhedron into as many pyramids as it has faces. The faces will be the bases of the pyramids, the center will be their common vertex, the line drawn from the center of the polyhedron to the center of any base will be perpendicular to the base, and will be the altitude of the pyramid.

From the foot of the perpendicular draw a perpendicular to one side of the base, and join the foot of this perpendicular with the center. We thus have a right triangle whose perpendicular is the altitude of the pyramid, the base the apothem of one face of the polyhedron, the angle opposite the perpendicular one half the dihedral angle of the polyhedron.

Let p be the perpendicular, a the apothem of one face, \(\frac{1}{2}\). I one-half of a dihedral angle, n' the number of sides of one face, and c one edge.

$$p = a \tan \frac{1}{2}A$$
, $a = \frac{1}{2}e \cot \frac{1}{2}180^{\circ}$. Article 166.
... $p = \frac{1}{2}e \cot \frac{1}{2}180^{\circ} \tan \frac{1}{2}A$.

Let F, n, and k be the same as in article 198. Then, $\frac{1}{2}pk =$ the volume of the polyhedron.

... $v = \frac{1}{2} n E e^3 \cot \frac{1}{2} 180^{\circ} \tan \frac{1}{2} A$.

Let e = 1, and verify the table subjoined:

AAAN TURKIL	226,	Table
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Names.	Surfaces.	1' dume
Tetrahedron	1.7320508	0 1178513
Hexahedron	6.0000000	1 - HXKKO
Octahedron	3.4641016	0.1714045
Dodecahedron	20.6457288	7 (6)11189
Iosahedron	8.6602540	2.1×16950
_		

227. Application of the Table.

Let v and v denote similar regular polyhedrons whose edges are 1 and c, respectively. Then we have

$$v':v:1^3-e^3$$
, $v=v'e^3$.

228. Examples.

- 1. What is the volume of a tetrahedron whose edge is 10 ft.?

 Ans. 117,8513 cu. ft.
- 2. The volume of a hexahedron is 134217728 cu. in.; what is its surface?

 Ans. 1572864 sq. in.

SURVEYING.

229. Definition and Classification.

Surveying is the art of laying out, measuring, and dividing land, and of representing on paper its boundaries and peculiarities of surface.

There are three branches—Plane, Geodowe, and Topo-

Plane surveying is that branch in which the portion surveyed is regarded as a plane, as is the case in small surveys.

Geodesic surveying is that branch in which the curvature of the surface of the earth is taken into consideration, as is the case in all extensive surveying.

Topographical surveying is that branch in which the slope and irregularities of the surface, the course of streams, the position and form of lakes and ponds, the situation of trees, marshes, rocks, buildings, etc., are considered and delineated.

INSTRUMENTS.

230. Classification.

The instruments employed in surveying may be classed as Field instruments and Plotting instruments.

The principal field instruments are the class and mily pins, marking tools, field-book and pencil, the second seco

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conducts, the solar compass, the transit compass, the letel. and the throdolite.

The principal plotting instruments are the dicidera the reter and triangle, parallel rulers, the diagonal scale the semicorcular protractor.

231. The Chain and Tally Pins.

The chain is 4 rods or 66 feet in length, and is divided into 100 links, each equal to 7.92 inches.

After every tenth link from each end is a piece of brass, notched so as to indicate the number of links from the end of the chain, thus facilitating the counting of the links.

A half-chain of 50 links is sometimes used, especially in rough or hilly districts.

The tally pins are made of iron or steel, about 12 inches in length and one-eighth of an inch in thickness, heavier toward the point, with a ring at the top in which is fastened a piece of cloth of some conspienous color.

These pins are conveniently carried by stringing them on an iron ring attached to a belt which is passed over the right shoulder, leaving the pins suspended at the left side.

In Government surveys cleven tally pins are used.

232. Marking Tools,

A surveying party will need an ar for cutting notches, cutting and driving stakes and posts; a spade or muttock for planting or finding corners; knices, or other tools, for cutting letters or figures; and a fite and whetstone for keeping the tools in order.

233. Field-Book and Pencil.

In ordinary practice one field book will be sufficient: but in surveying the public lands, four different books are required -one for meridian and base likes, another for standard parallels or cornection lines another for exterior or township lines, and another for submy ston or section lines, as designated on the title-page

A good pencil, number 2 or 3, well sharp ned, should be used, so that the notes may be legable

A temporary book may be used on the ground, and the notes taken with a pencil. These notes can then be carefully transcribed with pen and ink anto the permanent field-book.

234. The Magnetic Compass.

The vernier magnetic compass is exh. test in the drawing on page 150,

The needle turns on a pivot at the center, and settles in the magnet meridian.

The compass circle a divided, on its upper surface. to half-degrees, numit | from 0° to 90° each site of the line of zeros

The sight standards are firmly fastened at right angles to the plate by screws, and have slits cut through nearly their whole length, terminated at in break by aperture to a which the elect toward which this night or tal can be readly fund

Two spirit levels it i thank to cach other are attached to the par

Pangent scales are males on the right and in of the morth night standard, the one on the mile laing used in taking angles of elevation, and the one on the left in taking angles of depression.

Eye-pieces are placed on the right and left sides of the south sight standard—the one on the right near the bottom, the one on the left near the top—each on a level, when the compass is level, with the zero of its tangent scale. These eye-pieces are centers of ares tangent to the tangent scales at the zero point.

The vernier is a scale movable by the side of another scale, and divided into parts each a little greater or a little less than a part of the other and having a known ratio to it. In the drawing the vernier is represented on the plate near the south sight.

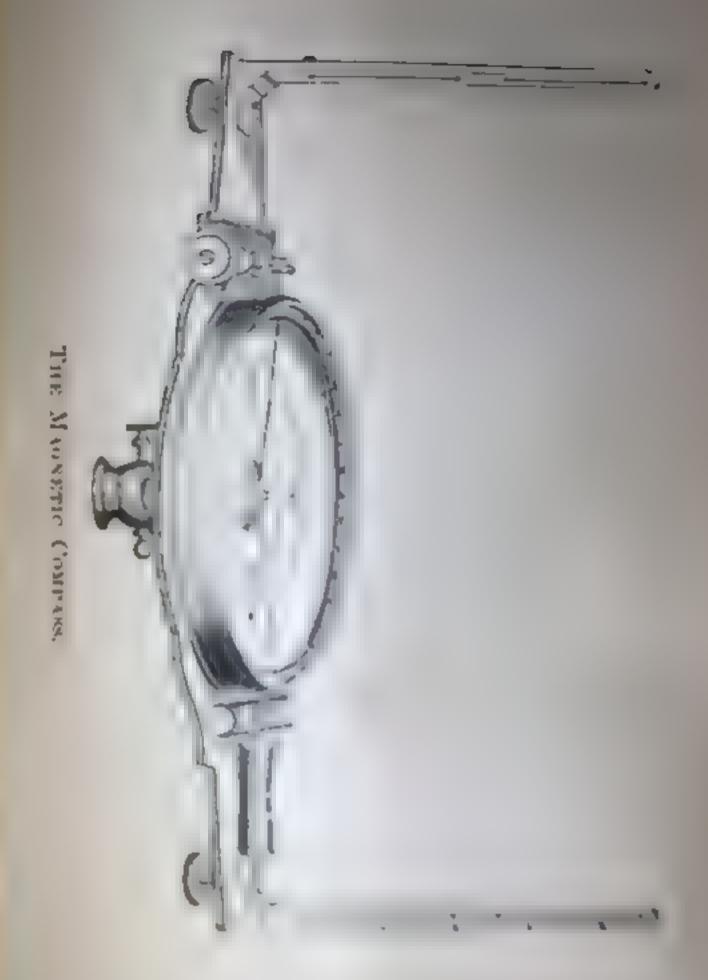
The needle lifter is a concealed spring, moved from beneath the main plate, by which the needle may be lifted to avoid blunting the point of the pivot in transporting the instrument.

The out-keeper is a small dial plate, having an index turned by a milled head, and is used in keeping tally in chaining.

The ball spindle is a small shaft, slightly conical, to which the compass is fitted, having on its lower end a ball confined in a socket by a light pressure, so that the ball can be moved in any direction in leveling the instrument.

The clamp screw is a screw in the side of the hollow cylinder or socket, which fits to the ball spindle, by which the compass may be clamped to the spindle in any position.

A spring catch, fitted to the socket, slips into a groove when the instrument is set on the spindle, and secures it from slipping from the spindle when carried.



The Jacob staff is a single staff to support the comtion about 5} feet long, having at the upper end to bell and seeket joint, and terminating at the lover or long a sharp steel point, so as to be set to by in the ground

The tripod is a three-legged support sometimes used test ad of the Jacob staff.

235. Adjustments of the Compass.

- 1. To adjust the level.—Bring the bubbles to the center of the tubes by pressing the plates so as to turn the ball slightly in its sockets. Turn the compass half-way round, and if either bubble runs to one end of its tube, that end is the higher. Loose the screw under the lower end, and tighten the one at the higher end till the bubble is brought half-way back. Level the plate again, and repeat the operation till the bubble will remain in the center during an entire revolution of the compass.
- 2. To adjust the sights.—Observe through the slits a fine thread made plumb by a weight. If both sights do not exactly range with the thread, file a little off the under surface of the highest side
- 3. To adjust the needle.—Bring the eye nearly in the same plane with the graduated circle, move with a splinter one end of the needle to any division of the circle, and observe whether the other is discovered with the division 180° from the first; if so, the needle is said to cut opposite degrees; if not, bend the center pin with a small wrench about one-eighth of an inch below the point, till the ends of the mode cut opposite degrees. Hold the needle in the same direction, turn the compass half-way round, and again see

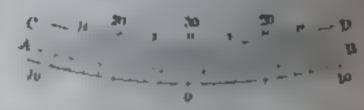
whether the needle cuts opposite degrees; if not, correct half the error by bending the needle, and the remainder by bending the center pin, and repeat the operation till perfect reversion is secured in the first position.

Try the needle in another quarter, and correct by bending the center pin only, since the needle was straightened by the previous operation, and repeat the operation in different quarters.

The adjustments are made by the maker of the instrument, but the instrument can be re-adjusted by the surveyor when necessary.

236. Nature of the Vernier.

Let the arc or limb AB, on the main plate of the instrument, be graduated to one-half



degrees or 30', numbered each way from 0 at the middie; and let the vernier CD, attached to the compass box, which is movable around the main plate, be so graduated that 30 spaces of the vernier shall be equal to 31 spaces of the limb, that is, equal to 31 - 30'; then 1 space of the vernier will be equal to 31', and the difference between one space of the vernier and one space of the limb will be 31'—30'=1'

The vernier is numbered in two series: the lower, nearer the spectator, who is supposed to stand at the south end of the instrument, is numbered 5, 10, 15, each way from 0; the upper series has 80 above the 0, from the observer, and 20 each way above the 10 of the lower series.

Let, now, the O points of the vernier and had co-incide; then, if the vernier be moved forward I to

the right, aboth is done by means of a tangent screw, the first division line of the vernier at the left of its 0 will coincide with the first division line of the 1 mb at the bit of its 0; if the vernier be moved forward 2 to the right, then the second division line of the vernier at the left of its 0 will coincide with the second division line of the second division line of the limb at the left of its 0.

If the vernier be moved to the right so that its fifteenth division line at the left of its 0 shall coincide with the fifteenth division line of the limb at the left of its 0, the vernier will have moved forward 15'.

over 15' is found by reading the division line, in the vernier, which coincides with a division line of the limb, from the upper row of figures on the vernier, on the other side of 0, and so on, up to 30', when the 0 of the vernier will coincide with the first division line from the 0 of the limb.

over 30', up to 15' and then to 30' is found as before.

If the 0 of the vernier coincides with a division line of the limb, the reading of the division line of the limb will be the true reading

If the 0 of the vernier has passed one or more division lines of the limb, and does not coincide with any, read the limb from its 0 point up to its division next preceding the 0 of the vernier; to this add the reading of the vernier, and the sum will be the true reading.

If the vernier be moved to the left, the minutes must be read off on the vernier scale to the right

Sometimes the spaces of the vernier are less than the spaces of the limb; then if the vernier be moved

either way, the minutes must be read off the same way from the 0 of the vernier. Verniers may be so graduated as to read to any appreciable angle; but the graduation which reads to minutes is the most common

237. Uses of the Vernier.

1. To turn off the variation.— Let the instrument be placed on some definite line of an old survey, and the tangent screw be turned till the nealle indicates the same bearing for the line as that given in the field notes of the original survey.

Then will the reading of the lumb and vernier indicate the variation.

- 2. To retrace an old survey. Turn off the variation as above, and screw up the clamping nut beneath, then old lines can be retraced from the original notes without further change of the vernier.
- of the needle being known, not simply its change since a condate, move the vernier to the right or left adding as the variation is west or cast, till the given variation is turned off, some up the clamping nut beneath, and turn the compass till the needle is made to cut zeros, then will the line of sights indicate a true meridian.

Such a change in the position of the vermer is heressary in subdividing the public lands, after the principal lines have been truly run with the solar compass.

4. To read the needle to minutes. — Note the degrees given by the needle, then turn back the compass on le, with the tangent screw, till the nearest whole degree mark coincides with the point of the wedle the space

passed over by the vermer will be the inmutes which, will do the acgrees, will give the reading of the new to not do

The operation is simplified when the 0 of the vertor is that made to coincide with the 0 of the limb, torwise the difference of the two readings of the variety must be taken.

238. Uses of the Compass.

1. To take the bearing of a line. Place the compass on the line, turn the north end in the direction of the course, and, standing at the south end direct the sights to some well-defined object, as a flag-staff, in the course. Read the bearing from the north end of the needle, which can be done accurately to quarter-degrees by observing the position of the point of the needle, since the compass circle is divided into half-degrees.

It will be observed that the letters E and W, on the face of the compass, are reversed from their true position. This is as it should be; for if the sights are turned toward the west, the north end of the needle is turned toward the letter W. If the north end of the needle is turned toward E, the sights will be turned toward the east. If the north end of the needle point exactly to either letter E or W, the sights will range east or west.

In general, to guard against error, let the surveyor turn the letter S toward himself, and read the arc cut off by the north end of the needle from the nearest zero of the compass circle. If, for example, the nearest of is at S, and the north end of the needle is turned toward E, cutting off 25° from this 0, then the course is S 25° E

If it is desired to find the bearing to minutes, the vermer must be used

- 2. To run from a given point a line having a given bearing.—Place the compass over the point, and turn it so that the reading of the needle shall be the given bearing; the line of sights observed from the south end of the compass will be the required line.
- 3. To take angles of elevation. Level the compass, bring the south end toward you, place the eye at the eye-piece on the right side of the south sight, and, with the hand, fix a card on the front surface of the north sight, so that its top edge shall be at right angles to the divided edge and coincide with the zero mark; then, sighting over the top of the card, note upon a flag-staff the height cut by the line of sight, move the staff up the elevation, and carry the card along the sight until the line of sight again cuts the same height on the staff, read off the degrees and half-degrees passed over by the card, and the result will be the angle required.
- 4. To take angles of depression.—Proceed in the same manner, using the eye-piece and scale on the opposite sides of the sights, and reading from the top of the standard

239. Surveyor's Transit.

The Surveyor's transit exhibited in the drawing on page 197 is, in fact, a transit theodolite, combining the advantages of the ordinary transit and the theodolite

The vernier plate, carrying two horizontal vertex, two spirit levels at right angles, the telescope and attachments, moves around a circle graduated to labelegrees, so that, by the vernier, horizontal unclassion be taken to minutes, and any variation turned of

The telescope and its attachments, the clamp and the air the verm if circle, the level and the sights, we to this festionent a great advantage over the circle of the circle of the circle.

The cross wires, two fine fibers of spider's web, extailing across the tube at right angles, intersect in a mat which, when the wires are adjusted, determines the optical axis or line of collimation of the telescope, and enables the surveyor to fix it upon an object with and epistesion

The clamp and tangent screw consist of a ring encircling the axis of the telescope, having two projecting arms—the one above, slit through the middle, holding the clamp screw; the other, longer, connected below with the tangent screw.

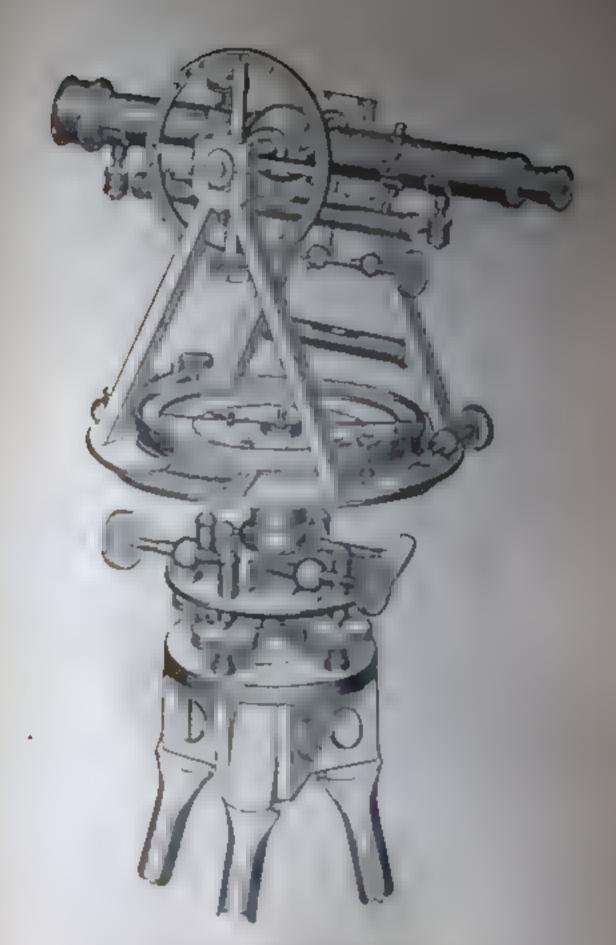
The ring is brought firmly around the axis by means of the clamp screw, and the telescope can be moved up or down by turning the tangent screw

The vertical circle, graduated to half-degrees, is attached to the axis of the telescope, and, in connection with the vernier, gives the means of measuring vertical angles to minutes with great facility

The level attached to the telescope of the surveyor to run horizontal lines, or to find the difference of level between two points.

Sights on the telescope are useful in taking backsights without turning the telescope, and in sighting through bushes or woods

Sights for right angles attached to the plate of the instrument, or to the standards supporting the telescope, afford the means of laying off right angles, or running out offsets without changing the position of the instrument.



SURVEYOR'S TRANSIT.

240. Adjustments.

1 The levels are adjusted in the same manner as those of the compass, and when adjusted should keep that position if the two plates are clamped together and turned on a common socket.

2 The needle is adjusted as in the compass.

3. The line of the collimation is adjusted by bringing the intersection of the wires into the optical axis of the telescope, which is accomplished as follows:

it carefully, then, having brought the wires into the focus of the eye-piece, adjust the object glass on some well defined object, as the edge of a chimney, at a distance of from two to five hundred feet. Determine whether the vertical wire is plumb by clamping the instrument firmly to the spindle, and applying the wire to the vertical edge of a building, or observing if it will move parallel to a point a little to one side; if it does not, loosen the cross-wire serches, and, by the pressure of the hand on the head outside the tube, move the ring within the tube to what the wires are attached, gently around till the error is corrected.

The wires being thus made respectively horizontal and vertical, fix their point of intersection on the object selected, clamp the instrument to the spindle, and, having revolved the telescope, find or place some object in the opposite direction, at about the same distance from the instrument as the first object.

Great care should be taken in turning the telescope not to disturb the position of the instrument upon the spindle.

Having found an object which the vertical wire bisects, unclamp the instrument, turn it half-way round, and direct the telescope to the first object selected, and having bisected this with the wires, again clamp the instrument, revolve the telescope and note if the vertical wire bisects the second object observed; if so, the wires are adjusted, and the points bisected are, with the center of the instrument, in the same straight line.

If the vertical wire does not bisect the second point the space which separates this wire from that point is double the distance of that point from a straight line drawn through the first point and the center of the instrument, as is shown thus:



Let A represent the center of the instrument, BC the line on whose extremities, B and C, the line of collimation is to be adjusted, B the first object, and D the point which the wires bisected after the telescope was made to revolve on its axis. The side of the telescope which was up when the object glass was directed to B, is down when the object glass is turned toward D. When the telescope is unclamped from its spindle and turned half-way round its vertical axis, and again directed to B, the side of its tube which was down when the object glass was first directed to B will now be up. Then clamping the instrument, and revolving the telescope about its axis, and derecting it toward D, the side of its tube which was down when the object glass was first turned toward D will now be up, or the telescope will virtually have revolved about its optical axis, and the vertical was will appear at E as far on one side of C as D is on the other side.

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To move the vertical wars to its true position, turn to captain head screws on the sales of the telescope, na aberage that the eye piece inverts the position or the wire, and, therefore, that in loosening one of the seres and in tightening the other the operator mast proceed as if to increase the error. Having neved each the vertical wire, as nearly as can be 1 street, so as to bisect the space ED, unclamp the instroppert, direct the telescope as at first, so that the cross wires breet B, proceed as before, and continue the operation till the two points D and E coincide at C.

4. The standards must be of the same height, in order that the wires may trace a vertical line when the telescope is turned up or down. To ascertain this, and to make the correction, proceed as follows:

Having the line of collimation previously adjusted, set the instrument in a position where points of observation, such as the point and base of a lofty spire, can be selected, giving a long range in a vertical direction.

Level the instrument, fix the wires on the top of the object, and clamp to the spindle; then bring the telescope down till the wires bisect some good point, either found or marked at the base; turn the instrument half around, fix the wires on the lower point, clamp to the spindle, and raise the telescope to the highest object, and if the wires bisect it, the vertical adjustment is effected.

if the wires are thrown to one side, the standard opposite that side is higher than the other

The correction is made by turning a screw underneath the sliding piece of the bearing of the movable axia.

5. The vertical circle is adjusted thus: First carefully level the instrument, bring the zeros of the wheel and vernier into line, and fad or place some well defined point which is cut by the horizontal wire; then turn the instrument half-way around, revolvethe telescope, fix the wire on the same point as before, note if the zeros are again in line,

If not, loosen the screws, move the zero over half the error, and again bring the zeros into coincidence, and proceed as before till the error is corrected

6. The level on the telescope can be adjusted thus; First level the instrument carefully, and with the clamp and tangent movement to the axis make the telescope horizontal as nearly as possible with the eye. Then, having the line of collamation previously adjusted, drive a stake at a distance of from one to two hundred feet, and note the height cut by the horizontal wire upon a staff set on the top of the stake.

Fix another stake in the opposite direction, at the same distance from the instrument, and, without disturbing the telescope, turn the instrument upon its spindle, set the staff upon the stake and drive in the ground till the same height is indicated as in the first observation.

The top of the two stakes will then be in the same horizontal line, whether the telescope is level or not.

Now remove the instrument to a point on the same side of both stakes, in a line with them, and from fifty to one hundred feet from the nearest one; again level the instrument, clamp the telescope as marly horizontal as possible, and note the heights indicated on the staff placed first on the nearest, then on the more distant stake.

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It is the ague, the transcorpe is level, if they do not have treen with the tangent serew move the wire over mathy the whole error, as shown at the distant side and repeat the operation just described till the right if wire will indicate the same height at both the same they have when the telescope will be level. Bring the label into the center by the leveling nuts at the taking care not to disturb the position of the telescope, and the adjustment will be completed.

The adjustments above described are always made by the maker of the instrument, but the instrument may need re-adjusting.

241. Ises of the Transit.

- 1. The transit may be used for all the purposes for which the compass is employed, and, in general, with much greater precision.
- 2. Horizontal angles can be taken by the needle, or without reference to the needle, as follows: Level the plate set the limb at zero, direct to the personal so that the intersection of the wires shall the personal one of the objects selected, clamp the next that the findy to the spandle, unclamp the vernier plate than it with the hand tall the intersection of the vernier to binds, and with the tangent serow fix the intersection of the wires precisely upon the second object. The reading of the vernier will give the angle whole vertex is at the center of the instrument, and whose sides pass through the objects respectively.
- 3 Vertical angles can be measured thus. Level the instrument, fix the zeros of the vertical circle and vernier in a line, note the height out upon the staff

by the horizontal wire, carry the staff up the clevation or down the depression, fix the wire again upon the same point, and the angle will be read off by the vernice. Sometimes, of course, it will be impossible to carry the staff up the elevation, as in taking the angle of elevation of the top of a steeple from a given point in a horizontal plane.

4. Horizontal lines can be run, or the difference of level easily found, by means of the level attacked to the telescope.

242. The Solar Compass.

Burt's solar compass, represented in the drawing on page 205, includes the essential parts of the magnetic compass, together with the solar apparatus, which consists mainly of three area of circles by which the latitude of the place, the declination of the sun, and the hour of the day can be set off.

The latitude arc, a, graduated to quarter-degrees and read to notice s by a vermer, has its center of motion in two pix to one of which is seen at d, and is moved by the toront serew, f, up or down a fixed are of similar curvature through a range of about 35°.

The declination are, b, having a range of about 21°, is gradue to I to quarter-degrees and read to minutes by the verminal fixed to the movable arm, b, which has its center of motion in the center of the declination are at g. The vernier may be set to any reading by the tingent's row, k, and the arm clamped in any position by a screw concealed in the engraving

A solar lens, set in a rectangular block of brise at each end of the arm, h, has its focus at the resident

the opposite black on the surface of a silver plate on a condition drawn certain lines, as shown in the and a vol 12 are. The lines bb, called hour lines, and the

et such other at right angles. The rectungular space between the lines is



is use of the sun formed by the solar lens on the op-

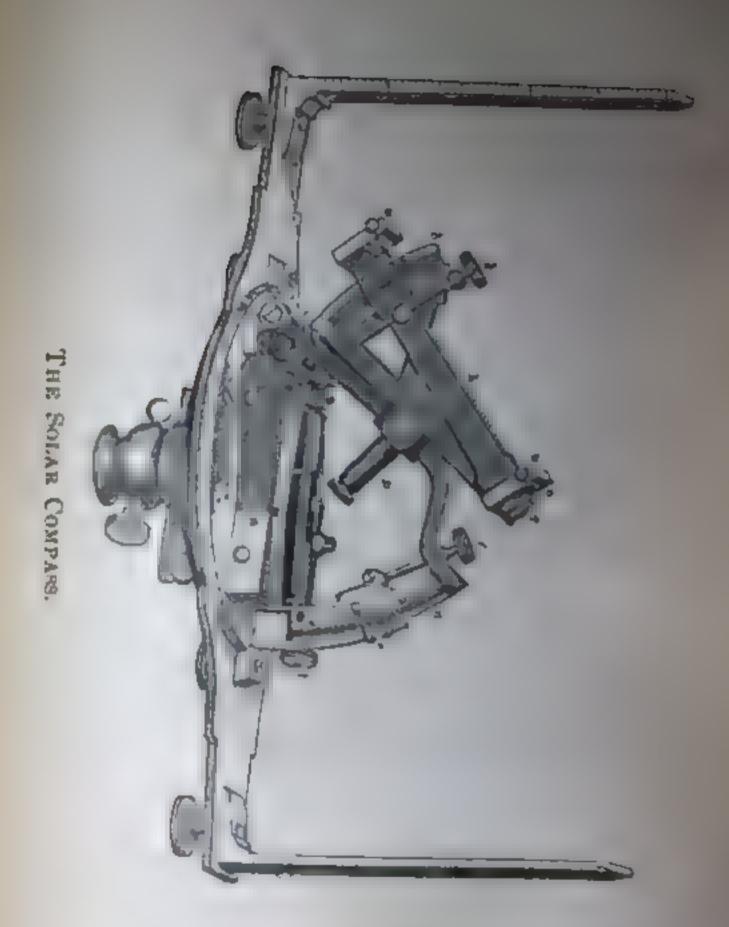
The three other lines below the equatorial lines are five minutes apart, and are used in making allowance for refraction.

An equatorial sight, used in adjusting the solar apparatus, is placed on the top of each rectangular block by a small milled head screw, so as to be detached at pleasure.

The hour are, c, supported by the pivots of the latitude are, and connected with that are by a curved arm, has a range of 120°, graduated to half-degrees and figured in two series, designating both the hours and the degrees; the middle division being marked 12 and 90 on either side of the graduated lines

The polar axis, p, consists of a hollow socket containing the spindle of the declination are, around which this are can be moved over the hour are, which is read by the lower edge of the graduated side of the declination are. The declination are may be turned half round, if required, and the hour are read by a point below q.

The needle box, n, with an arc of 36°, graduated to half-degrees, and numbered from the center as zero, is attached by a projecting arm to a tangent screw, 5 by which it is moved about its center, and its needle



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at to a viriat made him ty be read to minutes by the arm of the arm

ST 2.12 17 NO

The levels were after to theer of the ordinary com-

Lines of refraction are drawn on the inside faces . sights, graduated and figured to indicate the nt allowed for refraction when the sun is near 1 + 1 715001

The adjuster is an arm used in adjusting the instrui it It is not attached to the instrument, and is Is I asade in the box when the adjustment is effected,

243. Adjustments.

1. The levels are adjusted by bringing the bubbles into the center of the tubes by the leveling screws of the tripod, reversing the instrument or the spinile, reserve or lowering the emis of the tree till the I justice will remain in the enter drages complete revolution.

2 The equatorial lines and solar lenses and justed as f llows: First details the arm, I to declinot an are by withdrawing the season in the drawing from the ends of the posts . . tangent s row, k, and also the climp serew, and the conical pi at with its small screws by which to arm and designation are are connected

Attach the adjuster in the place of the rea, h, by replacing the compilier to adserws, and as it the comp serew so as to clear the adjuster at our point on the declination arc.

Now level the instrument, place the arm, h, on the adjuster, with the same side resting against the surface of the declination are as before it was detailed,

turn the instrument on its spindle, so as to bring the solar lens to be adjusted in the direction of the sun, raise or lower the adjuster on the declination are till it can be clamped in such a position as to bring the sun's image, as near as may be, between the equatorial lines on the opposite silver plate, and bring the image precisely into position by the tangent of the latitude are, or the leveling screws of the tripod. Then carefully turn the arm half-way over, till it rests upon the adjuster by the opposite faces of the rectangular blocks, and again observe the position of the sun's image.

If it remains between the lines as before, the leus and plate are in adjustment; if not, loosen the three screws which confine the plate to the block, and move the plate under their heads till one-half the error in the posit or of the sun's image is removed.

Again bring the image between the lines, and repeat the operation till it will remain in the same situation in both portions of the arm, when the adjustment vill be complete.

To adjate the other lens and plate, reverse the arm, end for all on the adjuster, and proceed as in the former c

Remove the adjuster, and replace the arm, h, with its att lan ats

In that time the screws over the silver plate, care mast be taken not to move the plate.

3 The vernier of the declination are is adjusted as follows. Having leveled the instrument, and turned its lens in the direction of the sun, clamp to the Spindle, and set the vernier, r, of the declination are at zero, by means of the tangent serew, k, and clamp to the are.

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we that the spirile noves easily and truly in the was the part as and it is not lower the latitude is but and in the transport series, f. till the sun's marin a law t between the equatorial lines on one of to rate clarp the latatude are by the screw, and to git a reason precisely into position by the level ar we of the tripod or socket, and without dist the instrument carefully revolve the arm, h. t I the opposite lens and plate are brought in the docton of the sun, and note if the sun's image comes between the lines as before.

If the sun's image comes between the lines, there is no index error of the declination are; if not, then with the tangent serew, k, move the arm till the sun's imnge passes over half the error, and again bring the image between the lines, and repeat the operation as before till the image will occupy the same position on both plates.

We shall now find that the zero mas son the are and the vernier do not correspond d to remedy this error, the little flat-head screws at the vernier must be loosened till it can be moved as to make the zeros coincide, when the operation will be compiete.

4. The solar apparatus is adjusted to the sights as follows: First level the instrument, then with the clamp and tangent screws set the main plate at 90° by the verniers and horizontal limb. Then remove the clamp screw, and raise the latitude are till the polar axis is by estimation very nearly horizontal, and, if necessary, tighten the screws on the pivots of the arc so as to retain it in this position.

Fix the vernier of the declination are at zero, and direct the equatorial sights to some distant and wellmarked object, and observe the same through the compass sights. If the same object is seen through both, and the verniers read to 90° on the limb, the adjustment is complete; if not, the correction must be made by moving the sights or changing the position of the verniers.

The adjustments are all made by the maker of the instrument, and, ordinarily, need not concern the survevor, as the instrument is very little halde to derangement.

244. Use of the Solar Compass.

The declination of the sun, or its augular distance from the celestial equator, must be set off on the declination are.

The declination of the sun for apparent noon at Greenwich, England, is given from year to year in the Nautical Almange.

To determine the declination for another place and hour, allessed must be made for the difference of time ar is from longitude, and for the change of declaration from hour to hour

The last the place can be determined with sufficient a tracy by reference to that of given pronunent places which are situated nearly on the same meridian.

The difference of longitude, divided by 15, will, by changing degrees, minutes, and seconds into hours. minutes, and seconds, give the difference of time, which is usually taken to the nearest hour, as it will be sufficiently accurate.

In practice, surveyors in states just cast of the Mississippi allow a difference of 6 hours for long tude;

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The stribut the long tode of Santa FC; Shours for the castern pet and the limited States

Has ground the hour at any place from its longiresults it is a sea at Greenwich, the declination read at Greenwich will be the declination for the remained hour at the given place.

In find the declination for the following hours of the day, add or subtract, for each succeeding hour, the difference of declination for 1 hour, as given in the almanae.

Thus, let it be required to find the declination of the sun for the different hours of May 20th, 1873. W. lon, 95°, 95° = 6 h. 20 m., practically 6 h.

Sun's dec., Greenwich, noon — 20° 3 | 14' 6

Sun's dec., lon. 95°, 6 A. M. — 20 | ' 14' 6

Add difference for 1 h | 11' 03

Sun's dec. 7 A. M. - 26 | ' 45" 63

Add difference for 1 h. | '1' 03

Sun's dec. 8 A. M. | 20° 1 | 6 | 66

In like manner proceed for the remaining hours.

Such a calculation should be made by beginning the work of the day.

Refraction, or the bending of the sun's rays as they pass obliquely through the atmosphere, affects its declination by increasing its apparent altitude

The amount of refraction depends upon the altitude, being less as the altitude is greater. At the horizon the refraction is 35'; at the altitude of 45°, 1'; at the zenith, 0.

Meridional refraction, by in reasing the apparent altitude of the sun, when on the meridian, increases or

diminishes its apparent declination according as it is north or south of the equator.

To find the amount of meridional refraction, we must first find the meridional altitude of the sun for the given latitude, which is equal to the complement of the latitude, plus or minus the declination, according as the sun is north or south of the equator

The meridional altitude of the sun being given, the tables will give the refraction.

The meridional refraction, being quite small, may be disregarded in practice except when great accuracy is required, as in running great standard meridians or base lines.

Incidental refraction, as affected by the hour of the day and the state of the atmosphere, can not, in practice, be determined by a precise calculation.

It will about compensate for incidental refraction to keep the image of the sun square between the equinoctial lines for the middle of the day, but toward morning or evening, to run the image, which is then hazy round the edge, so that the bazy edge shall overlap one or two lines of the spaces below

To set off the latitude, find the declination of the sun for the given day at noon, and set it off on the dech nation are, and clamp the arm firmly to the arc.

Find in the almanac the equation of time for the given day, in order to ascertain the time when the sun will reach the meridian.

About twenty minutes before noon, set up the instrument, level it carefully, fix the divided surface of the declination are at 12 on the hour circle, and turn the instrument on its spindle till the solar lens is brought into the direction of the sun.

the latitude are, there will the harmonic series till the mater of the second brought processly between the equater of the second turn the matrument so as to keep the masses between the bour lines on the plate.

the sin ascends, in approaching the meridian, will move below the lines, and the are must be missed to follow it. Keep the image between the two sits of lines till it begins to pass above the equatorial, which is the moment after it passes the meridian.

Read off the vernier of the arc, and we have the latitude of the place which is to be set off on the latitude arc.

To run lines with the solar compass. If any adjusted the instrument and set off the declination and latitude, the surveyor places the instrument over the station, levels it carefully, clamps the plates at zero on the horizontal limb, and directs the sights north and south, approximately, by the needle

The solar leas is then turned that the sin, and with one hand on the instrument, and the other on the revolving arm, both are moved from side to side till the image of the sun is mad to appear on the silver plate, and is brought press y within the equatorial lines, when the line of sights of indicate the true meridian.

In running an east and west line, the verniers of the horizontal limb are set at 90°, and the sun's image kept between the equatorial lines.

The needle is made to indicate zero on the are of the compass box by turning the tangent serew. Lines can then be run by the needle in the temporary disappearance of the sun. The variation of the needle, which should be noted at every station, is read off to minutes on the are along the edge of which the vernier of the needle box moves.

Since the limb must be clamped at 0 when the sub's mage is in position, in order that the sights may indicate the meridian, it is evident that the braring of any line may be found by the solar compass, as well as by the compass or transit

In running long lines, allowance must be made for the curvature of the earth. Thus, in running north or south the latitude changes 1' for 92.30 ch., and six miles, or one side of a township, requires a change of 5' 12" on the latitude are.

In running cost and west lines, the sights are set at 90° on the limb, and the line run at right angles to the new lian; but this line, if sufficiently produced, would erect the equator. Hence, at the next station, a backs of this taken, and one-half the error is set off for the next foresight on the side toward the pole.

The most favorable season for using the solar compass is the summer; and the most favorable time of day, between 8 and 11 A. M., and 1 and 5 P M.

A solar telescope compass is sometimes used; and, in this case, the telescope is placed at one side of the center. All error from this position of the telescope is avoided by an offset from the flag-staff.

The solar compass, while indispensable in the survey of public lands, can be used, in common practice, with considerable advantage over ordinary needle instruments since lines can be run by it without regard to the variation of the needle or local attraction, and the bearings being taken from the true merukan will a main constant for all time.

245. Dividers and Pens.

SCREETING



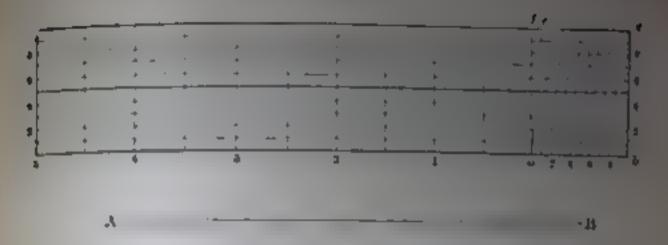
- I. Dividers with lead-pencil.
- 2. Hair dividers with one leg movable by serew.
- a, b. Lengthening bar and pen which a color inserted together or the pen alone instead of pental z
 - 3. Bow pen with spring and adjusti rew.
 - 4. Spacing dividers.
 - 5. Drawing pen.

246. Parallel Rulers.



- 1. Parallel ruler for drawing parallel lines
- 2. Sliding parallel ruler with scales.

247. Diagonal Scale.



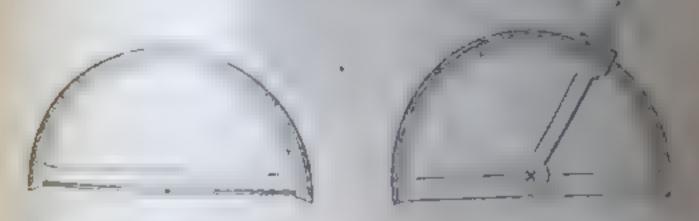
Let do be .I, then the distance from ad to ac on the first line above ab is .01, on the second line .02, etc.

Let it be required to lay off on AB 463.

Place one foot of the dividers at the intersection of the diagonal line, 6, and the horizontal line, 3 Extend the other foot till the horizontal line, 3, intersects the vertical line, 4, then will the distance from one point of the div less to the other be 4 63.

Now place one foot of the dividers at J, and the other at B, then AB will be 4.63.

248. Protractors.



These pretractors are used in laying off or measuring angles. The vertex of the angle is at the center. and one side is made to coincide with the horizontal line passing through the center; then, counting the degrees from the horizontal line round the circumberence till the required degree is reached, and drawite

. : . : me to the content we shall have

to the potribors will give angles to the rate of a latter second, by means of a ver-

to the person of the person.

SURVEY OF PUBLIC LANDS.

249. Division into Townships.

In the rectangular system of surveying the public lands, adopted by the government, two principal lines —an east and west line, called a base line, and a north and south line, called a principal meridian — are established before the survey of the tows

Six miles to the north of the base line another east and west line is run, and six miles to the north of this another, and so on.

Every fifth parallel from the base i 1 d a standard - parallel, or correction line.

Six miles to the west of the principal meridian, measured on the base line, another north and south line is run to the first standard parallel, and six miles to the west of this another, and so on.

The intersection of the east and west with the north and south lines divides the tract into townships, which would be exactly six miles square were it not for the convergence of the meridians

To preserve as nearly as possible the form and size of the townships, the standard parallels before men-

the townships north up to the next standard parallel.

Tiers of townships north and south are called ranges, and are numbered east or west, as the case may be, from the principal meridian

Lines running north and south, bounding the townships on the cast and west, are called range lines

Lines running east and west, bounding the townships on the north and south, are called township lines.

A township marked thus, T. 5 N., R 4 W, read township five north, range four west, would be in the fifth tier north of the base line, and in the fourth tier west of the principal meridian.

Townships are divided into sections, or square notes, contains g 610 acres; each section into four quarter sections, each quarter section into two half-quarter sections, and each half-quarter section into two quarters quarter sections. These are called legal subdivisions, and are the only divisions recognized by the government, except pieces made fractional by water-courses of other natural agencies.

On base lines and standard parallels two sets of corners are established.

- 1. Standard corners, established when these lines are run, embracing township, section, and quarter section corners, common to two townships, sections, or quarter sections north of the base line or standard parallels
- 2. Closing corners, established when exterior and subdivision lines close on them from the south, embracing township and section corners, common to two townships or sections south of the standard parallels.

In consequence of the convergence of the meridians, the north and south lines, produced to the standard

S N 19

In . . w it close on the standard corners proxi-to the rost of the standard corners, making . vien. 2 is the fibliof operation is west or cist .. to prograd meridian.

St. 1121/16

The following diagram will illustrate the subject

AB is the base line.

AC, the principal me-Piclins.

AB, a standard paral-1 1

nh, al, etc., township lines.

ij, kl, etc., range lines.

s, u, w, etc., standard corners.

j, l, a, etc., closing cor- By hers.

The distances js, lu, etc., are measured and recorded in the field book.

The details of running lines will be given after describing the methods of perpetuating corners, the process of chaining, and the method of marking lines.

Burt's improved solar compass is used in surveying standard and township lines, but the ordinary compass may be used in subdividing.

250. Methods of Perpetuating Corners.

1. Corner trees. - A sound tree, five inches or more in diameter, standing exactly at a corner, is the best monument.

- 2. Corner stones. A stone, at least 11 inches long and 6 inches square, set from two thirds to threefourths in the ground, is preferred to other mounments, except a tree.
- 3 Posts and witness trees. In the absence of corner teres and stones, when trees are near, a post near be planted and witnessed by taking the bearing and distance of two or more trees in different directions from the corner. These trees are marked by a blaze in which is marked the number of the township, range, and section. A notch is cut in the lower end of the blaze, under which another blaze is made in which are cut the letters B. T., signifying bearing tree
- 4. Posts, mounds, and witness pits. When neither corner trees, stones, nor witness trees are available, corners may be marked by posts, mounds, and witness pits. The posts are planted 12 inches in the ground, and at the lower end, on the north or west side, according as the course is north or west, a marked stone, a small quantity of charcoal, or a charred stake must be deposited. Four pits are dug, 6 feet from the post, on opposite sides, 2 feet square and 1 foot deep, and the excavated earth packed round the post within 1 foot of the top. If sod is to be had, it is to be used in covering the mounds.

The method of marking the corner is to be carefully noted in the field book.

251. Township Corners.

1. Posts used in marking township corners must be 4 feet in length, and 5 juches, at least, in dimeter These posts are to be set 2 feet in the ground, and

to the part a med to receive the marks to be

If the erer is the latest the material part a part TIN TEN a not we are too present the L (3) RIW. a us the direction of 8 34, b 31, to line; as I the number . t. e township rings, and TILN TIN. wet a most be marked on R CW 1. 3 35 the sale from and six B. 1. 5 0 not be cut on such of the text colgon.

If the township corner is on a base line or standard parallel, unless it is also on the principal meridian, it will be common to two townships only; and if these are on the north, the corner will be a standard corner. In this case, six notches are cut on the east, north, and west edges, but not on the south edge, and the letters S. C., signifying standard corner—it on the dat surface.

If the corner is common to two tow to ps on the south, but not on the north, it will be alosing corner, and six notches are cut on the south, and west edges, but not on the north edge and the letters C. C. signifying closing corner, cut on the flat surface.

2. Township corner stones should be inserted at least 10 inches in the ground, with their sides facing the cardinal points of the compass, and small mounds of stones heaped against them.

These corner stones are notched in the same manner as posts in similar circumstances, but are not otherwise marked.

3. A tree of proper size on the corner is marked in the same manner as a post

The mounds, when node round the posts, must be 5 feet in diameter at the base, and 2½ feet high. The posts, therefore, must be 4½ feet long, so as to be 1 foot in the ground and 1 foot above the top of the mound

Witness pits for township corners must be 2 fort long, 1½ feet wide, and 1 foot deep. If the corner is common to four townships, there will be four pits placed lengthwise on the lines; but if the corner is common to only two townships, only three pits are dug, and are placed lengthwise on the lines. Thus the kind of township corners are readily distinguished

These pits are made only in the absence of witness trees, which are to be selected, if possible, one from each township.

252. Section Corners.

Section corners are established at intervals of 80 chains or 1 mile, and are perpetuated by the following methods:

1. Section corner posts are 4 feet in length, and at least 4 inches in diameter. They are planted 2 feet in the ground, and the part above the ground squared to receive the marks.

If the corner is common to four sections, the post is set cornerwise to the lines, the number of the section is marked on the side facing it, and the number of the township and range on the north east face

Mile-posts on township lines have as many notches on the corresponding edges as they are miles from the respective township corners.

so ; a paste within a township have as many a they are talks. to the township, I that the an est on the north and west edges

Section tests must be witnessed by trees, one in er h weto a er, in the absence of trees, by pits 18 1 les square and 12 inches deep.

2 Section corner mounds are 43 feet in diameter at t ... base, and 2 feet high. The post must be 4 feet long, I foot in the ground, and I foot high above the mound, and at least 3 inches square.

At corners common to four sections, the edges are in the direction of the cardinal points; but at corners common only to two sections, the flattened sides face the cardinal points.

Section posts in mounds are to be marked and witnessed in the same manner as the post without the mound.

- 3. Stones used to mark section corners on township lines are set with their edges in the direction of the line; but for interior sections they face the north. They are witnessed in the same matther as posts, but are not marked except by notches.
- 4. Section corner trees are marked and witnessed the same as posts.

253. Quarter Section Corners.

Quarter section corners are established at intervals of 40 chains or half a mile, except in the north of west tiers of sections of a township,

In subdividing these sections, the quarter post is placed 40 chains from the interior section corner, so that the excess or deficiency shall fall in the last half mile

PUBLIC LANDS.

Quarter section corners are not required to be established on base or standard parallel lines on the north,

The methods of perpetuating these corners are the following:

- 1. Quarter section posts, 4 feet in length and 4 inches in diameter, are planted or driven 2 feet into the ground, and the part above the ground squared and marked \ S., signifying quarter section. These corners are witnessed by two bearing trees,
- 2. Quarter section mounds are, like section mounds, packed round the posts, and pits may be used in the absence of witness trees.
- 3. Quarter section stones have | cut on the west side of north and south lines, and on the north raie of east and west lines, and are witnessed by two bearing trees or pits.
- 4. A quarter section tree is marked and witnessed in the same manner as a post.

254. Meander Corners.

Meander corners are the intersections of town-hip or section lines with the banks of lakes, bayons, or mavigable rivers.

These corners are marked by the following methods:

- 1. Meander posts of the same size as section posts, are planted firmly in the ground, and witnessed by two bearing trees or pits, but are not marked.
- 2. Mounds of the sume size as those for section corners are, in the absence of witness trees, formed round

t e posts, and a pit dug exactly on the line, 8 links fact, or treat the water than the mound.

1 Stones or trees, with seed in the same manner as pasts, may be employed.

255. Chaining.

Eleven tally pine are employed, ten of which are taken by the fore chainman, or leader, and the remaining one by the hind chainman, or follower, who sticks it at the beginning of the course, and against it brings the handle at one end of the chain.

The leader, holding the other handle of the chain and one pin in his right hand, draws out the chain to its full length in the direction of the course; both taking care that the chain is free from kinks.

The leader standing to the left of the line, so as not to obstruct the range, with his right arm extended, draws the chain tight, brings the pin into line according to the order "right" or "left," from the follower, sticks it at the order "down" by pressing his left hand on the top of the pin, and replies "down."

The follower then withdraws his pin, and both advance, the leader drawing the chain in the direction of the course, but a little to one side to avoid dragging out the pin, till the follower comes up to the pin, against which he brings the handle at his end of the chain, and directs the sticking of another pin, as before, and so on.

When the leader has stuck his last pin, he cries "tally," which is repeated by the other, and each registers the tally by slipping a ring on a belt

The follower then comes forward, and counting in presence of his fellow, to avoid mistake, the pins taken

up, takes the foreward end of the chain and proceeds, as the leader, for another tally

If a whole chain is employed, a tally is ten chains; and accordingly four tallies make half a mile, and eight tallies a mile.

If a half-chain is employed, a tally is five chains, eight tallies are half a mile, and sixteeen tallies a mile.

In measuring up or down a hill, the chain must be kept horizontal, so that it is often necessary to use but a portion of the chain.

The chain employed in the field must be compared, from day to day, with a standard chain furnished by the Surveyor-General, and any variation promptly corrected.

256. Marking Lines.

Line trees, called also "station trees," or "sight trees," are marked by two notches on each side of the tree, in the direction of the line.

The line is marked, so as to be casily followed, by blazing a sufficient number of trees near the line on two sides quartering toward the line

Saplings nor the line are cut partly off by a blow from the ax at the usual height of blazes, and bent at right angles to the line.

Random lines are not marked by blazing trees, but to enable the surveyor to retrace the line on his return, bushes are lopped and bent in the direction of the line, and stakes are driven every ten chains, who have are pulled up when the true line is established

Insuperable objects, such as ponds, marshes, etc., are passed by making right-angled offsets, or by tracero-

to trival operations, a complete record of which must be mad in the told books

257. Initial Point and Principal Lines.

- 1 The initial point, which is usually some permaint natural object, as the confluence of two rivers, or an isolated mountain, is first selected.
- 2. Principal meridians are run from the initial points due north or due south, and the quarter section, section, and township corners on these lines are area-rately located and perpetuated.

The following are the principal meridians already established:

1st. The first runs north from the mouth of the Great Miami river, between Ohio and Indiana, to the south line of Michigan.

2d. The second runs north from the mouth of the Little Blue river through the center of Indiana to its north line.

3d. The third runs north from the mouth of the Ohio river through Illinois to its north line.

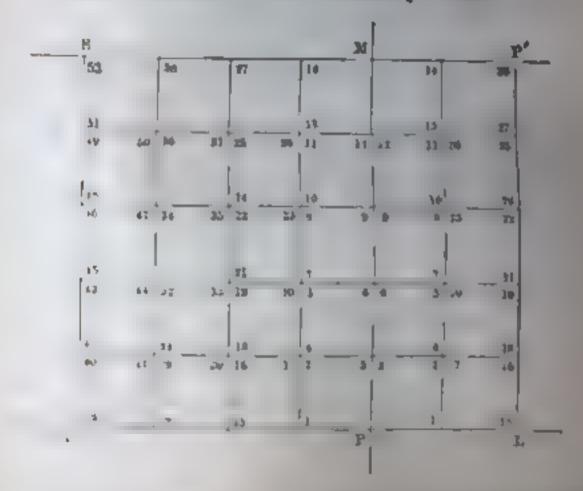
4th. The fourth runs north from the Illinois river through the western part of Illinois and the center of Wisconsin to Lake Superior.

5th. The fifth runs north from the mouth of the Arkansas river through the eastern portion of Arkansas, Missouri, and Iowa, and regulates the surveys in Minnesota west of the Mississippi river, and the surveys in Dakota east of the Missouri river.

6th. The sixth commences on the Arkansas river, in Kansas, and runs north through the eastern part of Kansas and Nebraska to the Missouri river.

- 7th. Independent meralians These are the Independent meridian of New Mexico, the Salt Lake meridian in Utah, the Willomette meridian of Oregon and Washington, and the Humboldt meridian, the Mt. Dublo meridian, and the St. Bernardino meridian of California.
- 3. Base lines are run from the initial points due cast or due west, and the quarter section, section, and town-ship corners, for the land north of the line, are accurately located, at full measure, and perpetuated.
- 4. Standard parallels are also run due east or due west thirty miles north of the base line or other standard parallel, and the corners located and perpetuated as on the base line.
- 5. Range lines are run between the ranges of town-ships due north from a base line or standard parallel to the next standard parallel.

258. Exterior or Township Lines.



In the above diagram let P denote the initial p into PM the principal meridian, BL the base line, SP the

first star land parallel north, and let the squares denote DADN JA

SURVEYING

1 Fr townships west of the meridian, begin at the first presentablished township corner on the base line n -t of the meridian. This is the S. W. corner of T 1 N, R. 1 W, and is marked 1 in the diagram.

M asure thence due morth 480 chains, establishing t'e quarter section and section corners, to 2, at which point establish the corner common to The 1 and 2 N. and R.'s 1 and 2 W.; thence cast on a random line, setting temporary quarter section and section stakes to 3.

If the random line should overrun, or fall short, or intersect the meridian north or south of the true corner, more than 3.50 chains, a material error has been committed, and the lines must be retraced.

If the random line should terminate within 3.50 chains of the corner, measure the distance at which the meridian is intersected north or south of the corner, calculate a course which will run a true line back from the corner to the point from which the random line started, measure westward to 4, which is the same point as 2, establish the permanent corners, obliterate the temporary corners on the random line, and throw the excess or defect, if any, on the west end of the line.

In like manner, measure from 4 to 5, from 5 to 6, from 6 to 7, and so on to 11, on the standard parallel, throwing the excess or deficiency on the last half mile. At the intersection with the standard parallel, establish the township closing corner, measuring and recording the distance to the nearest standard corner on said standard parallel.

If from any cause the standard parallel has not been run, the surveyor will plant the corner of the township in place, subject to removal north or south when the standard parallel shall have been run.

The surveyor then proceeds to the S. H. corner of T. 1 N., R. 2 W., on the base line at 15, and proceeds in a similar manner with another range of townships, and so on.

2. For townships east of the meridian, begin at the S. E. corner of T. 1 N., R. 1 E., at 1 on the base line, and proceed as on the west of the mernhan, except that the random lines are run west and the true lines east, throwing the excess over 480 chains, or the deficiency, on the west end of the line in measuring the first quarter section boundary on the north, the remaining distances will be exact half-miles and miles

With the field notes of the exterior or township lines, a plot of the lines, run on a scale of 2 mehes to the mile, must be submitted, on which are noted all objects of topography, which will illustrate the mites, a the direction of streams, by arrow-heads pointing down stream, the intersection of the lines by likes streams, ponds, marshes, swamps, ravines, mountains, etc.

259. Subdivision or Section Lines.

The deputy employed to run the exterior lines of a township is not allowed to subdivide it, but another is employed to do this work, that the one may be a check to the other, thus securing greater accuracy

Before subdividing a township, the surveyor must ascertor, and note the change in the variation of the needle which has taken place since the town-hip lines were run, and adjust his compass to a variation which will retrace the eastern boundary of the township.

PUBLIC LANDS.

He must den compare his own chaining with the compare his own chaining with the count hy not asurang the first mile both of the wath and est have of the township, and not the derignance, if any,

The following is a diagram of a township:

	*	44	54	54	al.
6	5	1 4	3	2	ı
	24	43	30 40	21 34	16 15
7		1 0	1		1
7	8	9	10	II	12
P 87	100 PM	r es es	[6] 63 60	ти от ти	1) 12
18	17	16	15	14	. 13
	ma .	44	11	57	0
MI 442		u ye mu T	ia a	.d 76	n u
19	20	21	22	23	24
2 7	79 15 t	A 31	20 41		2
1	29	28		26	25
	11	A 0	2	20	20
72 .	N 1		4	,	1
31	32	33	34	35	36
		×1.	34	> _H	

The sections are designated by begin ing at the N. E. corner and numbering west, 1, 2 1 1 5, 6, then east on the next tier, 7, 8, ..., then west, and so on.

In running the subdivision lines, begin on the south line of the township, at the first section corner west of the east line, numbered 1 in the diagram, and common to sections 35 and 16.

Measure thence due north 40 chains, at which point establish a quarter section corner; thence due north another 40 chains to 2, where establish a section corner common to sections . 5, 26, 35, and 36

Run a random line from 2 due cast to the township line, setting up a temporary quarter section stake 40 chains from 2.

If the random line intersect the township line preessely at the pre-established section corner at 3, it may be established as the true line by blazing back and making the quarter section corner permanent.

If the random line intersect the townst ip line either north or south of the section corner, measure and note the distance of the intersection from said corner, and calculate a course which will run a true line from the corner back to 4, where the random line started.

Let A correspond to section corner 2, B to 3, and
C to the intersection of the township and random lines, and north, for example, of B the section corner.

Then, tan
$$A = \frac{BC}{AB}$$
.

Let l - the number of links in BC, and m the number of minutes in A. Then, practically, we shall have,

If
$$AB = \frac{1}{2}$$
 mile, $m = l + l$.
If $AB = 1$ mile, $m = \frac{1}{2}l - \frac{1}{2}l$.
If $AB = 3$ miles, $m = \frac{1}{2}l$.
If $AB = 6$ miles, $m = \frac{1}{2}l$.

Let us suppose that we have found A = 10Y

Now, as CA is west by the compass, BA is N. 812 49½ II. Run this line and establish the quarter sees tion at a point equidistant from the two section corners, which will be, with sufficient accuracy, one-half the length of the random line from 2. Pull up the temporary quarter section stake on the random line.

Present from 4 to 5, then on a random line to 6, and to know a true 1 to 15, and so on to 16.

From to run due north on a random line to the north the of the township, setting up a temporary quarter section stake at 40 chains.

township at the pre-established section corner, the random line will be the true line, and is made permanent by blazing back, and making the quarter section corner permanent.

If the random line does not close exactly on the pre-established section corner, measure and note the distance of the intersection from said corner, calculate a course that will run a true line southward from the corner to 16, run this line, and establish the quarter section corner on it just 40 chains from 16, throwing the excess or deficiency, if any, on the last half mile.

ard parallel, no random line is a base line or standard parallel, no random line is run, but a true line due north, on which a quarter section post is established 40 chains from 16; and at the intersection with said base line or standard parallel, establish a closing corner, measuring and noting its distance from the corresponding standard corner.

Pass from 17 to 18, and survey the second tier of sections in the same manner as the first, closing on the interior section corners before established as upon those on the east line of the township

In running the line between the fifth and sixth tiers of sections, not only is a random line run east as before, but one is run west to the range line, and a true line run back, and the permanent quarter section corner established on it just 40 chains from the in-

terior corner, throwing the excess or deficiency on the west half mile.

The Surveyor-General furnishes the outline of the diagram, and the deputy fills it out, and makes the appropriate topographical sketches.

260. Meandering.

Navigable rivers, lakes, and bayous, being public highways, are meandered and separated from the adjoining land.

Standing with the face down stream, the bank on the right hand is called the right bank; the bank on the left, the left bank.

If a river is navigable, both banks are meandered, care being taken not to mistake, in high water, the border of bottom-land for the true bank

Commence at a meander corner of the township line, take the bearing along the bank of the river, and measure the distance of the longest possible straight course to the nearest chain, if the distance exceeds 10 chains; otherwise, to the nearest ten links; and so on to the next meander corner on another boundary line of the township.

Enter is the field book, after the township notes, keeping the notes separate through each fractional section, the date, the point of beginning, the bearings and distances in order, the intersections with all intermediate meander corners, the height of falls, the length of rapids, the location and width at the month of streams running into the water you are meandering, the location of springs on the banks, the nature of their waters, the location of islands, the elevation of banks, etc.

S. N. 20

If the river is not navigable, meander the right look, and so it presents formidable obstacles not found in the left bank, but the crossing of the stream, in a lering, must be made from a pre-established mean it room rom one bank to the corner on the other inch, and the width of the river between the corners of puted trigonometrically.

Wide flats, whose area is more than 40 acres, permanently covered with water, along rivers not navigable, are meandered on both banks.

The position of islands in rivers is determined by measuring, on or near the bank, a base line, connected with the surveyed lines, and taking the proper bearings to a flag or other object on the island, and computing the distance from the meander corners of the river to points on the bank of the island. The island can be meandered from such points.

In meandering lakes, ponds, or bayous, commence at a meander corner of the township line, and proceed as in case of a river. If, however, the body of water is entirely within a township, begin at a meander corner established in subdividing

In meandering a pond lying entirely within the boundaries of a section, run to the pond two lines from the nearest section or quarter section corners, on opposite sides of the pond, giving their bearings and distances, and at the intersection of these lines with the bank of the pond establish witness points by planting posts, witnessed by bearing trees or mounds and pits, then commence to meander at one of these points, and proceed around to the other, and thence to the point of beginning.

No blazes or marks are made on meander lines between established corners.

261. Swamp Lands.

By the act of Congress approved Sept. 28th, 1850, swamp and overflowed lands, unfit for cultivation, are granted to the state in which they are situated.

If the larger part of the smallest legal subdivision is swamp, it goes to the state; if not, it is returned by the Government.

In order to determine what lands fall to the state under the swamp act, it is required that the field notes, beside other things required to be noted, should indicate the points where the public lines enter and leave all such land.

The aforesaid grant does not embrace lands subject to casual inundation, but those only where the over-flow would prevent the raising of crops without artificial aid, such as levees, etc. The surveyor should therefore state whether such lands are continually and permanently wet, or subject to overflow so frequently as to render them totally unfit for cultivation.

The depth of inundation is to be stated, as determined from indications on the trees, and the frequency of inundation should be given as accurately as persible, from the nature of the case or reliable testamony

The character of the timber, shrubs, plants, etc., growing on such lands, and on the land near rivers, lakes, or other bodies of water, should be stated

The words "unfit for cultivation" should be culployed, in connection with the usual phraseology, in the notes, on entering or leaving such lands

If the margin of bottoms, swamps, or marsh some which such uncultivable land exists, is not idented with the body of land unfit for cultivation, a separate entry must be made opposite the marginal distance.

In case the land is overflowed by artificial means, which as done for milling, logging, etc., such overflow wal not be officially regarded, but the lines of the partie surveys will be continued across the same without setting meander posts, stating particularly in the notes the depth of the water, and how the overflow was caused.

262. Field Books.

The field books are the original and official records of the location and boundaries of the public lands, and afford the elements from which the plots are constructed.

They should, therefore, contain an accurate record of every thing officially done by the surveyor, pursuant to instructions in running, measuring, and marking lines, and establishing corners, and should present a full topographical description of the tract surveyed.

There are four distinct field books

- L. A field book for the meridian and base lines, exhibiting the establishment of the township, section, and quarter section corners on these lines, the crossing of streams, raymes, laber, and mountains, the character of the soil, timber, minerals, etc.
- 2. A field book for standard parallels of rection lines, showing the township, section, and quarter section corners on the lines, and the topography of the country through which the lines pass
- 4. A field book for subdivision or section lines, giving the corners and topography as aforesaid

The variations of the needle must be stated in a separate line, preceding the notes of measurement, which must be recorded in the order in which the work is done, and the date must immediately follow the notes of each day's work.

The exhibition of every mile surveyed must be complete in itself, and be separated from the preceding and following notes by a line drawn across the paper.

The topographical description must follow the netex for each mile, and not be mixed up with them

No abbreviations are allowed, except for words constantly occurring, as sec. for section, ch. for chains, #, for feet, \(\frac{1}{4}\) sec. cor. for quarter section corner.

Proper names are never to be abbreviated

The field books must be so kept as to show the amount of work done in each fiscal year.

The notes should be expressed in clear and precise language, and the writing legible.

No record is to be obliterated, or leaf mutilated or taken out

The title-page of each book should designate the kind of lines run, giving prominently the name of the state or territory and surveyor, the states of contract, and of commencing and completing the work

The second page should contain the names and duties of assistants; and whenever a new assistant is employed, or the duties of any of them changed, such facts, with the reason, should be stated in an appropriate entry, immediately preceding the notes taken under such changed arrangements.

An index, in the form of a diagram or plot of the survey, with number on each line, referring to the page of the field notes on which is a und the description of the line, must accompany the notes

2.8

263. Records in the Field Book,

- I General heading of the pages. The number of the township and range, and the name of the principal the dian of reference, stand at the head of each page,
- 2 Heading for each mile.-The bearing, location, and kml of line run, whether random or true, must be stated in a line; and the variation of the needle, in a reparate line on the page at the head of the notes, for each mile run.
- 3. Courses and distances. The course and length of each line run, noting all meessary offsets therefrom, with the reason and mode thereof.
- 4. The method of perpetuating corners. If a tree, note the kind and diameter; if a stone, its dimensions, as factors in the order of length, breadth, and thickness; if a post, its dimensions, the kind of timber, the kind of memorial, if any, buried by its side, and if surrounded by a mound, the material of which the mound is constructed, whether of stones or earth: Il course and distance of the pits from the cent - the mound where a necessity exists for deviating the general rule of witness trees.
- 5. Bearing trees.—The kind and di ter of all bearing trees, with the course and dist of the same from their respective corners, and the recise relative position of the witness corners with a spect to the true corners.
- 6. Line trees.-The kind, diameter, and distance on the line, from the corner, of all trees coch the line intersects.
- 7. Intersection of land objects.—The distance at which the line first intersects and then leaves every settler's claim and improvement, prairie, bottom land, awamp.

- marsh, grove, or windfall, with the course of the same at both points of intersection; the distance at which a line begins to ascend, arrives at the top, or reaches the foot of all remarkable hills and ridges, with their courses and estimated height above the surrounding country.
- 8. Intersection of water objects. The distance at which the line intersects rivers, creeks, or other bodies of water, the width of navigable streams, and small lakes or ponds between the meander corners, the height of banks, the depth and nature of the water.
 - 9. Surface.-Level, rolling, broken, or hilly
- 10. Soil.—First, second, or third-rate; clay, sand, loam, or gravel.
- 11. Timber. Kind, in order of abundance, and undergrowth.
- 12. Bottom-lands. Wet or dry; whether subject to inundation, and to what depth.
- 13. Springs .- Fresh, saline, or mineral; and course of their streams.
- 14. Improvements. Towns and villages, Indian villag - and wigwams, houses and cabins, fields, fences, sugar to groves, mill-seats, forges or factories.
- 15. Coal beds. Note the quality of coal beds, and their extent to the nearest legal subdivision.
- 16. Roads and trails.-Whence, whither, and direction.
- 17. Rapids, cascades. Length of rapids, height of falls in feet.
- 18. Precipices. Describe precipices, caves, ravants. sink-holes,
- 19. Quarries, -- Whether marble, granite, line store or sand-stone.

30 Natural currosities. -- Interesting fessils, am tent works, is it it als, I rist ations, embankments, etc.

SURI EYING.

- 21 Change of variation. Any material change in the v. . . at u of the accedie must be noted, and the exact 1. .. its where such variation occurs.
- Dates. State the date of each day's work in a wharste line, immediately after the notes for that day,
- 23. General description. At the conclusion of the notes for the subdivisional work, taken on the line, the deputy must subjoin a general description of the township in the aggregate, in reference to the face of the country, its soil, timber, geological features, etc.
- 24. Verification of Deputy Surveyor. The deputy must append to each separate book of field notes his affidavit that all the lines therein described have been run, and all the corners established and perpetuated according to the instructions and laws, and that the foregoing notes are the true and original field notes of such survey.
- 25. Verification of Assistants. The compassman, flagman, chammen, and axir an in est to a transfer oath, that they assisted said deputy in ex and surveys, and that, to the best of their ky la and belief, the work has been strictly performed according to the instructions furnished by the Surveyor-General.
- 26. Approval and certificate of the Surveyor-General. --The Surveyor-General will attach his efficial approval to cach of the or and field books, and affix his oflatal certificate to the copies of the field notes transmitted to the general land office, that they are true copies of the orginals on file in his other

The following specimen pages of field notes, taken from the United States Manual of Sugreman Instructions, will illustrate the subject

FIELD NOTES

OF THE

Exterior and Subdivision Lines

OF TOWNSHIP 25 NORTH, RANGE 2 WEST, WILLAMETTE MERIDIAN,

OREGON.

Surveyed by Robert Acres, Deputy Surveyor,

Under his contract, dated ---, 18-.

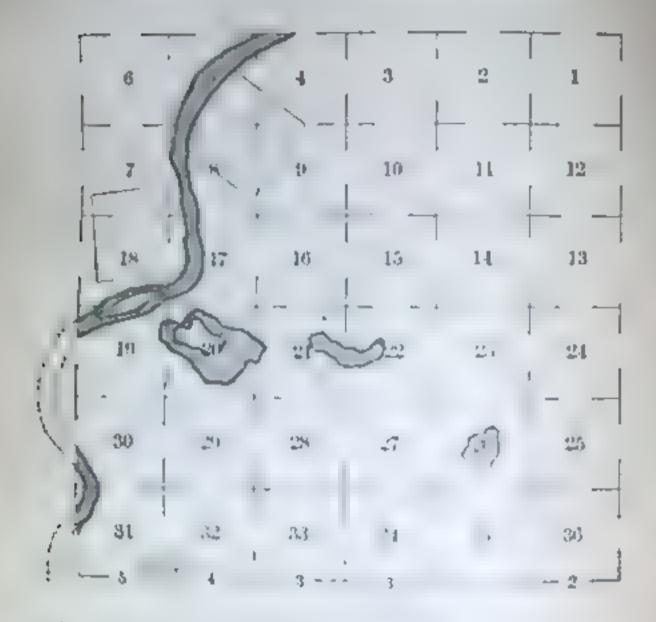
Survey commenced ----.

Surery completed -----

264. Index.

By cruej the lines to the pages of the field notes.

T. 25 N., R. 2 W., Willamette Meridian.



The lines numbered are described in the notes on the pages indicated by the numbers.

NAMES OF SURVEYOR AND ASSISTANTS.

Robert Acres, Surveyor George Shor Axman.

Peter Long, Chainman Alam D. J. Axman

John Short, Chainman Harry Flor Compassman.

265. Field Notes.

South Boundary, T. 25 N. R. 2 W., Wellamette Meridian.

Chains.

Begin at the post, the established corner to Townships 24 and 25 North, in Ranges 2 and 3 West. The witness trees all standing, and agree with the description farnished meby the office, viz:

A Black Oak, 20 in. dia., N. 37° E. 27 links,

A Burr-oak, 24 in. dia., N. 43° W. 35 links,

A Maple, 18 in. dia , S. 27° W. 39 links.

A White Oak, 15 in dia., S. 47° E 41 links. East on a random line on the South Boundaries of sections 31, 32, 33, 34, 35, and 36

Variation by Burt's improved solar compass, 18° 41' E.

I set temporary half-mile and mile posts at every 40 and 80 chains, and at 5 miles, 74 chains 53 links, to a point 2 chains and 20 links north of the corner to Townships 24 and 25 North, Ranges 1 and 2 W.

(Therefore, the correction will be 5 chains, 47 links West, and 87 links South per mile;

I find the corner post standing and the witness trees to agree with the description furnished me by the Surveyor-General's office, viz:

A Burr-oak, 17 in. dia., bears N. 41° E 31 links,

A White Oak, 16 in. dia, bears N. 26° W. 21 links,

A Linden, 20 in. dia., bears S 42° W 15 lks., A Black Oak, 24 in. dia., bears S. 27° E 11 links

(3)

10:00

80.00

STRVELING.

Sees Beendress T 25 N. R. 2 W. W. lamette Meridian.

I rom the corner to Townships 24 and 25 (balas N. Ranges I and 2 W. I run cat a variation of 182 H' East [See Arts, 258, 289.]

N. 89° 44' W., on a true line along the South Boundary of section 36, s t a post for quarter section corner, from which

A Beech, 24 in. dia., bears N. 11° E. 38 links dist

A Beech, 9 in. dia., bears S. 9° E. 17 links dist.

A Brook, 6 links wide, runs North. (52,51)

Set a post for corner to sections 35 and 36, 1 and 2, from which

A Beech, 9 in. dia., bears N. 22° E. 16 links dist

A Beech, 8 in. dia., bears N. 19° W. 14 links dist.

A White Oak, 10 in, dia, bears S. 52° W. 7 links dist

A Black Oak, 11 in, dia bears S. 46° E. 8 links dist

Land -level, good soil, fit r cultivation.

Timber Beech, various kit | O.k. Ash, Hickory

N 89 H'W on a troy line long the South Boundary of section 35, Variation 18° 41' E.

Set a post for quarter section corner, from which

A Beech, S in, dia , bears N, 20° E, 8 links dist

No other tree convenient; made a trench

South Boundary, T. 25 N., R. 2 W., Willamette Meruluan

Chains. Begin to ascend a moderate hill; bears N $65\ 00$ and S.

Set a post with trench, for corner of sections 80.00 34 and 35, 2 and 3, from which

> A Beech, 10 in. dia, bears N. 56° W. 9 links dist.

> A Beech, 10 in. dia., bears S. 51° E. 13 links dist.

No other tree convenient to mark.

Land -level, or gently rolling, and good for farming.

Timber - Beech, Oak, Ash, and Hickory; sonia Walnut and Poplar.

N. 89° 44' W. on a true line along the South Boundary of section 34, Variation 18° 41' E.

Set a quarter section post with treuch, 40.00 from which

> A Black Oak, 10 in. dia, bears N. 2° E. 635 links dist.

No other tree convenient to mark.

To point for corner sections 33, 34, 3 and 4 80,00 Dr. v. charred stakes, raised mounds with to ches, as jee instructions, from which

> A Burr-oak, 16 in. dia., bears N. 31° E. 344 links,

> A Hickory, 12 in. dia., bears S. 43° W. 231 links.

No other tree convenient to mark.

Land - level, rich, and good for famous Timber - some scattering Oak and Walnut.

40,00

around post

40,00

(4)

Sat Paniery, T. 25 N. R 2 W., Willamette Township.

Chains. N. 80° 44' W. on a true line along the South Boundary of section 33, Variation 18° 41' E.

37.51 A Black Oak, 24 in. dia.

and 5, from which

Set a post for quarter section corner, from

A Black Oak, 18 in. dia., bears N. 25° E. 32 links dist.

A White Oak, 15 in. dia., bears N. 43° W. 22 links dist.

62 00 To foot of steep hill, bears N. E. and S.W. 80 00 Set a post for corner to sections 32, 33, 4

A White Oak, 15 in. dia, bears N. 23° E. 27 links dist.

A Black Oak, 20 in. dia., bears N. 82° W. 75 links dist.

A Burr-oak, 20 in. dia., bears S. 37° W. 92 links dist.

A White Oak, 24 in. dia, bears S. 26° E. 42 links dist.

Land — gently rolling, rich farming land. Timber — Oak, Hickory, and Ash.

N. 89° 44′ W. on a true line along the South Boundary of section 32, Variation 18° 41′ E.

A Creek, 20 links wide, runs North.

Set a granite stone, 14 in. long, 10 in. wide, and 4 in. thick, for quarter section corner, from which

A Maple, 20 in. dia., bears N. 41° E. 25 links dist.

A Birch, 1 in dia, bears N. 35° W. 22 Hinks dist. -(5)

South Boundary, T. 25 N., R. 2 W., Willamette Meridian.

Chains.

To S. E. edge of swamp.

As it is impossible to establish permanently the corner to sections 31, 32, 5 and 6, in the swamp, I therefore, at this point, 4,00 chains east of the true point for said section corner, raise a witness mound with trench, as per instructions, from which

A Black Oak, 20 in. dia., bears N. 51° E. 115 links.

80.00

A point in deep swamp for corner to sections 31, 32, 5 and 6.

Land -rich bottom; nest of creek, part wet; cost of creek, good for farming

Timber - good; Oak, Hickory, and Walnut.

N. 89° 44' W. on a true line along the South Boundary of section 31, Variation 18° 41' E.

11 (a) Leave swamp and rise bluff 30 feet high, bears N. and S.

40 oc Set post for quarter section corner, from which

A Sugar tree, 27 in. dia., bears S. 81° W. 12 links dist.

A Beech, 24 in. dia., bears S. 71° E. 21 links dist.

54.00 Foot of rocky bluff 30 feet high, bears N. E. and S. W.

57.50 A spring branch comes out at the foot of the bluff, 5 links wide; runs N. W. into swamp.

70.00 | Enter swamp; bears N. and S. To.00 | Leave swamp; bears N. and S.

37.50

40.00

41)

Sach Roundsey, T 25 N, R 2 W, Willamette Meradian.

thains The swamp contains about 15 acres, the greater part in section 31.

74.73 The corner to Townships 24 and 25 N., Ranges 2 and 3 W.

Land - except the swamp, rolling, good, rich soil.

Timber - Sugar-tree, Beech, Swamp Maple, Jan. 25th, 1854.

Between Ranges 2 and 3 West, from corner to Townships 24 and 25 N., I run

North, on the range line between sections 31 and 36, Variation 18° 56' East.

Set a post on the left bank of Chickeeles river, for corner to fractional sections 31 and 36, from which

A Hackberry, II in. dia., bears N. 50° E.
Il links dist

A Sycamore, 60 in. dia., bears S, 15° W. 24 links dist.

I now cause a flag to be set on the right bank of the river, and in the line between sections 31 and 36. I now cross the river, and from a point on the right bank thereof, west of the corner just established on the left bank, I run North on an offset line, 25 chains and 94 links, to a point 8 chains and 56 links west of the flag. I now set a post in the place of the flag, for corner to fractional sections 31 and 36, from which

A Beech, 10 in. dia., bears N. 2° E. 12 links dist (7)

Between Ranges 2 and 3 W., T. 25 N., Williamette Meridian.

A Black Oak, 12 in. dia , bears N. 80° W. Chains. 16 links dist. The corner above described. 34 50 Set a post for \(\frac{1}{2} \) section corner, from which 40.00 A Burr-oak, 20 in. dia., bears N. 37° E. 26 links dist. A Black Oak, 24 in. dia, bears N. 80° W. 16 links dist. A Black Walnut, 30 in. dia. 43.41 Set a post for corner to sections 30, 31, 25, 80.00 and 36, from which A Beech, 14 in. dia., bears N. 20° E. 14 links dist. A Hickory, 9 in. dia., bears N. 25° W. 12 links dist.

A Beech, 16 in. dia., bears S. 40° W. 16 links dist.

A White Oak, 10 in. dia., bears S. 44° E. 20 links dist.

Land-level; rich bottom; not inundated. Timber-Oak, Hickory, Beech, and Ash.

In like reamor all the other Township lines are run.

General Description.

This township contains a large amount of first-rate land for farming. It is well timbered with Oak, Hick-ory, Sugar-tree, Walnut, Beech, and Ash.

Chickeeles river is navigable for small boats in low water, and does not often overflow its banks, which are from ten to fifteen feet high.

The township will admit of a large settlement, and should therefore be subdivided.

35,

45

50

(h)

Field Nova of the Substitution Lines and Mounders f Ch. keeles River, in Township 25 No. R. 2 W. Willamette Merulian.

To determine the proper adjustment of Chains. my compass for subdividing this township, I commence at the corner to Townships 21 and 25 N, R. 1 and 2 W, and run

North, on a blank line along the East Boundary of section 36, Variation 17° 51' East.

To a point 5 links west of the quarter 40.05 section corner.

To a point 12 links west of the corner to £0.00 sections 25 and 36.

> To retrace this line, or run parallel thereto, my compass must be adjusted to a variation of 17° 46' East.

Subdivision commenced Feb. 1, 1854.

From the corner to sections 1, 2, 35, and 36, on the South Boundary of the Township, I run

North, between sections 35 and 36, Variation 17° 46' East,

A Beech, 30 in. din. 9 19

29 97 A Beech, 30 in. dia.

40.00 Set a post for quarter section corner, from which

> A Beech, S in. dia., bears N. 23° W. 45 links dist

> A Beec 15 in dia, ben S 18° E 12 links dist.

51.00 A Beech, 18 in. dia. 76.00 A Sugar-tree, 30 in. dia.

	(14)
Town	ship 25 N., Range 2 W., Willamette Meridian.
Chains, 80,00	Set a post for corner to sections 25, 26, 35, and 36, from which A Beech, 28 in. dia., bears N. 60° E 45 links dist. A Beech, 24 in. dia., bears N. 62° W. 17 links dist. A Poplar, 20 in. dia., bears S. 70° W. 50 links dist. A Poplar, 36 in. dia., bears S. 66° E. 31 links dist. Land—level, second-rate. Timber—Poplar, Beech, Sugar-tree, and some Oak; undergrowth—same, and Hazel.
9 (0) 15,00 [40,00	East, on a random line between sections 25 and 36, Variation 17° 46' East. A Brook, 20 links wide, runs north. To foot of hills, bears N. and S. Set a post for temporary quarter section

post for temporary quarter section corner.

55.00 To opposite foot of hill, bears N, and S, 72.00 A brook, 15 links wide, runs N.

80,00 Intersected East Boundary at post corner to sections 25 and 36, from which corner I run

West, on a true line between sections 25 and 36, Variation 17° 46' East.

40,00 Set a post on top of hill, bears N. and S. from which

A Hickory, 14 in. dia, bears N. 60° E 27 links dist.

A Beech, 15 in. dia, bears S 74° W. 9 links dist

(10)

Tanal p 25 N, Range 2 W., Willamette Meridian.

The corner to sections 25, 26, 85, and 36.
Land - east and west parts, level, first-rate;
middle part, broken, third-rate.
Timber - Beech, Oak, Ash, etc.; under-
growth—same, and Spice in the bottoms.
North, between sections 25 and 26, Vari-
ation 17° 46' East.
A Poplar, 40 in. dia.
A Brook, 25 links wide, runs N. W.
A Walnut, 30 in. dia.
A Brook, 25 links wide, runs N. E.
Set a post for \ sec. corner, from which
A Burr-oak, 36 in. dia., bears N. 42° E. 18
links dist.
A Beech, 30 in. dia., bears S. 72° W. 9
links dist.
A Beech, 30 in. dia.
Set a post for corner to sections 23, 24, 25,
26, from which
A White Oak, 11 in. dia., bears N. 50° E.
40 links.
A Sugar-tree, 12 in. dia., bears N. 14° W.
31 links.
A White Oak, 13 in. dia., bears S. 38° W.
32 links
A Sugar-tree, 12 in. dia., bears S. 42° E.
14 links.
Land - level on the line; high ridge of
hills through the middle of section 25, run-
ning N. and S.

Timber Beech, Walnut, Ash, Maple, etc.

(11)

Township 25 N., Range 2 W., Williamette Merulian.

Chains In like manner other subdivison lines are run.

Notes of the Manuters of a Small Lake in Section 26.

Begin at the 1 sec. cor on the line between sections 23 and 26, run thence South

To the margin of the lake, where set a post for meander corner, from which

A Beech, 14 in. dia., bears N. 45° E. 10 links dist.

A Beech, 9 in. dia., bears N. 15° W. 14. links dist.

Thence meander around the lake as follows: S. 53° E. 17.75. At 75 links, cross outlet to lake 10 links wide, runs N. E.

S. 3° E. 13.00.

24.00

S. 30° W. 8 00:

S. 65° W. 12.00 to a point previously determined 20.30 chains North of the quarter section corner on the line between sections 26 and 35.

Set post meander corner, Maple, 16 in. dia , bears S. 15° W. 20 links dist.

Ash, 12 in. dia., bears S. 21° E. 15 links dist.

N. 63° W. 10.00 N. 13° W. 21.00 In this vicinity we discovered remarkable fessil remains of an mals well worth the attention of naturalists

1.2)

T . of p 25 N. Rame 2 W. Willamette Meridian,

Clark N. 52° E. 17.30 to the place of beginning
This is a beautiful lake, with well-defined
banks from 6 to 10 feet high.
Land — first-rate.

Meanders of the left bank of Chickeeles River.

Begin at the corner to fractional sections 4 and 33, in the North Boundary of the Township, and on the left and S. E. bank of the river, and run thence down the stream with the meanders of the left bank of said river, in fractional section 4, as follows:

Courses.	Dist.	l Remarks.
8.76°W.	18.50	
861°W.	10,00	
\$.59°W.	8.30	To the corner to fractional sections
		4 and 5; thence in section 5,
8.54°W.	10.70	, , , , , , , , , , , , , , , , , , , ,
8.40°W.	5.60	
8.50°W.	850	
S.37°W.	17.00	
8 44°W	22 00	
S.38°W.	26.72	To the corner to fractional sections
		5 and 8; thence in section 8,
8.21°W.	16.00	The section of
S 10°W.	13.00	
South	8.50	To the head of rapids
S.9°E.	5.00	talifile
S.17°E.	20.00	
S.10°E.	12.00	To the foot of rapids.
8.221°E.	8.46	To the corner to fractional sections
		8 and 17
	1	
'	,	Land, along fractional section 8,

(13)

Township 25 N., Range 2 W., Willamette Meruluan.

Courses.	Dist.	Remarks.
		high, rich bottom; not inundated. The rapids are 37 00 chains long; rocky bottom; estimated fall, 10 feet. Meanders in Section 17.
S.17°E.	15.00	At 5 chains, discovered a vein of coal, which appears to be 5 feet thick, and may be readily worked.
S.8°E.	12 00	
S.4°W.	22 00	At 3 chains, the ferry across the river to Williamsburgh, on the opposite side of the river.
S.25°W.	17.00	
8.78°W.	1200	
8.71°W.	9.55	To the corner to fractional sections 17 and 18; thence in section 18,
S.65°W.	15.00	
873}°W	15.93	To the corner to fractional sections 18 and 19.
8.65°W	11.00	In section 19.
8.60°W.	23.00	
\$.42° W.	10.00	
\$.20°W	10.00	
S16¾°W	13.83	pond and lake, 50 links wide, to the corner to fractional sections 19 and 24, on the range line, 32 50 chains North of the corner to sections 19, 30, 24, and 25.

The above selections will serve as specimens of the manner of taking the field notes

266. General Description.

The quality of the land in this township is considerably above the average. There is a fair proportion of rich bottom-land, chiefly situated on both sides of Chackeeles river, which is navigable, through the township, for steambouts of light draft, except over the rapids in Section 8.

The uplands are generally rolling, good first and second rate land, etc.

267. Certificates.

I, Robert Acres, Deputy Surveyor, do solemnly swear that, in pursuance of a contract with surveyor of the public lands of the United States, in the State [or Territory] of shearing date the day of the United States and the instructions furnished by the said Surveyor-General, I have faithfully surveyed the exterior boundaries [or subdivision and meanders, as the case may be] of Township number twenty-five North of the base line of Range number two West of the Willamette Meridian, in the aforesaid; and do further solemnly swear that the soling are the true and original field notes of such survey.

ROBERT A. ...

Deputy Surveyor.

Subscribed by said Robert Acres, Deputy Surveyor, and sworn to before me a Justice of the Peace for the County in the Charter of the Peace for the

County, in the State [or Territory] of this day of , 18 .

HENRY DOOLITTIE,

Justice of the Pence.

We hereby certify that we assisted Robert Acres, Deputy Surveyor, in surveying the exterior boundaries, and subdividing Township number twenty-five North of the base line of Range number two West of the Willamette Meridian, and that said Township has been, in all respects, to the best of our knowledge and belief, well and faithfully surveyed, and the boundary monuments planted according to the instructions furnished by the Surveyor-General.

Peter Long, Chainman,
John Short, Chainman,
George Sharr, Arman,
Adam Dull, Arman,
Henry Flagg, Compassman,

Subscribed and sworn to by the above named persons, before me, a Justice of the Peace for the county of , in the State [or Territory] of , thus day of , 18 .

Henry Doolittie,

Justice of the Peace

SURVEYOR'S OFFICE AT , 18

The foregoing field notes of the Survey of [here describe the survey], executed by Robert Acres, under his contract of the day of , 18, in the month of , 18, having been critically examined, the necessary corrections and explanations made, the said field notes, and the surveys they describe, are hereby approved A. B.,

Surreyor-General

To the notes of each Township, in the copies of the field notes transmitted to the seat of government, the Surveyor-General will append the following certainates

A 1. 115

1 cert by that the foregoing transcript of the field in the of the Survey of the [here describe the character of the surveys, whether meridian, base line, standard parallel, exterior township lines, or subdivision lines and meanders of a particular township], in the State [r Territory] of ______, has been correctly copied from the original notes on file in this office. A. B.,

Surveyor-General.

268. Corners and Boundaries Unchangeable.

According to an act of Congress, entitled "An act concerning the mode of Surveying the Public Lands of the United States," approved February 11th, 1805, and still in force,

by the Surveyor-General, shall be established as the proper corners of sections or subdivisions of sections which they were intended to designate; and the corners of half and quarter sections, not marked on said surveys, shall be placed, as nearly as possible, equidistant from those two corners which stand on the same line."

2d. "The boundary lines actually an and marked in the surveys returned by the Surveys General, shall be established as the proper boundary lines of the sections or subdivisions for which they were intended; and the length of such lines, as returned by the Surveyor-General aforesaid, shall be held and considered as the true length thereof."

If it is afterward found that a post is out of line, or that the line has been unequally subdivided, the general government only has the power of correction, and that only while it holds the title to the lands affected.

Such boundaries only as are established by the Surveyor-General, or the deputy, in the performance of his official duties, and in accordance with law, como under the above rules.

269. Restoring Lost Boundaries.

Lost boundaries must be restored in conformity with the laws under which they were originally established.

At an early day, three sets of section corners were established on the range lines; later, two sets on all the township boundaries; at present, the section lines close on previously established corners on township corners, making one set of corners, except on the base lines and standard parallels, where double corners—standard corners and closing corners—are established.

In order to restore lost boundaries correctly, the surveyor must know the manner in which townships were originally subdivided.

In case of three sets of corners on the range lines, one set was planted when the exteriors were run

Corners on the east and west lines between two townships, belong to the sections of the township north.

From these corners, section lines were run due north, which would not, in general, close on the corners of the township line on the north, thus making two sets of corners on the north and south boundaries of the township.

The east and west lines were run due east and west from the last interior section corner, and new corners established at the intersections with the range lines.

In case of two sets of corners, the subdivisions were made as above, except that the cast and west lines

were closed on the corners previously established on the cast boundary, but were run due west from the last interior section corner to the range line, and new section corners established at the intersection with the range line.

The method of making but one set of corners, excapt on the base line and standard parallels, is the one now in vogue, and has been sufficiently considered.

270. Restoring Lost Corners.

Lost corners must be restored, if possible, to their exact original position.

The surveyor should seek to accomplish this, first, by the aid of bearing trees, mounds, etc., described in the original field notes.

If the corner can not be located in this way, good testimony may be taken.

It often happens that in retracing lines, the measurements do not agree with the field notes. When such cases occur, from whatever cause, the surveyor must establish his corners at intervals proportional to those given in the original field notes

1. To restore a lost corner common to four sections.

Find the distances between the nearest noted line trees or well-defined corners, north and south, and east and west of the lost corner. Establish the corner between them at a point intercepting distances proportional to those given in the original notes

2. To restore one of a double corner when the other is standing.

First ascertain to which sections the existing corner belongs. Then re-establish the lost corner in the

direction and at the distance stated in the original notes. Verify the work by chaining to noted line trees or corners, having previously compared your chaining with that of the United States deputy by rechaining between corners noted in the original survey, and making all distances proportional.

3. To restore that one of a double corner established in running the township lines when both are moving.

Run a straight line between the nearest noted line trees or corners on the line, and, at the distance given in the notes, establish the corner which will be common to two sections north or west of the line.

Let the accuracy of the result be verified by measuring to the next section corner west or north

4. To restore that one of a double corner established in subsliving the township when both are missing.

Retrace the section line which closed on the corner, and establish the section post at the intersection with the township line. Verify the result by measuring on the township line to noted objects.

The rest of corner will be common to two sections south or east of the line.

5. To restore one of a triple corner, on a range line when one at least remains standing.

The one of the triple corner, established when the range line was run, is not a section corner.

First identify the existing corners, then establish the lost corner, according to the field notes, north or south of the existing corner, on the line, and verify the result

If the field notes do not give the distances between the triple corners, retrace the section line closing on said corner

6. To restore a triple corner when all are lost.

Rechain the range line, and retrace the section lines closing on the range line.

7. To restore lost quarter section corners.

1st. Except on those section lines which close on the north or west boundaries of a township, quarter section corners are equidistant between the two section corners. Hence, rechain the section line, then chain back one-half the distance.

2d. On township lines, where there may be double section corners, only one set of quarter section corners are actually marked in the field — those established when the exteriors are run half-way between the section corners established at the same time. These are restored as above

The same will apply when there are triple corners.

3d. If the section line closes on the north or west boundary of a township, the quarter section corner must be established 40 chains of the original measurement from the last interior section corner.

8. To restore lost township core

Ist. If the corner is common to four townships, retrace the township and range lines, and establish the corner at their intersection

2d. If the corner is common only to two townships, as may be the case on the base line or standard parallels, retrace the base line or standard parallel from the

last standing corner, if the lost corner is common to two townships north; but if the lost corner is common to two townships south, retrace also the range line.

9. To restore last mounder corners.

Retrace the lines which close upon the banks in the direction they were originally run.

Fractional section lines closing on Indian boundaries, private grants, etc., should be retraced, and the corners established in the same manner.

Remark.—If, in restoring a lost corner, the original corner is found by some unmistakable trace, it must stand, and the resurvey be made to correspond.

271. Subdividing Sections.

The United States deputy runs only the exterior or section lines, and makes the section and quarter section corners.

Lines printing the opposite quarter section corners divide the section into quarter sections of 160 acres each.

40 A. 40 A.

100 A.

THE ACT TO A

These quarter sections are divisible into half-quarters of 80 acres, and these into quarter-quarters of 40 acres

These are the legal subdivisions of a section, and are exhibited in the annexed diagram.

If private parties wish the subdivision lines traced on the ground, they employ the county surveyor, or a private surveyor, who must be governed by the section and quarter section corners previously established

The following rules will enable the surveyor to subdivide a section in accordance with the laws of the United States

- 1. The original section and quarter section corners mest stand where they were established by the government surveyor.
- 2. The quarter-quarter corners must be established equidistant, and on the line between the section and quarter section corners of the exterior lines of the section, and equidistant and on the line between quarter section corners of internal lines of the section.
- 3. All subdivision lines must run straight from the proper corner in one exterior line of the section to the corresponding corner in the opposite exterior line.
- 4. In fractional sections, where no opposite corresponding corner has been established, the subdivision line must be run from the given corner due north and south, or east and west, to the exterior boundary of said fractional section.
- 5. Anomalous sections or sections larger than a mile, sometimes close on a previously established line, in finishing up a public survey.

Quarter section and section corners are established 40 chains and 80 chains, respectively, from the previously established corners, and posts are planted every 20 chains of the remaining distance

Anomalous sections are subdivided by running straight lines from the corners on the south line to the corresponding corners on the north, and east, and west lines, the same as in regular sections

VARIATION OF THE NEEDLE.

272. Definitions and Illustrations.

The variation of the needle is the angle which the magnetic meridian makes with the true meridian.

The variation is cust or west, according as the north end of the needle is east or west of the true meridian.

The variation is different at different places, and it does not remain the same at the same place

The line of no variation is that line traced through those points on the surface of the earth where the needle points due north.

At all places east of this line, the variation is west; and at all places west of this line, the variation is east.

West variation is designated by the sign plus, and cast variation by the sign minus.

In the var 1840, at a point whose latitude is 40° 53', and lengitude 80° 13', being a little S. E. of Cleveland, O., the variation was nothing. The line of no variation : d through this point N 24° 35' W, and S. 24° 35 |

273. Changes of Variation.

- 1. Irregular changes. The needle is subject to sudden changes coincident, in time, with a thunder storm. an aurora borealis, solar changes, etc.
- 2. Diurnal changes. In the northern hemisphere. the north end of the needle moves from 10' to 15' west from about 8 A. M. to 2 P M, and then gradually returns to its former position

S N 21

3 Annual changes. The diurnal changes vary with the season, being about twice as great in the summer as in the winter

4 Secular changes.—In addition to the above changes, there is a change of variation, in the same direction, randing with considerable regularity through a period of about 234 years, as is indicated by observations at Paris.

In the United States, the north end of the needle was moving east from the earliest recorded observations till about the year 1810, since which time the movement has been west, at the rate, on an average, of about 5' per annum.

We give the following tables of places, their latitude and longitude, and variation as it was in 1840, and the annual change of variation, from the tables prepared by Professor Loomis for the 39th and 42d volumes of Silliman's Journal:

Places near the Line of no Variation.

Places,	Lat.	Low.	Tar.	An. Mo.
A Point.	40° 53′	80° 13'	0.00	+ 4'.4
Cleveland, O.	41° 31′	819451	- 0° 19'	4'.4
Mackinaw.	45° 51′	849 411	-2° 08′	3'.9
Charlottesville, Va.	39° 02°	78° 30'	1 0° 19′	31.7

Assuming the annual motion unifor a and correctly found for 1840, the variation for any absequent time can be found by multiplying the annual motion by the number of years since 1840, and taking the algebraic sum of the product and the variation at that date.

Places where the Variation was West.

Places.	Lat	Lon	Var	An Mo.
Point in Maine.	48° 00′	67° 37'	+ 19°30′	+ 8'8
Waterville, Me.	44° 27′	69° 32'	12°36'	57.7
Montreal.	45° 31′	73° 35′	10° 18'	377
Burlington, Vt.	44° 27′	73° 10'	9° 27'	5' 3
Hanover, N. H.	43° 42′	72° 14'	9° 20	5'.2
Cambridge, Mass.	42° 22′	71° 08′	9° 12	5'.
Hartford, Conn.	41° 46′	72° 41′	6° 58	5'
Newport, R. I.	41° 28'	71° 21′	79 45	5',
Geneva, N. Y.	42° 52'	77° 03'	40 18	4'1
West Point	41° 25'	74° 00′	6° 52'	41.
New York City.	40° 43	71° 01′	5° 34′	316
Philadelphia.	39° 57′	75° 11'	41.08	312
Buffalo, N. Y.	42° 52′	79° 06′	1° 37′	11 [

Places where the Variation was East.

Placos	Lot	Lan.	Var	An. Mo
Jacksonville, Ill.	310 43'	90° 20′	8025	- 2.5
St. Lo. a. Mo.	350 37	90° 17' 1	8º 37'	213
Nashville, Tenn.	36° 10'	86° 52"	6° 42'	2'.
Lean	29° 40'	91° 00'	8° 41'	1'4
Mohn V.	30° 42′	88° 16' 1	70 (6)	1'4
Tusca Ma	.33° 12'	87° 43'	7° 26'	1'6
Columbia Con	329 28	85° 11'	20 58,	2',
Milledgevi - Ca	.33° 07′	83° 24'	50 07'	21.4
Savanna Con	329 051	81° 12′	47.137	2.7
Tallahas Pa	30° 26'	810 27	20 00.	1.5
Pensacola, Pa	30° 24'	870 23	58 53	17.4
Logansport, Ind.	40° 45'	S60 22"	29.54.	2.7
Cincinnati, O	399 00	84° 27	47.16	3.1

274. Methods of Ascertaining the Variation.

First estadish a true meridian, which may be done I Briton soy Burt's Solar Compass,

2 By observation of the North star, when on the merulum.

The north star is about 1° 22' from the true pole, around which it revolves in a siderial day, or 23 h, 56 m., 4 s.

Twice in this period the star will be on the meridian.

The exact moment of its passage can be determined very nearly, from the fact that it reaches the meridian almost at the same instant as Alioth in the tail of the Great Bear, or the first star in the handle of the Dipper.

Suspend a plumb line a few total in front of the telescope and pluma a faint light near the object glass of the telescope, so that the spider lines may be seen.



Just 17 minutes after the plumb line, the North star, and Alioth all fall on the vertical species, the North star is on the meridian.

The horizontal limb of the instrument is then firmly clamped, and the talescope is turned to horizontally.

A light, shining through a small aperture in a board, at some distance, say ten rods, is moved by an assistant, according to signals, till it ranges with the intersection of the spider lines

A stake driven into the ground directly under the light, and another directly under the tracepe, will mark, on the ground, the true meridian.

The season of the year may be such that Alioth may be above instead of below the North star, when both are on the meridian at night. With the telescope, the stars can be seen in the day-time.

3. By the azimuth of the North star,

When the North star is farthest from the meridian, east or west, it is said to be at its greatest eastern or western clongation.

The azimuth of a star is the angle which a vertical plane, through the star, makes with the meridian plane

Let us now find the azimuth of the North star at its greatest elongation.

Let Z be the zenith, P the pole, S the North star at its greatest elongation, ZP, ZS, and PS area of great circles. Then ZPS will be a spherical transle, right-angled at S, and the angle Z will be the azimuth, PS the greatest elongation, and ZP the com-

plement of latitude, since the elevation of the poleabove the horizon is equal to the latitude.

New from Napier's principles, we have

$$\sin e = \cos l \cos (90^{\circ} - Z).$$

$$\therefore \sin Z = \frac{\sin e}{\cos l}.$$

Introducing R and applying logarithms, we have

$$\log \sin Z = 10 + \log \sin \epsilon + \log \cos L$$

House, the azimuth is readily computed if we know the greatest elongation of the star and the latitude of the place.

Gravest Fountian of Polaris,

	_				
Date	R puin	Pate	Elonquition.	Date.	Elongation.
1870	18 23 01%	1880	1° 19' 50".4	1890	1° 16′ 40′.7
1871	1° 22' 41" 9	1881	1° 19′ 31″.4	1891	1° 16' 21" 8
1872	1, 55, 55, 8	1882	1° 19′ 12″.5	1892	1° 16' 03"
1873	1° 22′ 03″ 8	1883	1° 18′ 53″.5	1893	1° 15′ 44″.1
1874	F° 21' 44" 8	1884	1° 18′ 34″.5	1894	1° 15′ 25″.3
1875	1° 21' 25" 7		1° 18′ 15″.5		
1876	1° 21′ 06′ 6		1° 17' 56".6		
1877	1° 20′ 47″ 6	1887	19 17 37 6	1897	1° 14′ 28″.7
1878	1° 20′ 28′ 5		1° 17′ 18″ 6		
1879	1° 20′ 09″ 5	1889	1° 16′ 59″ 7	1899	1° 13′ 51″
					- 1

The elongation in the table is given for the 1st of January of each year; but the clong tien by any month of the year can be readily found

Thus, let us find the elongation for May 1st, 1873.

Jan. 1st, 1873,	Elongation 1° 22'	03".8
Jan. 1st, 1874,	Elongation 1° 21'	44".8
Change for 12		19"
Change for 4	months	6.3"

- Then, for May 1st, 1873, we shall have.

 Elongation = 1° 22′ 03′ 8 6 1° 21′ 57″.5.
- I. Find the azimuth of the North star at its greatest elongation, May 1st, 1873 latitude 40°. . .1mt. 1° 47'.
- 2. Find the azimuth of the North star at its greatest elongation, July 1st, 1877. Let tude 12° 1 s 1° 49‡'.
- 3. Find the azimuth of the North star t its greatest elongation, Sept. 21st, 1880—latitude 45, 45.

1 1° 541.

It will be necessary to know the times of the greatest elongation. These times are given in the following tables, for the 1st, 11th, and 21st of each month of the year 1880, which will answer the purpose for the rest of the century, since the change of time is very slow, being only about 16 minutes in 50 years.

Eastern Elongation.

Month.	1st day.	11th day.	21st day.
April.	6h. 40m. A.M	6h, 01m, A M	5h 22m, A.M.
May.	4h, 42m, A.M.	4h. 03m. A.M.	3h. 24m. A M
June.	2h. 41m. A.M	2h, 01m, A M	1h. 22m. A.M
July.	0h. 43m. A.M.	0h. 00m. A.M.	41h, 21m P.M. 1
August.	10h. 38m. P.M.	9h, 59m, P.M.	9h 19m, P.M
Sept.	8h. 36m. P.M.	7h. 57m. P.M	7h. 17m P M

Western Elongation.

Month.	1st day.	11th day	21st day.
Oct.	6h. 31m. A.M.	5h 52m, A M	5h 13m, A.M
Nov.	4h. 30m. A.M.	3h. 50m. A M	3h.11m/A/M
Dec.	2h. 31m. A.M.	1h 52m A M	1h 13m A M
Jan.	Oh. 28m. A.M.		11h 04m P M
F 1	10h, 22m, P.M.	9h. 42m. P M	9h 03m P M
March.	8h. 31m. P.M.	7h. 52m. P.M	7h 13m P M

About half an hour before the greatest eastern or western elongation, place the transit in a convenient position, and level it carefully.

Paste white paper on a board about one foot square, and perforate the board through the center with a two-inch anger, and, on the lower edge, fix some contribution for holding a candle.

Let this board be fixed to a vertical staff, so as to slide freely up and down, and let it be placed about one test in front of the telescope, so that the light reflected from the paper will render the spider lines visible.

sar is visible through the telescope and orifice in the board, and bring the vertical spider line in range with the star.

As the star approaches its greatest elongation, move the telescope by a tangent screw, so as to keep the vertical line in range with the star. When the star reaches its greatest elongation, it will appear, for some time, to coincide with the spider line, and then leave it in the opposite direction.

Clamp the horizontal limb, and turn the telescope down till it is horizontal.

Let now a staff, with a light on its upper end, be carried ten or fifteen rods distant, toward the star, and placed so as to range, when vertical, with the vertical spider line of the telescope

Drive a stake at the foot of the staff, and another directly under the instrument, then will the line determined by the stakes make an angle with the true meridian, equal to the azimuth of the North star. The true meridian will lie west or confidence of the confine of stakes, north of the telescope, according as the elongation was east or west, and may readily be located by the instrument

The location of the meridian can be verified thus:

Let AB be the line of the stakes produced to a considerable distance, say from 20 to 40

chains, A the azimuth angle, AC the true meridian, and BC perpendicular to AB.

BC can be found from the formula,

 $BC = AB \tan A$.

Then laying off BC on the ground, and driving a stake at C, the stakes A and C will trace the true meridian.

Having found the true meridian, the variation of the needle can be readily determined by turning the telescope or the sights of the compass in the direction AC.

Without finding the true meridian, the bearing of AB being equal to the known azimuth of the North star at its greatest elongation, the variation of the needle can be found by directing the telescope or the sights of the compass in the direction AB.

The following method may be resorted to by the surveyor who does not possess an instrument with a telescope.

Fix a plank, firmly level, east and west, about three feet above the ground; then take a board about six inche square, and having detached one of the comparts. Its, fix it to the board, at right angles with its upper edge. Drive a nail obliquely a little way into the board, so that it can be tacked to the plank

About fifteen feet north of the plank suspend a plumb line, from the top of an inclined stake of such height that the North star, when seen through the sight while the board rests on the plank, will appear about one foot below the upper end of the plumb line.

Suspend the plumb in a vessel of water to prevent the line from vibrating, and let an assistant held a light near it, so that it can be seen through the secht About half an h ar before the time of the greatest of the net the North star, place the board on the place, and slade so that the star and plumb line shall reads when some through the sight. As the star applicable its greatest elongation, move the board along the plack in the opposite direction, so as to keep the range.

When the star reaches its greatest clongation, it will appear to keep the range for several minutes, then it will move slowly in the opposite direction.

Tack the board to the plank, taking care not to change its position. Then let a staff with a light on its top be placed about ten rods farther to the north, so as to range, when vertical, through the sight, with the plumb line.

Drive a stake at the foot of the staff, and one directly under the plumb line, then will the line of the stakes make, with the meridian, an angle equal to the azimuth of the North star at its in the longation.

The true meridian, and the variation of the compass, can then be found as above.

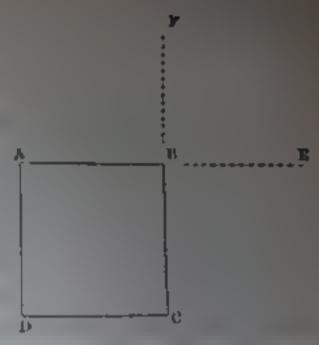
FIELD OPERATIONS

275. Finding Corners.

In searching for a corner, first see a the monument, whether tree, post stake, or sten a given and witnessed in the ories tell notes, a h, if found, must be considered access in establish, a the corner.

If no monument to the found, the corner can often be found by and metro thous, of which the following are the most available:

Thus, if a monument can be found at each of the corners A, C, D, but not at B, find the corners E and F, at each of which set up a flagstaff or high pole, and send the flag-man as near to B as possible, and let him stand facing D, so that he can see signals made both at A and C.



The observer at A can, by waving his hand, bring the flag man in the line AE, and the observer at C can bring him in the line CF, and being in both lines, AE and CF, at the same time, he will be at their intersection B, the corner required.

If the corner E can be found, but not F, measure AB the required distance in the line AE. If the distance AB is not known, but it is simply known that AB is equal to DC, first measure DC. If neither E nor F can be found, run AB parallel to DC, and CB parallel to DA, and the intersection of these lines will determine B, if the field is a parallelogram.

If the field is not a parallelogram, retrace one of the lines terminated by known corners, and compare the bearing with the bearing in the original notes, which will give the variation of the needle. Then run the lines AB and CB from the notes, allowing for the variation, and the intersection will determine B

In I I, a case r two or more last corners may be found.

If the termings and distances are given in the original in the only but one corner can be found, retractions established have in the neighborhood to find the viriation, and, beginning at the known corner, run the lines from the notes, allowing for the viriation

The importance of allowing for the variation may

Let the full lines bound the lot.

If the sarveyor should run this lot form the original notes, one corner being known, the dotted lines would



the corners, thus encroaching on one side, and leaving gaps on the other, which of course would never do.

276. Finding Bearings and Distances.

After finding the corners, set a stake at each, and, beginning at any corner, place the compass or transit directly over the stake, and send the flag-man to the next corner, who must place the flag-staff on the stake.

Take the bearing, and measure the distance as heretofore directed; and, in like manner, find the bearings and distances of the remaining sides

If obstacles should prevent the tance of the bearing of any line, measure the same distance from each corner, at right angles to the line, on the same side, so as to secure a line free from obstacles, and take the bearing of this line, which will be to be ring of the required line, since they are parallel.

Lines are measured a little to one side when fences, ponds, or other obstacles, are in the line

Thus, if the perpendiculars AC and BD are equal.

$$CD = AB$$

AB can be found by Trigo- A nometry, if AE and ER and two angles be measured



277. Offsets.

Offsets are perpendiculars measured from a line to the angles of a neighboring broken line, or to the banks or centers of creeks, rivers, or other bodies of water. Thus, a, b, c





278. Taking Field Notes.

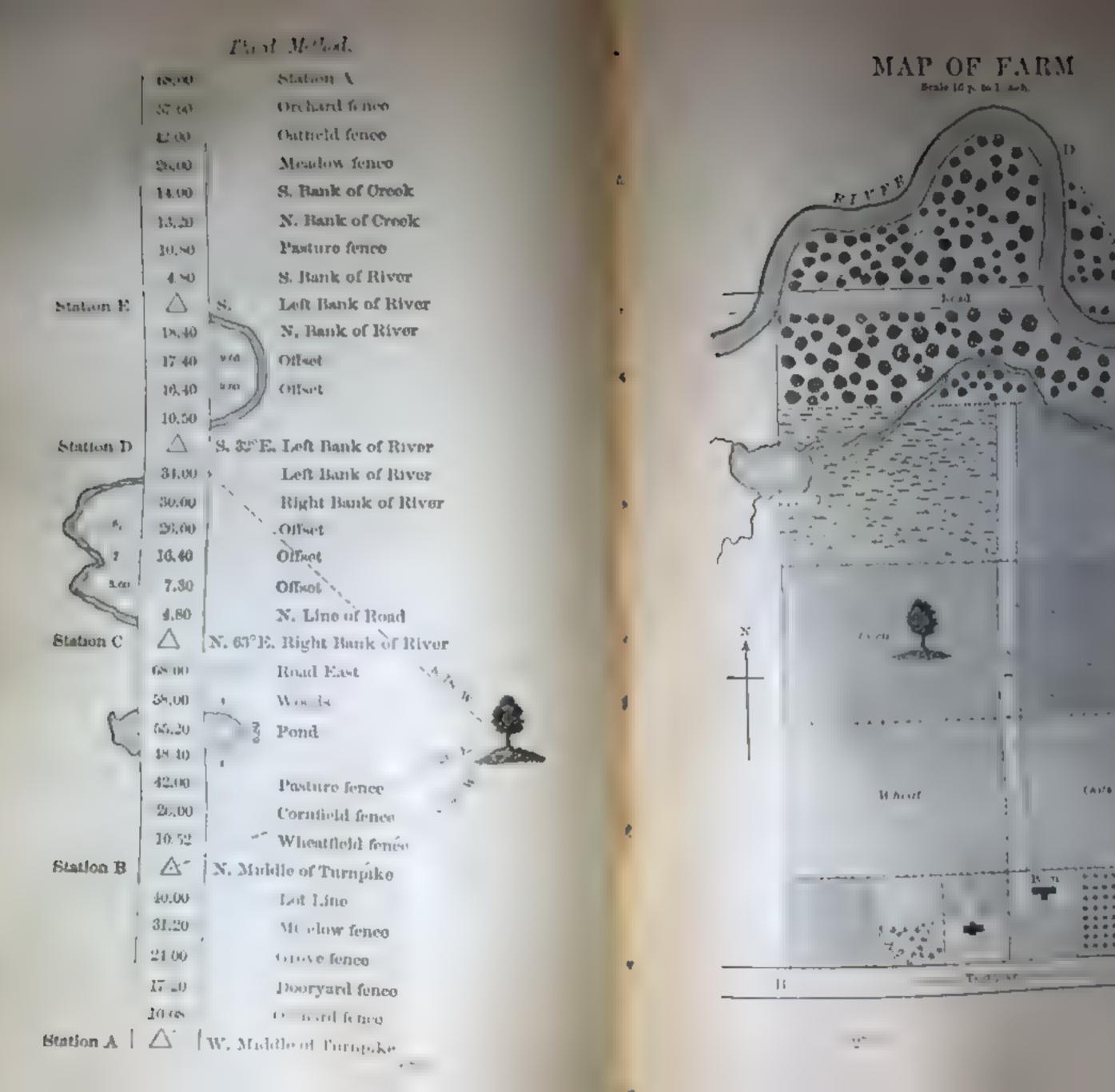
First Method.

Second Method

She	Bearings.	Dist	22 41 00	
1	N 20° E.	15 50	RR	
2	E.	18.00	1	
3	S. 20° E.	30.00	4,4	
1 4	W.	, 25,00		
	N 321° W.	10'08		15 FG

The first method is in the proper form for calculation, and may be conveniently employed when it is not a pertant to make a map of the lot surveyed

The second method, being a random outline with bearings and distances indicated, may be employed when it is desirable for the surveyor to keep betore bins, while at work, an outline of the lot



279. Remarks on the Third Method.

The third method should be employed whenever a mat, more or less perfect, is to be made. The notes at said be placed on a left-hand page of the field book. and the map on the right page, facing.

By referring to the notes and map illustrating this method, it will be observed that the survey began at A, the S. E corner of the farm, at the middle of the turnpike, and that we commenced to record the notes at the bottom of the page.

This will keep the notes of the objects, at the right or left of each line run, in their natural position on the page, at the right or left of the parallel lines inclosing the distance from the station at the beginning of the line to the objects worthy of record encountered in running the line.

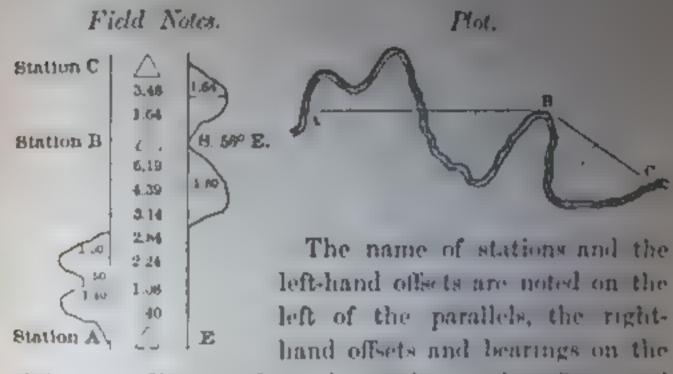
The character \(\triangle \) denotes station, at the left of which stands the letter marking its position on the map, and at the right the bearing of the next course.

A prominent object, such as the chimney of the house, a large tree standing in an open field, may be selected, and its bearings from the principal stations be taken. These bearings will serve as checks against errors in drawing the map, and may aid in finding the corners should they be lost. In the present example, a chestnut tree on the top of a hill, in the pasture at the left of the lane, is selected, and it- ining from A, B, and D given

280. Surveying Creeks and Roads.

I. Creeks may be meandered as described under the head of Survey of the Public Lands

2. They may also be surveyed by running straight lines connecting points on the bank, taking the bearings of these lines, the distances from the origin of these lines to the perpendicular offsets run from the lines to the bank of the river, and the length of the offsets, as exhibited in the following field notes and plot.



right, the distance from the station to the offsets, and the sign for station, between the parallels.

3. In surveying an existing winding read, keep in the road, run straight lines as far as possible, without running out of the road, note the bearing of these lines, the d tances to the offsets at different points to the sides of the road, the lengths of these offsets, and make an accurate plot of the road.



4. To survey a new road, find the bearing of the middle line from the origin to the next angle or intersection with another road, measuring the distance

× N 21

from the origin to the lines of farms, creeks, etc., which it interests.

street, and plant monuments at a given distance and bearing from the angular points, so that they will not be disturbed in making or working the road. Take notes, and make a correct plot of the road.

281. Surveying Towns.

Commence at the intersection of principal streets, take their bearings, measure their lengths, noting the distances to the streets and alleys crossed, taking offsets to corners of streets and prominent objects, as public buildings, etc., till a prominent cross-street is reached, which survey in the same manner, changing the courses at such stations as will lead back to the original station.

Survey all the streets and alleys enclosed. Then survey an adjoining district, and so on, till the entire town or city has been surveyed

Take notes, and make an accurate map of the town, on which locate not only the streets and alleys, but public buildings, parks, fountains, mont ments, etc.

282. Reverse Bearing.

Let AB be a line run from A to B, $A\Lambda$ and BS meridians, then will NAB be the bearing of AB, and SBA will be the reverse bearing.

Since the meridians AN and BS may be regarded as parallel, the bearing and reverse



pearing are equal. Thus, if the bearing of AB is N. 30° E., the reverse bearing is S. 30° W.

The bearing and reverse bearing agree in the value of the angle, and differ in both the letters which indicate the general direction of the line. In fact, the reverse bearing of a line is the bearing of the line if run in the opposite direction. Thus, SBA, the reverse bearing of the line AB, run from A to B, is the bearing of the line BA, run from B to A.

Of the letters used in bearings, we shall call N and S latitude letters, and E and B' departure letters.

To guard against inaccurate observations, and the disturbance of the needle occasioned by local attraction, the reverse bearing should be taken at every station. If the bearing and reverse bearing agree in value, the bearing may be considered as correctly taken; if they differ materially, both should be taken again. If they still differ, the difference may be regarded as occasioned by local attraction.

To ascertain at which station the local attraction exists, place the instrument at a third station, at a considerable distance from each of the doubtful stations, and sight to each, then from these back to the third station. The local attraction may be considered to exist at the station where the bearing of the third station distagrees with its bearing taken at the third station.

If the error occurred in the foresight, correct it before entering the bearing in the field notes, and note the an and of disturbance; if the error occurred in the bucksight, the next foresight will be affected, and should be corrected before entered.

PRELIMINARY CALCULATIONS.

283. Angles between Courses.

1. If the latitude letters are alike, also the departure letters, the included angle is equal to the difference of the bearings

If AB bears N. 40° E., and AC N. 20° E.,
$$BAC = BAN - CAN = 40°$$
 $-20° = 20°$.

If AD bears S, 40° W., and AE S. 20° W., $DAE = DAS - EAS = 40^{\circ}$ $-20^{\circ} = 20^{\circ}$.



2. If the latitude letters are alike, and the departure letters unlike, the included angle is equal to the sum of the bearings.

If AB bears N. 38° E., and AC N. 18° W., BAC = BAN + NAC 38° + 18° = 56°.

If AD bears S. 38° W., and AI S. 18° E., DAE = DAS + SAE 3° + 18° = 56°.



3. If the latitude letters are unlike, and the departure letters alike, the included angle is equal to 180° minus the sum of the bearings.

If AD bears S = 1 - 3V, and 4C $N.30^{\circ} W_{\circ} DAC = 180^{\circ} \cdot (DAS + CAN) = b$ \Rightarrow \Rightarrow $\Rightarrow 180^{\circ} - 75^{\circ} - 105^{\circ}$. 4. If the latitude letters are unlike, also the departure between, the included angle is opial to 180° me nos the difference of the bearings

If AB bears N 45° E, and AC S. 15° W., $BAC = 180^{\circ} - (NAB - 8AC) - 180^{\circ} - 30^{\circ} - 150^{\circ}$.

If AD bears S. 45° W., and AEN. 45° E., $DAE = 180^{\circ} - (8AD - \frac{1}{6})$ $NAE) = 180^{\circ} - 30^{\circ} - 150^{\circ}$.

Remark. These principles apply when both courses run from or toward the vertex; if one runs from the vertex, and the other toward it, reverse the bearing of one side before applying the principle.

284. Examples.

1 Fit d the angle A, if AB bears N 78° E, and AC N. 24° E. Ac., 54°.

2 Find the angle A, if BA bears S 31° E, and AC S. 48° W

3. Find the angle A, if BA bears S 70° W, and C4 N. 25° E.

A = 1 %

4. Find the angles of the polygon AECPE, if AE bear N cook E., RC, N. 60° E.; CD, S. 50° E; PL, S. 10 W / 1 N. 78° W.

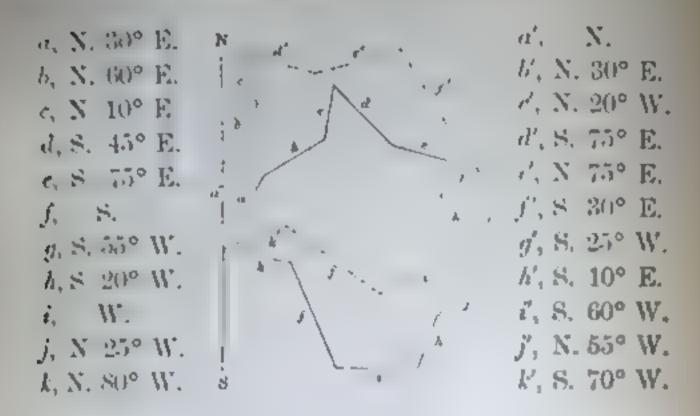
.1 72°, B = 150°, C = 110°, D = 90°, E = 118°

285. Problem.

to the field be supposed to rear a tell of the late.

the side to become a meridian.

In the following diagram let the full lines denote the original position of the sides of the field, a the side that is to become the meridian, and the dotted lines the revolved position of the sides.



From the above illustration we derive the following principles:

1. If the letters which indicate the general direction of the side which is to be made a meridian are both alike or both unlike those of another side, then,

1st. If the bearing of the former is less than that of the latter, the difference of the hearings will be the bearing of the latter, the letters remaining the same as before.

2d. If the bearing of the former is gother than that of the latter, the difference of the bearing will be the bearing of the latter, the departure letter being changed.

2. If one of the letters which indicate the general direction of the side which is to be made a meridian is like and the other unlike the corresponding letter of another side, then

1st. The sum of the bearings, if less than 90°, will be the bearing of that side, the letters remaining the same as before

2d. If the sum of the bearings is greater than 90°, its supplement will be the bearing of that sile, the latitude letter being changed

286. Examples.

The bearings of the sides of a field are as follows:

 1st, N. 30° E; 2d, N. 60° E; 3d, S. 40° E; 1th, S.
 30° W; 5th, W.; 6th, N. 18‡° W. Find the bearings of the sides if the second side becomes a meridian Ann. 1st, N. 30° W.; 2d, N.; 3d, N. 80° E; 4th, S.

 30° E.; 5th, S. 30° W.; 6th, N. 78‡° W.

2. The bearings of the sides of a field are as follows. 1st, N. 45° W.; 2d, N. 18° E.; 3d, E; 4th, N. 52° E., 5th, S. 42½° E.; 6th, S.; 7th, S. 65½° W. Find the bearings if the first side be made a meridian.

Ans. 1st, N.; 2d, N. 63° E.; 3d, S 45° E; 4th, N 77° E.; 5th, S. 23° W.; 6th, S. 45° W; 7th, N 654° W

3. The bearings of the sides of a field are as follows:

1st, N. 20° E.; 2d, N. 70° E.; 3d, E., 4th, S. 45° E.;

5th, S.; 6th, S. 45° W.; 7th, W.; 8th, N. 3;° W. Find the bearings if the sixth side be made a maridian

Ans. 1st. N. 25° W.; 2d, N. 25° E., 3d, N. 45° E.; 4th, E.; 5th, S. 45° E.; 6th, S.; 7th, S. 45° W. 8d., N. 489° W.

287. Latitude and Departure.

The latitude of a course is the distance between the two parallels of latitude possing through the externation of the course

The departure of a course is the distance between the two meridians passing through the extremities of the course

Lat AB be a course, AD and BC paral-Icls of latitude, and AC and BD meridians. Then will AC or DB be the latitude of the course, and CB or AD its departure.

$$AB \times \cos CAB$$
,

But
$$AC = AB \times \cos CAB$$
,
and $CB = AB \times \sin CAB$.

Hence, latitude = course × cosine of bearing, and departure == course × sine of bairing.

If the line runs due east or west, its latitude is 0. If the line runs due north or south its departure is 0. Latitude north is considered plus; latitude south, minus. Departure cast is considered plus; departure west, minus.

For brevity let us designate the bearing by b, the course by c, the latitude by l, and departure by d, then we shall have the cases given in the following article:

288. Table of Cases.

	Giera	Req	Г		
1	b, c,	l, d.	$t = c \cos b_i$	d	c sin b.
2	b, l,	c, d	c cos b	.2	$\ell \tan \theta$
3	b, d,	r, l	$r = \frac{d}{\sin b}$		$\frac{d}{\tan b}$
4	1 c, t,	b, d.	$\cos b \cdot \frac{t}{c}$,	rd	$1 e^2 - l^2$.
5	c, d,	b, t.	$\sin h = \frac{d}{c}$, .	l	1 02-02
6	$t_i d_i$	b, c.	$\tan b = \frac{d}{l}$,	c	$1 l^2 + d^2$

289. Examples.

PRELIMINARY | CALCULATIONS,

- 1. Given $b = N.53^{\circ} 20^{\circ} E$, and c = 26.50 ch; required Ans l = 15.82 ch. N., d = 21.26 ch. E. I and d.
- 2. Given b S. 75° 47' W., and t 22.04 ch. S.; required c and d. Ans. c 89.75 ch, d = 87 ch, W.
- 3 Given b = N, 35° W., and d = 1.55 ch W.; required Ans. c 2.70 ch , l 2.21 ch. N c and l.
- 4. Given c = 35.35 ch., and l = 31 ch. N.; required b and d.

Ans. b = N. 28° 44' E. or W., d = 16 99 ch. E. or W.

5. Given c = 81.30 ch., and d = 22.59 ch W.; required b and L.

Ans. b = N. or S. 47° W., and l 21 35 ch N or S.

6. Given l=7.02 ch. S., and d=7.14 ch W; required Ans. b = S. 45° 29' W., c 1001 ch. b and c.

290. Traverse Table.

The traverse table affords a ready method of finding the latar I and departure of a course whose distance and bearing are given.

Let all the I and d of a line whose h is N 35" 15' E., and c . 47,85 ch.

The first the traverse table, under 35 15 we this

· 10 gives l 32 67, d 23 (F)

c = 7 gives l = 5.72, d = 401

 $c \cdot .8 \text{ gives } l = .65, d = .66$

.05 gives (== 04, d 03

1. Signed 1 2008, d

4 1

The l and d for 40 are found from the l and d of 4, as given in the table, by multiplying by 10, or removing the decimal point one place to the right.

The l and d for the distance 7 are given in the table, but the right hand figure is dropped, and 1 is carried if the figure dropped exceeds 5.

The l and d for the distance .8 are found from the l and d for the distance 8 by removing the decimal point one place to the left, rejecting the figures at the right of the second decimal place, carrying as above.

For the distance .05, remove the decimal point two places to the left, reject and carry as before.

If the bearing exceeds 45°, the l and d will be found in columns marked at the bottom of the page.

291. Examples.

- 1. Given $b = N_0 28^\circ 45' E_0$ and c = -5.35 ch.; required l and $d_0 = Ans, l = -30.98$ ch. N., d = 17 ch. E.
- - 4. Given b S. 741° E., c 20.95 c required l and d.

 Ans. l 8.27 ch S 29.83 ch. E.
 - 5. Given $b = N_c 33 \stackrel{\circ}{\downarrow}^{\circ} W_{cr} c = 37 \text{ ch.}_{1/2} \text{ required } l \text{ and } d.$ $Au_{cr} = 30.94 \text{ ch.}_{1/2} N_{cr} d = 20.29 \text{ ch.} W_{cr}$

The work is written thus:

Vii	Bearings	Dist.	N Lat	S. Lat	F Dep	WIVE
1	N. 52° E	21/28	13.10		16.77	_
		8.18		7.11	4.06	
3	S. 317° W.	15.36		1406		84.8
4	N. 61° W	11.18	7.02			12.67

292. Balancing the Work.

It is evident that in passing around a feld to the point of beginning, we have gone just as far north as south, and just as far east as west. Hence the sum of the northings should be equal to the sum of the southings, and the sum of the eastings to the sum of the westings.

In practice, however, this is addom the case, on marto the fact that the bearings are taken only to quarter degrees, and that the chaining is not perfectly correct.

It is not a settled point among surveyers how great an error in latitude or departure can be allowed without resurveying the lot. Some would adnot an error of I link for every 10 chains in the sum of the courses; others, I link for every 3 chains. Each surveyer most settle this point for himself by ascertaining, by experience, how nearly he can make his work bulance

When an error is as likely to occur in one course as in another, the errors of latitude and departure are distributed among the courses in proportion to their length,

It will not, in general, be never-ary to make all the proportions, for after making one for lattale and one for departure, the remaining core to as can be a ade by a comparison of distance

Lat us take example 6 of the last article.

er Nil	Notes and	Dat	Mot	Stat.	£1⊱μ	9 Dep	CNL	rst.	CFD	C II D
2 3	N -37E 8 20P E 8 01 W N 61°W	8.18 15.36		7.11 13.06	4.06	8 08 12 67		7.10 13.05	16 74 4 05	8.11
•		59 30 Error Error	in L	at.	20.17).12			20.79,

SURVEYING

Corrections for Latitude.	Corrections for Departure.
59 30 : 21.28 - 05 : .02	5930 2128 , 08 : .03,
59.30 : 8.18 · 05 01.	59:30 \$15 (0.05), 01.
59:30 · 15:36 95 : .01.	59.30 [15.36] 05 : 02.
59 30 : 14 48 -: .05 : .01	59:30 11:48

The corrections are made to the nearest link or hundredth.

Since the north latitude is too small, and the south latitude too great, add to each north latitude the corresponding correction, and subtract from the south latitude. In a similar manner correct too laparture.

If one side is much more difficult to measure than the remaining sides, it is to be present that the error occurred chiefly in measuring that side and the corrections should be made accordingly.

If, in taking one bearing, the object build not be distinctly seen, the error probably or red in that bearing; then correct mainly in the 1 stude and departure of that course

In practice it will not be necessary to make additional columns for the corrected latitude and departures since they may be written in the same columns, over the others, with different colored ink

293. Examples,

1. Find the l and d, and balance the work from the following notes:

1st, N. 34\(\frac{1}{2}\) E., 8.19 ch.; 2d, N. 85\(\frac{1}{2}\) E., 3 84 ch.; 3d, S. 56\(\frac{1}{2}\) E., 6.60 ch.; 4th, S. 34\(\frac{1}{2}\) W., 1059 ch., 5tt. N. 56\(\frac{1}{2}\) W., 9.60 ch.

2. Find the l and d, and balance the work from the following notes:

1st, N. 5° E., 22.50 ch.; 2d, S. 83° E., 12.96 ch.; 3d, N. 50° E., 19.20 ch.; 4th, S. 32° E., 32.76 ch.; 5th, S. 41° W., 12.60 ch.; 6th, W., 16.86 ch.; 7th, N. 79° W., 21.84 ch.

3. Find the balanced I and d of the following:

1st N 30° E., 10 ch.; 2d, N. 60° E., 1848 ch.; 3d, S 10° E., 20.10 ch.; 4th, S. 30° W., 24 50 ch.; 5th, W., 15 ch.; 6th, N. 18‡° W., 19.92 ch.

294. Double Meridian Distance.

The double meridian distance of a course is double the distance of its middle point from a given meridian.

Let AB be a given course, NS the given meridian, P the middle point of AB, PQ perpendicular to NS.

Then will 2 QP be the double meridian distance of AB.

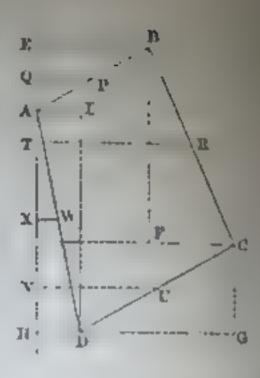
aume that the meridian of reference passes through the most westerly station, which we shall call the principal station, that departures of are pick and ters, minus, that the lines were run in the departures.

ABCD, so as to keep the field on the right.

The tollowing relations can be verified from the diagram:

- 1. 2QP EB.
- 2. 2TR = 2QP + EB + FC.
- 3. 2VU = 2TR + FC + (-GD).

4.
$$2XW = 2VU + (-GD) + (-IA) = AI$$
.



- 1. The double meridian distance of the first course is equal to its departure.
- 2. The double meridian distance of the second course is equal to the double meridian distance of the first course, plus the departure of the first course, plus the departure of the second course.
- 3. The double meridian distance of any course is equal to the double meridian distance of the preceding course, plus the departure of that course, plus the departure of the given course.
- 4. The double meridian distance of the last course is equal to its departure with its sign changed,

Take the example of a preceding article, as balanced.

Sta	Bearings	Dist. NLat.	SLat.	EDep B7	Dep. DMD.
1	N. 52° E	21 28 13.12		16.71	16.74
2		818			37.53
3	8317°W	1536	13.05	8	10 33.48
4	N. 61° W.	11.48 - 7.03		12	69 12.69

Dep. of 1st course = 16.74 - D.M.D. of 1st course. + dep. of 1st course = 16.74+ dep. of 2d course = 4.0537.53 = D.M.D. of 2d course.

The principal or most westerly station is not always the first station in the field notes

It will be observed that the word plus, in the above principles and illustrations, is used in the algebraic sense, that cost departure is considered plus and nest departure minus; that plus, an east departure, is a plus quantity, and plus a west departure a minus quantity; and that the double meridian distance of the last course is equal to its departure with its sign changed, which will serve as a verification of the work.

The first station of the notes, in the proceding example, is the most westerly, and was therefore taken for the principal station.

The most westerly station can readily be determined by inspecting the bearings of the courses as given in the field notes, and should be taken as the principal station, and the corresponding course as the test course in the course the double incredian distances.

295. Examples.

1. Given the following field b. b. a.

1st, N. 30° E., 10 ch.; 2d, N. 60° F., 1818 ch., 3d, 8, 40° E., 20 10 ch.; 4th, 8, 30° W., 24 50 ch., 5th, W., 15 ch.; 6th, N. 18° 45′ W., 19 92 cl.; Required the

latitude and departure; balance the work, and find the double meridian distances.

2. Given the following field notes:

1st, N. 45° W., 20 ch.; 2d, N. 18° E, 12.25 ch.; 3d, E., 12.80 ch; 4th, N. 32° E, 6.50 ch.; 5th, S. 423° E, 13.20 ch; 6th, S., 14.75 ch; 7th, S. 654° W., 16.30 ch. Required the corrected latitude and departure, and the double meridian distances.

AREA OF LAND.

296. Table of Linear Measure.

Mi. Ch. Rds.
 Yds.
 Plane
 Lks.
 In.

$$1 = 80 = 320 = 1760 = 5280 = 5280 = 8000 = 63300$$
.
 $= 66 = 100 = 792$.

 $1 = 4 = 22 = 66 = 100 = 792$.
 $= 16\frac{1}{2} = 25 = 198$.

 $1 = 3 = 4\frac{5}{11} = 36$.

 $1 = 3 = 4\frac{5}{11} = 12$.

 $1 = 71\frac{1}{11}$.

297. Table of Superficial Measure.

Mile.	Acres.	Roods,	Chains,	Perchos.	Links
1 ==	640 =	2560 ×	6400	102300	. 64000000.
	1 ==	4 ===	10	160	100000.
		1 ==	21	40	25000.
			1 ==	16	10000.
				1	625.

Note 1.—It should be remembered that in finding the area of a tract of land the inequalities of its surface are not considered, but the tract is treated as a horizontal plane.

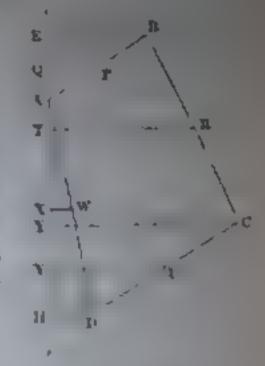
Note 2.—The area of a portion of land can, in a great variety of cases, be calculated by the rules almaly given for Mensuration of Plane Surfaces.

298. Problem.

To find the area of a truct of land when the length as I

direction of the bounding lines are given.

It is evident from the diagram that the area of ABCD is equal to the sum of the trapezoids EBCY and YCDH, minus the sum of the triangles AEB and ADH; and that twice the sum of the trapezoids, minus twice the sum of the triangles, is equal to twice ABCD.



The following table will exhibit the general form of operation:

It will be observed that we have taken the most westerly station for the principal station, and have multiplied the double meridian distance of each course by its latitude, and that the product is double the area of a triangle when the latitude is north, and double the area of a triangle when the latitude is north, and double the area of a trapezoid when the latitude is

If we had taken the most easterly station for the principal station, the reverse would be true.

In the above we have supposed that the lines were run in such direction as to keep the lot at the right.

If the lines were run in the opposite direction, so as to keep the lot at the left, the reverse would be true.

In any case, the sum of the double areas of the trapezoids, minus the sum of the double areas of the triangles, is equal to double the area required.

299. Rule.

Multiply the double meridian distance of each course by its latitude, placing the product in one column when the latitude is north, and in another column when the latitude is south, and divide the difference of the sums of the two columns by 2, and the quotient will be the area required

Take the example of a preceding article whose D. M. D.'s have been found

Sta. Bearings. Dist. NLat	SLat. EDry WDry DA	Triang. Trup
1 N.52°E. 21 28 13 12		4 219 6288
2 S29} E 8181	730 465	266 4630
3 S.314°W-15.36	L3 05	1,6,91400
4 N.61°W. 14.48 7.03	112 69 12	69 89.2107
Area == 19 A, 2 R,	36 P.	208,8395,703,3770
		3088343
Triangles 10.74 × 13.12 = 219.6288 12.69 + 7.03 = 89.2107	Traperads 7.55 7.10 266 5.48 13.05 - 4.66	19 226875
Divide double the by 10 to reduce the copy the decimal by and the next decima	aren by 2, the re- chains to acres, m 4 to reduce to r	000000 00 13 lues 10 10 10 10 10 10 10 10 10 10 10 10 10

300. Plotting.

Plotting is the process of representing, to a given scale, the length, direction, and relative position of the bounding lines of a tract of land

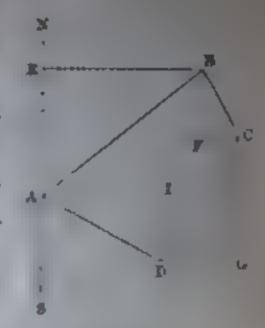
1st Method .- By means of latitudes and departures.

Take the example of the last article.

Let NS represent the meridian passing through the principal station A.

Select a scale whose unit shall represent 1 ch., and take AE 13.12 ch., the lat. of first course.

Through E draw a line perpendicular to NS; take EB = 16.74 ch., the dep. of first course, and draw AB.



Through B draw a meridian, and take EF = 7.10, the lat. of second course.

Through F draw a line perpend, ular to BF take FC = 4.05 ch., the dep. of second course, and draw BC.

Through C draw a meridian, and take (G 1805, the lat. of third course.

The 'G' draw a line perpendicular to CG, and take GD > 0 ch, the dep of third course and draw CD.

The half draw a merid in, and take DI = 7.03 ch. the half fourth course

Through I draw a line perpendicular to PI, take I4 12 60 cha, the dep. of fourth course, and draw P4

E = rk/1. — If the departure of fairth course term is notes at A, the work will be veries?

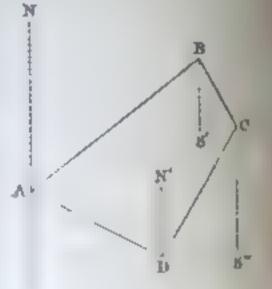
- 2. It will be observed that N. lat is laid off upward, S lat. downward, E. dep. to the right, and W. dep. to the left.
- 3. The auxiliary lines can be drawn with a pencil and afterward erased.
- 4. If every scale in possession of the surveyor should make the diagram too large or too small, all the latitudes and departures can be divided or multiplied by the same number, and the results taken instead of the given latitudes and departures.

2d Method. - By means of bearings and distances.

Take the same example.

Let NS represent the meridian passing through the principal station A.

With a protractor lay off the angle $NAB = 52^{\circ}$, the bearing of Artist course, and take AB = 21.28 ch., the first course.



Through B draw a meridian and lay off $S'BC = 293^{\circ}$, the bearing of second course, and take BC = 8.18 ch., the second course

Through C draw a maridian, and lay of \(CD = 31\) the bearing of third course, and take (D) = 15.46 ch., the third course.

Through D draw a meritan and χ off N'DA 61°, the bearing of feath course, and the DA 1448 ch, the fourth course, who havell term into at A if the work is correct

Remark 1.—The latitude and departure letters indicate the general direction of the lines, and the degrees the exact direction.

- 2. Let the examples of the following article be carefully plotted, and the area be found.
- 3. By a careful inspection of the bearings, the most westerly station can be found, which take for the principal station.
 - 4. The distances are all given in chains.

301. Examples.

	1.			2	
T _{N1}	Bearings.	Dut.	Na	Benresse	Inet
1	N. 30° E	10	1	N 47° E.	15 65
2	N. 60° E	1818	2	S 57° E	10.55
3	S. 40° E.	20.10	3	S 287 W	17.67
	~ 30° W	2150	4	2 Totale M.	1 11
()	W.)	15	- 5	8. 54° W	101
G	1 15/0 W	19.92	6	N 40½°W	15280
۱,	. So A. 1 R	25 P.	.6	e 23 A. 0 R.	38 P.

	3		4.	
r Sta	1 trenge	Dist	Sa Barras.	Ind
2 : 1 : 6	N. 45° W. N. 18° E. I F N. 123° E N. 424° E S. 654° W.	12.25 12.80 6.50 13.20 14.75	2 8 83 °E 3 8 83 °E 4 8 19 °E 5 8 64 °W	12 % 3 % 11 % 15 % 14 % 14 %
A	ns, 58 A. 3 R.	30 P.	A = 45 A 2 R	5 1

P.L	 OF	
	- C 1 A	

Sta.	Bearings.	Dist.
1	N. 20° E.	12.20
2	N. 70° E.	15.50
3	E.	18 25
4	S. 45° E.	20 00
5	S.	20.00
6	S. 45° W.	20.00
, "	W.	18 25
8	Z 303°W.	36,66

6.

Sta.	Bearings.	Dist.
		-
1	S. 34° E.	4 56
2	S. 66}°W.	13.84
3	N. 12 E.	12.15
4	N. 481°W.	12.30
5	N. 5 Y . E.	9 9 2
6	N 391 E.	5 22
7	S. 451°E.	18.63
8	S. 521°W.	1076

Ans. 32 A. 2 R. 26 P.

7.

Ans. 188 A. 3 R. 20 P.

-	Na	Bearings.	Dist.
	1	N. 30° E.	15.
,	2	N. 60° E.	15.
Ì	3	E.	15.
1	4	S. 60° E.	15.
1	5	S. 30° E.	15
	6	S.	15
	7	S. 30° W.	15
1	8	S. 60° W.	15,
-	9	W.	15.
	10	N. 60° W.	15.
ţ	11	N. 30° W.,	15
I ·	15	N.	15

Ans. 251.9 A. J

8.

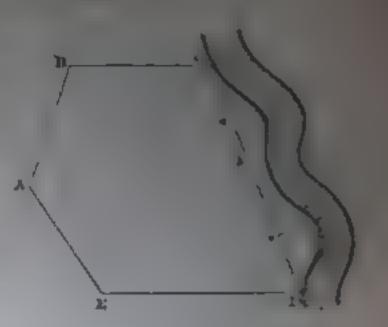
Sta	Ronnings	Dist.
1	S. 761°E.	6.69
2	S. 113°W.	5.96
3	S. 38° E.	9.82
1 4	N. 30½°E.	8.63
5.5	or aniolis !	943
ć,	- : H	15.70 (
1 7	S. Luz Way	13.06
8	N. 61° W.	11.93
9.1	S 700 W	10.45
10	1 22 11	11.60
11	N 37, T	14.37
13	N 22, T	10.79
	Ans. 76 11 A	

302. Problem.

To find the area when offsets are taken.

Find the area of the tract of land bounded by the full lines and middle of the river, as shown in the annexed diagram

Having run the stationary line CD, we have the following notes.



For ABCDE.

For Office

Sta.	Bearings.	Dist.	86	Dist	Office
1	N 20° E	15 50	1	0.00	2%
2	E	18:00	2	740	6.0
3	3 20° E	30.00	3	12.20	4.68
1	W	25 00	4	30.35	748
5	7 321°W	16 09	- 5	(Serve)	2 %

Area 70 A 1 R 33 P + 14 A 3 R 8 P S A 1 R 1 P

We do as in the last article, AECDE 70 A 1 R. 33 P.

To all the the area included between the state that's line CD and the line passing along the middle of the river, we find Ca = 7, $ab = C^{\dagger}$. On Ca = 7, $ab = C^{\dagger}$. On Ca = 7, Ca = 7. The parallel sides are given under the head of effects.

The altitude of a trapeze d nultiplied by the sum of the parallel sides will give two us are

The calculation is made as in the schooled table, the letters, $S_+ \otimes D_+ O_+ \cap ID_+ \otimes O_- \otimes O_- \cap ID_+ \otimes O_- \otimes$

columns of the table, denoting stations, station distances or distances from C, offsets, intercepted distances, sum of offsets, and double trapezoids.

8	S. D.	0.	I. D.	S. O.	D. T.
1	0.00	2.50			
2	7.00	6.00	7.00	850	59,5000
3	12 20	4 00	5.20	10 00	52,0000
4	22 25	7.00	10,05	11.00	110.5500
5	30.00	2.55	7.75	9 55	74.0125

Area, 14 A. 8 R. 8 P.

2) 296,0625

10) 148 03125

14.803125

3.212500

- 417

8.5000000

If the offsets fall within the stationary line, the sum of the trapezoids must be subtracted

In general, if the lines are run so as to keep the field on the right, the sum of the trapezoids must be added in case of left-hand offsets, and subtracted in case of right-hand offsets.

In case of navigable rivers, the bank is in general, the boundary—the first and last offsets become 0, and the first and last trapezoids become triangles, but the form of the computation is the same.

303. Examples.

I. Find the area of the lot of which the following are the field notes, and make a plot of the survey.

F	Pretilinear An	m,	L.H. C)fforts,*	R.H O	Turk **
Sta.	Bearings.	Dist.	St Dist	Offsets	St Dist	Offsetz
1 ,	N. 45° E	10 00	0.00	1.00	0.00	1 10
2	N.	10 00	6.50	4.25	5.62	4 (4)
3	N. 45° E.	10.00	1250	2.43	12 62	5.27
, 4 .	E.	10.00	17.50	5 17	17.07	1.13
5*	S.	31/21	26.21	5 53		
Chok	W.	17.07	31 21	1 25		
7	N. 45° W.	10.00				,

55,774715 A. + 12 17075 A 6 10160 A 61 A 3 R 15 P.

The left-hand offsets were made from the fifth course, as indicated by the single star, and the right hand offsets from the sixth course, as indicated by the double star.

2. Find the area of the lot of which the following are the field notes, and make a plot of the survey

R	etilinear Am	vi.	L.H. O.	Theta	R 11 Of	26.85
Sh.	Rearings.	Dot	St Dist	()442.	St Dist	f for war
ĭ	N. 30° E.	20.	000	0.00	0.00	0.00
2	E	20	600	3(0)	6.00	400
3*	S. 30° E.	20.	10.00	210	14 (0)	410
place	S. 30° W.	20	15.00	3.50	20.00	0.00
5	W.	20	20.00	0.00		,
ė,	N. 30° W.	20.				1

Ans. 102 A. 1 R. 36 P.

304. Pogue's Method of Finding the Area.

This method is illustrated by the following example:

Make a plot from the field notes, draw meridians through the most easterly and westerly stations, and parallels of latitude through the most northerly and southerly, thus enclosing the whole figure in a rectangle.

Find, from the traverse table, the latitudes and departures as in diagram.

To find xy, pass from the most westerly station, round the north, to the most easterly, taking the sum of the eastings minus the sum of the west and to find zw, pass from the most easterly state and the south, to the most westerly, taking the same the westings minus the sum of the eastings, thus:

$$xy = 8.38 + 15.27 + 23.96 - 10.66 + 22.75 = 59.70$$

 $xw = 9.03 + 30.21 + 20.11$ 59.68
 $\frac{1}{2}(xy + zw)$ the average 59.69

To find are, pass from the most southerly station, round the west, to the most northerly, taking the sum of the northings minus the sum of the southings; and to find yz, pass from the most northerly station, round the east, to the most southerly, taking the sum of the southings minus the sum of the northings, thus.

$$ux = 27.56 \pm 23.02 + 16.38$$
 67.26
 $yz = 8.72 \pm 12.70 \pm 10.60 \pm 10.03 \pm 14.20 \pm 10.99$ 67.24
 $\frac{1}{2}(ux + yz) = \text{the average altitude}$ 67.25
Area of rectangle -59.69×67.25 4014.15.25,

From the area of the rectangle we must dedu t the area included between ways and abedefyla, thus found.

abedefyhi = 4014.1525 eq. ch. -- 1633 9631 sq. ch 2380 1894 sq. ch -- 238 02 A

For additional exercises, work the examples of articles 301 and 303, and compare the answers obtained by the two methods

SUPPLYING OMISSIONS.

305. Case I.

When the bearing and length of one side are wanting.

The wanting side must be such that its latitude and departure will make the work balance. Hence, its latitude must be the difference between the sum of the northings and the sum of the southings of the given sides, and of the same name as the less; and its departure must be the difference between the sum of the eastings and the sum of the westings of the given sale, and of the same name as the less.

Having found the latitude and departure of the wanting side, construct a right-angle triangle by drauing on the paper, to represent the latitude, a line, up or down, according as the latitude is north or south; and at the terminus of the line, draw, to represent the departure, a horizontal line, to the right or left, according as the departure is east or west, and join the origin of the line representing the latitude with the terminus of the line representing the departure, and this last line will be the hypotenuse which will represent the course or length of the line sought, and the angle which it makes with the vertical line will be the bearing.

Denote the latitude by t, the depeture by d, the course by c, and the bearing by b, then we have,

(1)
$$c = V l^2 + d^2$$
. (2) $\tan b = \frac{d}{l}$.

Having found the bearing and distance, enter thom in the notes and find the area.

306. Examples.

Supply the omissions in the following field notes, calculate the areas, and plot the surveys.

1.		2.		
Sta. Bearings.	Dot.	71	Bereit It	Int
1 N. 18° E.	9.25	1	N 24° W	15.50
2 N. 71° E.	8.33	2	N 312 E.	17.07
3 S 43}°E.	12 37	3	E	20.
4 S. 361°W	16 00 1	4	Wanting	Want'g
5 Wanting, V	Vant'g	5	8 56 W	30(30)
(N. 43° W., 1		Anc	(S 12]° E , (56 A, 3 R	12 13 ch. 0 P

307. Case II.

When the lengths of two sides are was to g

Revolve the field so that one of the adia whose bearing only is given shall become a merchan, and find, by article 285, the bearings of all the adia at their new position.

The departure of the side made a nerstan will then be 0, and the difference of the same of the columns of the departures will be the departure, in the new position, of the other side whose distance is wanting.

Knowing the bearing and departure of this sal, we can find its distance and latitude. Then the dill rence between the sums of the columns of latitudes will be the length of the side node a nor dim

Revolve the field to its original perfect, calculations area, and make a plot of it or if the area only

is required after supplying omissions, it may be computed more readily without revolving the field to its original position.

308. Examples.

	1.			2			
St	Bearings.	Dist.	Sta.	Bearings.	Dist		
1	N. 30° E.	10 00	1	N. 47° E.	15 65		
2	N 60° E.	18.18	2	8 57° E	10 55		
3	S. 40° E	Want'g	3	S 28 /°W.			
. 4	8, 30° W.	Want'g	4	S 29]°W.	1.11		
, 5	. W.	15.00	5	8 51° W	104		
6	N. 181°W.	19.92		Z 40JoH.			
	(3d. 20 0S	ch.		(34 17 69	ch		
Ann.	4th. 24.52	ch.	Ano.	6th 16.01			
	(80 A. 1 R.	25 P.		(23 A. 1 R			

309, Case III.

When the bearings of two at . conting.

If the sides whose be traces a coverer care separated from each other by one or more, a consistency sides, suppose one of these sides and a sale of a to the other to change places, so as to bring a consideration together without change of the sides transposed.

Then, throwing these sales out a consideration, find, by Case I, the heraby and leads of the line joining the extremities of the sides wantings are wanting.

This line with the strong form a track whose sides are known, from who sides are known, from who sides are known, from who sides are constant.

Knowing the angles of the bearings of the other descan be read.

Restore to their original position the sides which have changed places, if such is the fact, calculate the area, and make a plot of the field

310. Examples,

1.				2.	
Sta.	Bearings	In-t	Sm	Bearings	Dist
1	N 45° W.	20.00	1	N 58° E	12.97
1 2	N. 18° E.	12.25	2	S 27]°E	3.30
3	E.	12.80	3	SSPE	11.65
4	N. 32° E	6.50	4	S 192 E	15.56
- 5	S 42}°E	13/20	5	Wanting	1403
6	Wanting	13.75	G	N 64° W	14.86
7	Wanting	16/30	7	Wanting	11.23
Ans.	600. S 70b. S 651	° W.	Aue	7th S 653 7th N 153 45 A 2 R.	
	59 A 3 R.	30 P.		C45 A 2 R.	o P.

311. Case IV.

13 here the bearing of one side and the langth of another ace

Revolve the field so that the sub-whose bearing only is given shall become a naridian

The departure of this side will then by 0, and the difference of the suns of the orbinus of departures will be the departure, in its new position, of the side will bearing is weeting

be grand hittel can be found

to the diff is a of the same of the exhaust of late to the will be the late of the who hade a north or

It volve the fild to its era, and position, computed the area and plot the work

It will be two solutions to the problem. If but one solution is admissible, the omission should be supplied by a remeasurement; and if the lost bearing or distance can not be taken directly, auxiliary lines may be run, and the omissions supplied by Trigonometry.

2. From the fact that two omissions can be supplied, the surveyor should not deem it unimportant to find all the measurements on the ground, since thus he can ascertain the correctness of his notes by balancing his work—a test not applicable when omissions are supplied.

312. Examples.

1. Sta. Bearings. S . Bearings. Thist, N. 20° E. 4.56 S 34° E. $12 \ 20$ N 70° E 15.50 5 6612W. 13 81 3 NICE 18 25 12 15 S. 45° E. 20.00 4 | Wenting 12 30 2011K) 5 N S/16 993 Wanting. 20.00 5 22 W Would Want'g. N 30\sh 16 66 8 8 32 W 10.76 6th, S 45° W Ana. 7th. 18.25. Ams. \ 7th. 18.65. 188 A. 3 R. 20 P. 132 A. 2 R. 26 P.

LAYING OUT LAND.

313. Laying out Squares.

To lay out a given quantity of land in the form of a square.

Let a be the area of the square, and z one side.

Reduce the given area to square chains, extract the square root, and the result will be the length of one sale.

With the chain and transit lay out the square on the ground,

EXAMPLES.

- 1. Lay out 12 A. 3 R. 20 P. in the form of a square
- 2. Find the side of a square containing 1 A., and lay out the square on the ground.

314. Laying out Rectangles.

1 To lay out a given quantity of land in the form of a retangle, one side of which is given.

Let a be the area of the rectangle, b the given side, and z an adjacent side

Then,
$$bx = a$$
, $x = \frac{a}{b}$.

2 To lay cut a given quantity of land in the form of a property in the property of the form of a property of the form of the f

I to denote the area of the rectangle, x its length, y its broadth, and more the ratio of x to y

Then,
$$\{x \mid y = n : n\}$$
 $\{x \mid y = n\}$

LAYING OUT LAND.

315

3. To lay out a given quantity of land in the form of a weets spic when the sum of its length and breadth is given.

Let a be the area of the rectangle, x the length, y the breadth, and * the sum of x and y.

Then,
$$\begin{cases} x + y + s \\ xy = a \end{cases} \rightarrow \begin{cases} x + \frac{1}{2}s + \frac{1}{2}V s^2 - 4a \\ y + \frac{1}{2}s - \frac{1}{2}V s^2 - 4a \end{cases}$$

4. To lay out a given quantity of land in the form of a rectangle when the difference of the length and breadth is given.

Let a denote the area of the rectangle, x its length, y its breadth, and d the difference of x and y.

Then,
$$\begin{cases} x - y = d, \\ xy = a. \end{cases}$$
 $\therefore \begin{cases} x = \frac{1}{2} \mid d^2 + 4a + \frac{1}{2}d, \\ y = \frac{1}{3} \sqrt{d^2 + 4a} - \frac{1}{2}d, \end{cases}$

315. Examples.

1. The area of a rectangle is 3 A., one side is 4 ch. Find an adjacent side and lay out the rectangle.

2. The area of a rectangle is 8 A.; the length is to the breadth as 3 is to 2. Find the sides and lay out the rectangle.

Ans. 10.95 ch. and 7.30 ch.

3, The area of a rectangle is 48 A.; the sum of the length and breadth is 14 ch. Find the sides and lay out the rectangle.

Ann. 8 ch. and 6 ch.

4. The area of a rectangle is 18 A.; the difference of the length and breadth is 3 ch. Find the sides and lay out the rectangle.

Ans. 15 ch. and 12 ch.

316. Laying out Parallelograms.

1. To lay out a given quantity of land in the form of a parallelogram when the hare is given.

Let a be the area, b the base, and z the altitude.

Then
$$bx = a$$
, $\therefore x = \frac{a}{b}$.

Measure the base, from any point of which erect a perpendicular equal to the calculated altitude.

Through the extremity of the perpendicular run a line parallel to the base, any point of which may be taken for one extremity of the upper base, which may then be measured off on this line.

2 When one side and an adjacent angle are quen.

Let a be the area, b the given side. I the given angle, and r the other side adjacent to this angle.

Then
$$bx \sin A = a_1 \dots x - \frac{a}{b \sin A}$$
.

3. When two adjacent sides are given.

Let a be the area, b and c the given sides, and r their included angle.

Then be
$$\sin x = a$$
, ... $\sin x = \frac{a}{b^a}$.

Remark.—If bc = a, then sin z = 1, $z = 90^{\circ}$, and the parallelogram becomes a rectangle.

If bc < a, the solution is impossible.

317. Examples.

- 1. The area of a parallelogram is 6 A, the base is 6 ch. Find the altitude and lay out the lund
- 2. The area of a parallelogram is 12 A, one sale is 12 ch., and an adjacent angle is (a). Find the other side adjacent to the given angle and by out the laid.

317

316

3. The area of a parallelogram is 8 Å, two adjacent sales are 8 ch. and 12 ch. Find their included angle and by out the land.

318. Laying out Triangles.

1 To lay out a given quantity of land in the form of a triangle when the base is given.

Let a denote the area, b the base, and x the altitude.

Then,
$$\frac{1}{2}lx = a$$
, $\therefore x = \frac{2a}{b}$.

Measure the base, at any point of which erect a per-

Through the extremity of this perpendicular draw a line parallel to the base. This parallel will be the locus of the vertex, any point of which may be taken for the vertex.

2. When the base is to the altitude in a given ratio.

Then,
$$\begin{cases} \frac{1}{2}xy = a. \\ x : y :: m : n. \end{cases}$$
 $\begin{cases} \frac{1}{2}nm \\ \frac{1}{2}nm \end{cases}$

3. When the triungle is equilateral

Let a denote the area and z one side

Then,
$$.4330127 \ x^2 \rightarrow a$$
, $... x = \sqrt{\frac{a}{.4330127}}$

4. When one sale it of odpoint angle , quen.

Let a denote the sice / the given sub- z the adja-

Then,
$$\frac{1}{2}bx \sin A = a$$
, $\therefore x = \frac{2a}{b \sin A}$

5. When two sides are given.

Let a denote the area, b and c the given sides, and z their included angle.

Then,
$$\frac{1}{2}bc\sin x = a$$
, $\therefore \sin x = \frac{2a}{bc}$.

319. Examples.

- 1. The area of a triangle is 3 A, the base is 5 ch. Find the altitude and lay out the triangle on the ground.
- 2. The area of a triangle is 12 A, the base is to the altitude as 3 is to 2. Find the base and altitude and lay out the triangle on the ground.
- 3. The area of an equilateral triangle is 1 A Find a side and lay out the triangle.
- 4. The area of a triangle is 1.2 A., one side is 2 ch., an adjacent angle is 45°... Find the other side adjacent to the given angle and lay out the land.
- 5. The area of a triangle is 2 A., two sides are 6 ch. and 10 ch. Find the included angle and lay out the triangle.

320. Laying out Circles or Regular Polygons.

1 Let a be the area of the circle, and x the radius.

Then, 3.1416
$$z^3 = a$$
, $z = \sqrt{\frac{a}{3.1416}}$

2. Let a be the area of a regular polygon, x one sale, a one angle, n the number of sides, and a the area of a similar polygon whose side is 1. Article 167

Then,
$$a'x^2 = a$$
, ... $z = \sqrt{\frac{a}{a}} \cdot y = \frac{180^{\circ} (n-2)}{n}$.

DIVIDING LAND.

319

321. Examples.

- 1. Find the radius of a circle whose area is 1 A. and lay out the circle.
- 2. Find the sides and angles of a regular hexagon containing 1 A. and lay out the hexagon.
- 3. Find the sides and angles of a regular octagon containing 1 A, and lay out the octagon.

DIVIDING LAND.

322. Division of Rectangles or Parallelograms.

1. To cut off a given area by a line parallel to a given side.

Let a be the area, b the given side, x the distance to be cut off on the sides adjacent to b, and A the acute angle of the parallelogram

For the rectangle, $bx = a_1 + x = \frac{a}{b}$.

For the parallelogram, $bx \sin A = a$, $\therefore x = \frac{a}{b \sin A}$

2. When the lot is to be divided into parts having a given ratio, by lines parallel to two of the sides, divide the other sides into parts having the same ratio

323. Examples.

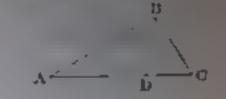
- 1. The sides of a rectangle are 15 ch, and 10 ch.; out off 8 A. by a line parallel to the shorter sides.
- 2. The adjacent sides of a parallelogram are 12 ch, and 20 ch., and their included angle is 65°; cut off 10 A. by a line parallel to the shorter sides
- 3. A man willed that his farm, which was I mile long and ½ mile wide, be divided among his three

sons, A, B, and C, aged 21 yrs., 18 yrs., 15 yrs., respectively, in proportion to their ages, by lines parallel to the shorter sides. Make the divisions.

324. Division of Triangles.

1. To find a point on a given side of a triangle from which a line drawn to the vertex of the opposite angle will divide the triangle into parts having a given ratio.

Let b = AC, the given side; D, the required point; x = AD, and ABD; DBC; m; n.



By composition we have,

ABC : ABD :: m + n : m; but ABC : ABD :: b : x.Hence, $m + n : m :: b : x, \dots x = \frac{bm}{m + n}$.

2. Two vides of a triangle being given, to divide the triangle into parts having a given ratio by a line parallel to the third side.

Let a = BC, b = AC, the given sides; x = CE, y = CD, and DEC : ABED :: m : m.

By composition we have,

ABC: DEC:: m+n:m; but $ABC: DEC:: a^2: x^2 = b^2 \cdot y^3.$

Hence, $\begin{cases} m+n : m :: a^2 : x^2, \\ m+n : m = b^2 : y^2, \end{cases}$

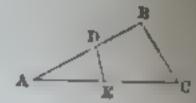
If for example, the triangle is to be divided into three equal parts by lines parallel to the third side, then,

The distances cut off on a are $a + \frac{1}{3}$, $a + \frac{1}{3}$.

The distances cut off on b are $b + \frac{1}{3}$, $b + \frac{1}{3}$.

3. Two sides of a triangle being given, to cut off, by a line interesting the given sides, an isomeles triangle having a given out to the given triangle.

Let
$$b = AC$$
, $c = AB$, the two given sides; $x = AE = AD$, and



ADE:ABC::m:n.

But, $ADE: ABC:: \pi^2:bc$.

Hence,
$$m:n :: x^2:bc, \dots, x^n : \sqrt{\frac{hcm}{n}}$$
.

4. Two sides of a triangle being given, to cut off a triangle having a given ratio to the given triangle by a line running from a given point in one of the given sides to the other given side.

Let b = AC, c = AB, the given sides; D, the given point; d = AD, $z \in AE$, and AED : ABC :: m : n



But, AED . ABC :: dx 1.

Hence,
$$m:n$$
 d_f , r $\frac{bcm}{dn}$.

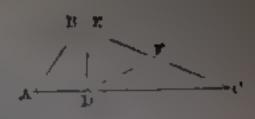
5 The three sides being given, to decrease the triangle into their equal parts by lines running trial a given point in one of the sides.

Let a, b, c be the sides of the triangle, respectively, opposite the angles if, $B, C, j \in AD$, q = CD, x = AE, and $y \in CF$.



Then,
$$\left\{ \begin{array}{l} 3:1::bc:px \\ 3:1::ab:qy \end{array} \right\} \rightarrow \left\{ \begin{array}{l} x - \frac{bc}{3\tilde{p}}, \\ y - \frac{ab}{3\tilde{q}}, \end{array} \right.$$

If z, thus found, is greater than c, both lines will intersect a. Then find y as above.



Let x CE.

Then,
$$3:2::ab:qx$$
, $x:x=\frac{2ab}{3q}$.

If y, found above, is greater than a, both lines will intersect r. Then find x as in first case.



Let AF = y.

Then,
$$3:2::bc:py$$
, . . $y = \frac{2hc}{3p}$

6. To divide a triangle into four equal triangles, join the middle-points of the sides.

The lines ED, EF, and DF are, respectively, parallel to BC, AC, \triangle and AB.

EBF EDF, since each is \frac{1}{2} the parallelogram BD.

ADE EDF, since each is & the parallelogram AF.

CDF EDF, since each is \frac{1}{2} the parallelogram CE.

Hence, the triangles are all equal, and cach is \ .1 EC

7. The hearing of two sides being given, to cut off a traangle having a given area by a line of a given bearing intersecting the sides whose bearings are given.

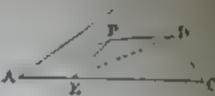
Let ADE be the triangle cut off, a the area of ADE; z = AD and y + AE. The angles A, D, E can be determined from the bearings.



Then,
$$\begin{cases} \frac{1}{2}xy\sin A = a, \\ \sin E \cdot \sin D \cdot x = y \end{cases} \rightarrow \begin{cases} \frac{x}{x} & \frac{2a\sin E}{\sin A \sin D}, \\ y & \frac{2a\sin D}{\sin A \sin E}. \end{cases}$$

R. To decide a triangle into two equal parts by lines

Let ABC be the given triangle, and P the given point.



Run a line from P to the vertex

A. and another from P to D, the middle point of the opposite side BC. Run DE parallel to PA, and run PE. PD and PE will be the dividing lines, and CDPE will be \(\frac{1}{2}\) ABC.

For, draw the line AD, then we have,

$$CDPE = CDE + PED$$
, and $ACD = CDE + AED$.
But $PED = AED$, ... $CDPE = ACD$.
But $ACD = \frac{1}{2}ACB$, ... $CDPE = \frac{1}{2}ACB$.

9. Through a given point, within a given triangle, to draw a line which shall cut off a triangle having a given ratio to the given triangle.

Let ABC be the given triangle; a, b, c, the sides opposite the angles A, B, C, respectively; D the point given by knowing p = AF = ED, parallel to AC; q = AH, q = AG, and AGH : ABC :: m : n. Then,

$$\left\{ \begin{array}{l} x:y \cdots x - p:q, \\ xy, bc \cdots m:n, \end{array} \right\} \cdots \begin{cases} x = \frac{bcm + 1}{2} \frac{b^2 c^2 m^2 - 4 \ bcm npq}{2 \ nq}, \\ y = \frac{2 \ bcm + 1}{bcm + 1} \frac{bcm npq}{bcm + 1} \right\}$$

Remark — If either x = h or $y > c_i$ the 1 are cuts off the triangle from another angle; and the distances cut off from the vertex of this angle can be found in a manner similar to the x^{k-1} 10. To find a point within a triangle from which the lines drawn to the vertices will divide the triangles into three equal triangles.

Let ABC be the triangle. Take $AD = \frac{1}{3}AB$, $CE = \frac{1}{3}CB$, and draw DE. Take $BF = \frac{1}{3}BA$, $CG = \frac{1}{3}CA$, and draw FG.

P, the intersection of these lines, will be the point required.

For AD:AB: altitude of APC: altitude ABC: But $AD=\frac{1}{3}AB, ...$ altitude $APC:=\frac{1}{3}$ altitude ABC:

 $APC \rightarrow \frac{1}{3} ABC$

In like manner, $BPC = \frac{1}{3} ABC$.

$$APB = \frac{1}{2} ABC$$

Remark.—If APC, BPC, and APB are to be to each other as p, q, τ , take $AD = \frac{p}{p+q+r}$ of AB, CE $\frac{p}{p+q+r}$ of CB, $BF = \frac{q}{p+q+r}$ of BA, $CG = \frac{q}{p+q+r}$ of CA, and draw DE and FG, their intersection will be the point required.

325. Examples.

1. One side of a triangle is 15 ch.; from what point in this side must a line be drawn to the vertex of the opposite angle so as to divide the triangle into two triangles which are to each other as 2 to 3?

Ans. 6 ch. from one extremity

2. Two sides of a triangle are 10 ch and 15 ch, respectively; find the distance from the vertex of the

angle included by these sides, cut off on each of these sides by a line parallel to the third side, dividing the triangle into a triangle and a trapezoid, so that the triangle cut off shall be to the trapezoid as 9 to 16.

Ann. 6 ch. and 9 ch.

3. Two sides of a triangle are 4 ch. and 9 ch., respectively; find the distance from the vertex cut off on each of these sides by a line cutting off an isoccles triangle which shall be to the given triangle as 18 to 25.

Ans. 480 ch.

4. Two sides of a triangle are 7 ch. and 9 ch, respectively. From a point in one side, 5 ch. from the vertex of the angle included by these sides, a line is run to the other given side, cutting off a triangle which is to the given triangle as 5 to 9. How far from the same vertex does this line intersect that side?

Ans. 7 ch.

5. The sides of a triangle, ABC, are a=6 ch, b=12 ch, and c=9 ch. From the middle point of b two lines are run, dividing the triangle into three equal parts. To what points of what sides must the lines be run?

Ans. To e, 6 ch. from A, and to a, 4 ch. from C.

6. The sides of a triangle, ABC, are a=10 ch., b=12 ch., and c=4 ch. From a point in b, 3 ch. from A, two lines are run, dividing the track a into three equal parts. To what points of what side must these lines be run?

Ans. To a, 8.89 ch. from C, and to a, 4.44 ch. from C.

7. The sides of a triangle, ABC, are n 5 ch, b = 18 ch, and c = 15 ch. From a point in b, 12 ch. from A, two lines are run, dividing the triangle into three equal parts. To what points must these lines be run?

Ans. To c. 750 ch. from A, and to B.

8. In the triangle ABC, the side AB runs N. 50° E, AC runs E. DE, running N. 10° W, intersects these lines in D and E, and cuts off ADE 10 A. Required AD and AE. And AD 1654, AE 1881.

9. In the 9th general problem of the last article, b=10 ch, c=12 ch, m=1, n=4, p=2 ch, q=3 ch. Find x and y. Ans x=7.24 ch, y=4.14 ch.

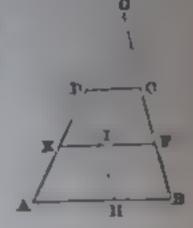
326. Division of Trapezoids.

1. Given the bases and a third side of a trapezoid, to divide it into parts having a giren ratio by a line parallel to the bases.

Let ABCD be the trapezoid, b = AB, b' = CD, a = AD, x = AE, y = EF, the dividing line, parallel to the bases, and ABFE : EFCD :: m : n.

Produce AD and BC to G.

Then,
$$\begin{cases} ABG : DCG :: b^2 : b'^2, \\ EFG : DCG :: y^2 : b'^2. \end{cases}$$



These proportions taken by division give,

$$ABCD : DCG :: b^2 - b'^2 : b'^2,$$

 $EFCD : DCG :: y^2 - b'^2 : b'^2,$

Since the consequents are the same, we have,

$$ABCD : EFCD :: b^3 - b'^2 : y^2 - b^2.$$

This proportion taken by division gives,

$$ABFE: EFCD :: b^2 - y^1 : y^2 - b'^2,$$
 But $ABFE: FFCD :: m : n.$

$$\cdot, b^2 - y^2 \cdot y^2 - b'^2 :: m : n, \quad \cdot, \quad y - \sqrt{\frac{b^2n}{m + n}}.$$

Drawing DII parallel to BC, we have,

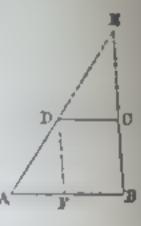
or
$$b \rightarrow b': y \rightarrow b':: s: s \rightarrow x$$
, $x \rightarrow \frac{s}{b-b'}(b-y)$.

...
$$x = \frac{8}{b} (b - \sqrt{\frac{b^2 n + b'^2 m}{m + n}})$$

2. Given a side and two adjacent angles of a tract of land, to cut off a trapezoid of a given area by a line parallel to the given side.

1st. When the sum of the two angles < 180°.

Let a = ABCD = the area cut off, b = AB the given side, x = AD, y = BC, z = DC, $E = 180^{\circ} - (A + B)$.



(1) Area $ABE = \frac{1}{2} EB \times EA \sin E$.

$$\sin E : \sin A :: b : EB, \dots EB = \frac{b \sin A}{\sin E}$$

$$\sin E : \sin B :: b : EA$$
, $\therefore EA = \frac{b \sin B}{\sin E}$.

Substituting the values of EB and LA in (1), we have,

(2)
$$ABE = \frac{b^2 \sin A \sin B}{2 \sin E}$$

... (3)
$$DCE = \frac{h^2 \sin A \sin B}{2 \sin E} = a$$
.

But ABE: DCE:: b2::21.

$$\frac{b^2 \sin A \sin B}{2 \sin E} : \frac{b^2 \sin A \sin B}{2 \sin E} \rightarrow a :: b^2 : s^3.$$

$$b^2 = \frac{2 a \sin E}{\sin A \sin B}$$

Draw DF parallel to EB, then ADF = E and DFA - B.

$$\sin E:\sin B::b-z:z,\ \cdot\cdot,\ z=\frac{b-z\cdot\sin B}{\sin E}.$$

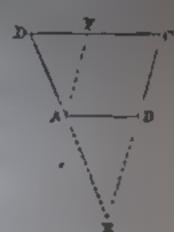
In like manner we shall find $y = \frac{(h - z \sin A)}{\sin E}$.

Since z is known, z and y are known.

2d. When the sum of the two angles > 180°.

E and DC lie on opposite sides of AB.

Let a = ABCD = the area to be cut off, b = AB the given side, x = AD, y = BC, z = DC, $E = A + B + 180^{\circ}$.



By a process similar to that em-

$$s = \sqrt{b^2 + \frac{2 a \sin E}{\sin a \sin B}}.$$

$$s = \frac{(s - b) \sin B}{\sin E}.$$

$$y = \frac{(s - b) \sin A}{\sin E}.$$

3. To dirule a improvid into proportional parts by a line joining the bases.

Let ABCD be the trapezoid, be and b' the bases, a the altitude, m and n the ratio of the parts.



Take
$$AE = \frac{mb}{m+1}$$
, then $EB = \frac{mb}{m+n}$,

nlso
$$DF = \frac{mh'}{m+n}$$
, then $FV = \frac{nh'}{m+n}$.

Then,
$$AEFD = \frac{am(b+b')}{2(m+n)}$$
, and $EBCF = \frac{an(b+b')}{2(m+n)}$.

But $\frac{am(b+b')}{2(m+n)} : \frac{an(b+b')}{2(m+n)} :: m: n$.

AEFD: $EBCF :: m: n$.

Remark. — If the line is to be drawn from a given point P, in one base, first divide as above; then, if P is on one side of E, take P' as far on the other side of F, and draw PP'.

This change in the dividing line does not affect the altitude of the parts, nor the sum of their bases, since one is increased as much as the other is diminished, nor, consequently, their area.

A similar process can be employed whatever be the number of parts.

327. Examples.

- 1. A trapezoid whose bases are b=15 ch. and b' 12 ch., and third side s=10 ch., is divided by a line parallel to the bases into two parts, such that the part adjacent to b is to the part adjacent to b' as 3 to 2 Required the length of the dividing line, and the distance from b cut off on s. Ans. 13.28 ch., and 5.73 ch.
- 2. Given a side 14.30 ch., and the two adjacent angles, 60° and 70°, respectively, of a tract of land from which 10 A, are to be cut off by a line parallel to the given side. Required the length of the dividing line, and the respective distances from the given—ide cut off on the adjacent sides.

Ans. 403 ch., 1260 ch., and 11.61 ch.

3. Given a side 10 che and the two advect angles, 120° and 115°, respectively of a tract of land, from which 15 A are to be cot of by a line parallel to the

given side. Required the length of the dividing line, and the respective distances from the given side cut off on the adjacent sides.

Ans. 20.32 ch., 11 42 ch., 10.91 ch.

4. A trapezoid whose parallel sides are AB = 14 ch, and DC = 7 ch, is divided by the line PP' into two parts which are to each other as 3 to 4; AP = 4 ch, find DP'.

328. Division of Trapeziums.

1. Given a side, two adjacent angles, and the area of a trapezium, to divide it, by a line parallel to the given side, into parts having a given ratio.

Let ABCD be the trapezium; b = AB, the given side; A and B, the given angles; $G = 180^{\circ} - (A + B)$, a =the area of ABCD, x = AE, y = BF, and $ABFE : EFCD :: m \cdot n$.

$$ABFE = \frac{ma}{m+n}, EFCD = \frac{na}{m+n},$$

$$ABG = \frac{1}{2}BG \times AG + \sin G.$$

$$BG = \frac{b \sin A}{\sin G} \text{ and } AG = \frac{b \sin B}{\sin G}.$$

$$ABG = \frac{b^2 \sin A \sin B}{2 \sin G}.$$

$$ABG = \frac{b^2 \sin A \sin B}{2 \sin G} = \frac{ma}{m+n}.$$

 $ABG + EFG + AG^2 + EG^3$, $ABG + FFG + BG^2 - FG^2$.

Substituting, in the proportions, the values of ABG, LFG, AG and BG, find EG and FG, and substituting the values of AG, EG, BG and FG in the equations,

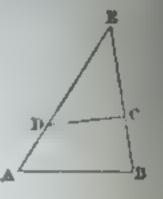
z AG -- FG and y BG FG, we have, 8. N. 28.

$$s = \frac{b \sin B}{\sin G} = \frac{b^2 \sin^2 B}{\sin^2 G} = \frac{2 \max \sin B}{(m+n) \sin A \sin G}$$

$$y = \frac{b \sin A}{\sin G} = \frac{h^2 \sin^2 A}{\sin^2 G} = \frac{2 \max \sin A}{(m+n) \sin B \sin G}$$

2. Given the bearings of three adjacent sides of a tract of land, and the length of the middle side, to cut off, by a line running a given course, a trapezium of a given area.

Let a = ABCD, the area cut off; b = AB, the given side; x = AD, y = BC, z = CD.



From the given bearings, find the angles A, B, C, D, E.

$$BE = \frac{b \sin A}{\sin E}$$
 and $AE = \frac{b \sin B}{\sin E}$.

$$ABE = \frac{1}{3}BE \times AE \times \sin E = \frac{b^2 \sin A \sin B}{2 \sin E}.$$

$$DCE = \frac{b^2 \sin A \sin B}{2 \sin E} - a.$$

$$DE = \frac{z \sin C}{\sin E}$$
 and $CE = \frac{z \sin D}{\sin E}$.

$$DCE = \frac{1}{2}DE \times CE \times \sin E = \frac{z^2 - \ln C \sin D}{2 \sin E}$$

$$\frac{z^2 \sin C \sin D}{2 \sin E} = \frac{b^2 \sin A \sin B}{2 \sin E} = a.$$

$$\therefore z = \sqrt{\frac{b^2 \sin (1 \sin B)}{\sin (c \sin D)}} = \frac{2a \sin P}{\sin (c \sin D)}.$$

Substituting the value of z in the value of DE and CE, then the values of AE, DE, BE and CE in the equations,

$$x = AE - DE$$
, and $y = BE - CE$, we find,

$$x = \frac{b \sin B}{\sin E} = \sqrt{\frac{b^2 \sin A \sin B \sin C}{\sin E \sin D}} = \frac{2 a \sin C}{\sin B \sin E}$$

$$y = \frac{b \sin A}{\sin E} - \sqrt{\frac{b^2 \sin A \sin B \sin D}{\sin^2 E \sin C}} - \frac{2 n \sin D}{\sin C \sin E}$$

Remark.—If $A+B > 180^{\circ}$, the values of x and y are

$$x = \sqrt{\frac{b^2 \sin A \sin B \sin C}{\sin B} + \frac{2 a \sin C}{\sin B \sin E}} = \frac{b \sin B}{\sin E}$$

$$y = \sqrt{\frac{b^2 \sin A \sin B \sin D}{\sin^2 E \sin C}} + \frac{2 a \sin D}{\sin C \sin E} = \frac{b \sin A}{\sin E}.$$

3. The hearings of several adjacent sides of a tract of land being given, and the length of each, except the first and last, to cut off a given area by a line of given bours of intersecting the first and last sides.

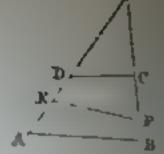
Let the bearings and distances of AK, KL, LM, MN, NB be given, and the bearings of AD and BC; and let to be the area cut off by CD.

Draw AB; then, in the polygon, ABNMLK, the bearings and distances of all the sides are known,

except AB, which can be computed, and the area of ABNMLK found. Subtract the area thus found from the area to be cut off by CD, and the remainder will be the area of ABCD.

Then, by the last case, find AD and BC.

4 The hearings of the sales of any quadrilateral tract of hand and the distances of two of peats sales being given, to divide it into parts having a given ratio by a line of a given ratio intersection the other sales.



 $BG = \frac{b \sin A}{\sin G}, \quad AG = \frac{b \sin B}{\sin G}, \quad DG = \frac{c \sin C}{\sin G},$

 $CG = \frac{e \sin D}{\sin G}$, $FG = \frac{e \sin E}{\sin G}$, $EG = \frac{e \sin F}{\sin G}$.

 $ABFE = \frac{m(h^2 \sin A \sin B - v^2 \sin C \sin D)}{2(m+n) \sin G}$

 $ABFE = \frac{b^2 \sin A \sin B - z^2 \sin E \sin F}{2 \sin G}.$

Equating these values of ABFE, we find,

$$z = \sqrt{\frac{ab^2 \sin A \sin B + ac^2 \sin C \sin D}{(m+n) \sin E \sin F}}.$$

Substituting this value of z in the values of FG and EG, then the values of AG, EG, BG and FG in

x AG EG, and y RG Fee, w have,

 $\frac{b\sin B}{\sin G} = \frac{\sin F}{\sin G} \sqrt{\frac{ab^2 \sin (1 + a)E}{B}} = \frac{i \sin F}{\sin F}$

 $\frac{b \sin A}{\sin G} = \frac{\sin E}{\sin A} + \frac{ab^2 \sin A \sin A}{\sin a + a + a + b} + \frac{\sin C \sin D}{\sin B}$

5. The bearings and distances of the sides of any quadrelateral tract of land being given, to do to it into parts having a given rates by a line dividing to opposite sides proportionally.

$$b = AB, c = CD, d = AD,$$

$$c = BC, x = AE, y = BE,$$

$$ABFE : EF(D) :: m : n,$$

$$x : d - x : y : c - x, ..., y = \frac{cr}{d}.$$

From the bearings find the angles A, B, C, D, G.

 $BG = \frac{b \sin A}{\sin G} = p, \text{ and } AG = \frac{b \sin B}{\sin G} = q$ $ABFE = \frac{m \cdot b^2 \sin A \sin B}{2 \cdot (m + n) \sin G} = \frac{m \cdot b^2 \sin A \sin B}{2 \cdot (m + n) \sin G} = \frac{e^2 \sin C \sin D}{2 \cdot (m + n) \sin G}$ $EFG = \frac{b^2 \sin A \sin B}{2 \cdot (m + n) \sin G} = \frac{e^2 \sin C \sin D}{2 \cdot (m + n) \sin G}$ $But EFG = \frac{1}{2} (q - x) (p - \frac{ex}{d}) \sin G$ $\therefore \quad \frac{1}{2} (q - x) (p - \frac{ex}{d}) \sin G$ $\therefore \quad x = \frac{dp + q + \sqrt{dp - q}}{2e} = \frac{8 dee}{\sin G}$ $\therefore \quad y = \frac{dp + q \pm \sqrt{(dp - q)^2 + \frac{8 dee}{\sin G}}}{2e}$

6. The bearings and distances of the sides of a quadrulateral being given, to cut off a given area by a line running torough a point whose bearing and distance from the vertex of one of the angles are given.

Let a be the area of ABFE, cut off by EF through P.

> $b = AB, \quad c = CD, \quad u = EG,$ $v = FG, \quad x = AE, \quad y = BF.$

The bearings give the angles A, B, C, D, PCQ, PCD.

$$BG = \frac{b \sin A}{\sin G}, \quad AG = \frac{b \sin B}{\sin G}, \quad ABG = \frac{b^2 \sin A \sin B}{2 \sin G}.$$

$$EFG = \frac{b^2 \sin A \sin B}{2 \sin G} \quad a \quad a'$$

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In the triangle DCP we have given CD, CP, and PCP, times CPP and PP can be found; then PDR CPR CPP

PE DP sin PDR p, and PQ CP sin PCQ q

EPG $\frac{1}{2}$ pu, and FPG $\frac{1}{2}$ qv.

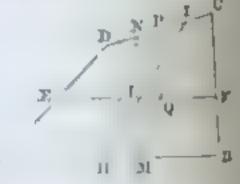
$$\begin{cases} u = \frac{a'}{p} = \sqrt{\frac{a'}{p}^2 - \frac{2aq}{p\sin q}} \\ \text{But } \frac{1}{q} uv \sin G = a', \end{cases}$$

$$\frac{\int_{a}^{b} \frac{b \sin B}{\sin G}}{\sin G} = \frac{a'}{q} + \sqrt{\frac{a'^{2}}{p^{2}}} = \frac{2\overline{a'q}}{p \sin G}$$

$$\frac{b \sin A}{\sin G} = \frac{a'}{q} + \sqrt{\frac{a'^{2}}{q^{2}}} = \frac{2\overline{a'p}}{q \sin G}$$

7. The bearings and distances of the sides of a quadrilateral being given, to divide it into four equal parts by two lines intersecting the pairs of opposite sales, respectively, one line being parallel to one side

Let EF, parallel to AB, and MN, parallel to BC, each divide ABCD into two equal parts; and PQ, parallel to FC, divide EFCD into two equal parts.



Find AE, BF, BM, CN, CP, and FQ, by problem 1 of this article.

$$EF = AB - AE \cos A - BF \cos B$$
.

Likewise find MN and PQ. NP = CN - CP.

Produce MQ to I, draw NH parallel to IM, and draw HI; then will EF and HII be the lines required.

The line EF is endertly one of the required line.

We are now to prove that HI is the other.

The two triangles, HNI and HNM, are equal, since they have a common base, HN, and a common altitude, their vertices being in LM, parallel to the base.

To each of these equal triangles add AHND, and we have AHID - AMND - § ABCD.

We are now to prove that III divides EFCD, and also ABFE into two equal parts.

 $IMH: IQL:: \overline{IM}^2: IQ^2,$ $IMN: IQP:: \overline{IM}^2: IQ^2,$ IMH: IQL:: IMN: IQP, But $IMH: IMN, \dots, IQL: IQP,$

To each add QFCI, and we shall have,

$$LFCI = QFCP = \frac{1}{2} EFCD.$$

Again, HBCI = AHID and LFCI = ELID.

Subtracting the second from the first, member from member, we have,

$$HBFL = AHLE$$
.

Hence, HI is the other division line required

Let us now find the situation of the points H and I, on the lines AB and CD, respectively.

$$NM: PQ:: NP+PI: PI.$$
 $\therefore NM \times PI = PQ \times NP + PQ \times PI.$
 $(NM \rightarrow PQ) PI = PQ \times NP.$
 $PI = \frac{PQ \times NP}{NM \rightarrow PQ}.$ Then, $CI = CP \rightarrow PI.$

The bearing and length of IM, and the area of ICBM, can be found by Art. 305. IMH = ICBH - ICBM

If p be the perpendicular from I to AB, $p = IM \sin IMB, \quad MH = 2\frac{IMH}{p}, \quad BH = BM + MH$

329. Examples.

I A trapezum, one side of which is 20 ch., the adjust angles to and 80°, respectively, and the area to A, is divided into two equal parts by a line parally to the given side. Required the distance from the zven side cut off on the adjacent sides.

Ans. 3.04 ch., and 2.68 ch.

- 2. From a tract of land, the bearings of three of whose adjacent sides are S. 20° W., E. and N. 10° W., and the distance of the middle side is 10 ch., 5 A. are cut off by a line running S. 70° W, and intersecting the first and third of the above mentioned sides. Required the distances cut off on these sides from the anddle side.

 Ans. 4.91 ch., and 7.29 ch.
- 3. From a tract of land, the bearings of whose sides are S 38° E, S. 29]° E, S 31]° W. N 61° W., and N 40° W., respectively, and the district of the second, third, and fourth sides are S 18 ch., 15 % ch., and 14 48 ch., respectively, 39 A 2 R 36 P. are cut off by a line running N, 80° E, and intersect. It is first and last sides. Required the distances out of an these sides respectively.

 38 38° E, S. 29]° E, and intersect. It is first and last sides. Required the distances out of an these sides respectively.
- 4. A tract of land, the bearing and distances of whose sides are AB, E 22.21 ch., BC, N · cD N 565° W, 12 ch.; DA, S. 24° W., is cut by EF running S 765° E, intersecting AD and BC and dividing the fold so that ABFE: EFCD = 5 · C Required AE, and BF.

 Are AL = 16.50 ch., BF 41.34 ch.
- 5. A trapezium whose are AB = 20.15 ch. BC = 21.73 ch. CD = 15.68 ch., DA = 15.32 ch., and whose angles are A = 277, $B = 64^\circ$, $C = 804^\circ$, $D = 100^\circ$, is divided into two equal parts by the line EF.

dividing AD and BC proportionally. Required AE and BF. And AE = 6.22 ch, BF = 10.15 ch.

- 6 Within a tract of land whose sides are 1st. E. 45.58 ch., 2d. N. 13\frac{1}{2}^{\circ}\$ W., 40.86 ch.; 3d. S. 82° W., 30.40 ch., 4th. S. 9\frac{1}{2}^{\circ}\$ W., 36 ch. there is a spring whose hearing and distance from the 3d corner is S. 21° W., 15.80 ch. It is required to cut off 40 A, from the north side of this tract by a line running through the spring and intersecting the 2d and 4th sides. Required the distance from the 1st corner to the point of intersection on the 4th side.

 [108, 26.73 ch.]
- ch., 2d. N. 11]* W., 15.25 ch.; 8d. N. 51]* W. H. 50 ch. 4th. S. 27° W., 24.82 ch.—is to be divided into four equal parts by two lines, one parallel to the first side, the other intersecting the first and third sides. Required the distances cut off by the parallel from the first and second corners, measured on the fourth and second sides, respectively; also the distances cut off by the other line from the first and fourth corners, measured on the first and third sides, respectively.

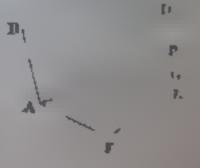
Ans. 8.57 ch., 7.79 ch., 10.66 ch., 3.15 ch.

330. Division of Polygons.

1. From a given point in the boundary of a tract of land, the land ings and distances of whose sides are given, to run a line which shall cut off a given area.

Let A be the point, and suppose it probable that the dividing line will terminate on DE. Suppose the closing line AD to be run, the bearing and distance of which can be found on the

S N. 29.



plying on smars, from the bearings and distances of AB, I'C, and CD. Compute the area of ABCD, which, if I so than the area to be cut off, subtract from that are a which gives the addition, a, to ABCD. The bearings of AD and DE give the angle ADE.

The perpendicular, $AG=AD\sin ADG$.

Then, if AP is the dividing line, $DP = \frac{a}{\frac{1}{2} AG}$.

If $DP > DE_r$ run another closing line AE_r and proceed as before.

If ABCD is greater than the area to be cut off, subtract the area to be cut off from ABCD and divide the difference by one-half the perpendicular from A to CD, and the quotient, if less than DC, will be the distance from D to the point on DC to which the division line is to be drawn.

If the quotient is greater than DC, run another closing line, AC, and proceed as before

2. Through a given point within a tract of land, the bearings and distances of whose sides are given, to run a line which shall cut off a given area

Let P be the given point. Run a trial line, AB, and calculate the area which it cuts off.

Let d be the difference between this area, which we will suppose too small, and the sarea to be cut off.

Let CD be the division line required

Let m = AP, and n = PB, which measure; find the angle PAC, also PBD. We are to find the angle P.

 $C = 180^{\circ} \leftarrow (A + P_{+}) \text{ and } D = 180^{\circ} = B + P_{+}.$ $\sin C = \sin (A - P_{+}) \text{ and } \sin D = \sin (B + P_{+}).$ $PC = \frac{m \sin A}{\sin (A + P_{+})} = \frac{m \sin P}{\sin (A + P_{+})}$ $\therefore APC = \frac{m^{2} \sin A \sin P}{2 \sin (A + P_{+})}$ $PD = \frac{n \sin B}{\sin (B + P_{+})} = \frac{n \sin P}{\sin (B + P_{+})}$ $\therefore BPD = \frac{n^{2} \sin B \sin P}{2 \sin (B + P_{+})}$ $d = \frac{m^{2} \sin A \sin P}{2 \sin (A + P_{+})} = \frac{n^{2} \sin B \sin P}{2 \sin (A + P_{+})}$ $2d = \frac{m^{2}}{\cot P + \cot A} = \frac{n^{2}}{\cot P + \cot B}$

Use natural co-tangents, find cot P, and then P.

331. Examples.

1. The boundaries of a tract of land are: AB, W. 25 cb : BC, N 32½° W., 16.09 ch.; CD, N. 20° E., 15.50 ch.; DE, E. 25 ch.; EF, 8, 80° E.; and FA, 8, 25° W., to the point of beginning. A line is run from A, cutting off 70 A. 1 R. 33 P. from the west side. Required the and point in which this line cuts the boundary.

Ans. The side DE, 18 ch. East of P

2. It is required to run a line through a point, P_c within a field, so as to cut off 10 A. A guess line through P_c intersecting opposite sides in A and P_c cuts off 9 A. Required the angle which the true division line, CD_c makes with AB_c if AP = 12 ch. PR = 4 ch. $PAC = 90^\circ$, $PBD = 60^\circ$.

LEVELING

332. The Y Level.

The Y level, so called from the form of the supports a which the telescope rests, is exhibited in the an nexed engraving

The telescope is inclosed in rings, by which it can be revolved in the Y's or clamped in any position.

The Y's have each two nuts, adjustable with the steel pin, and the rings are clamped in the Y's by bringing the clips firmly on them by means of tapering Y pins.

The interior construction of the telescope is exhibited in the following figure.



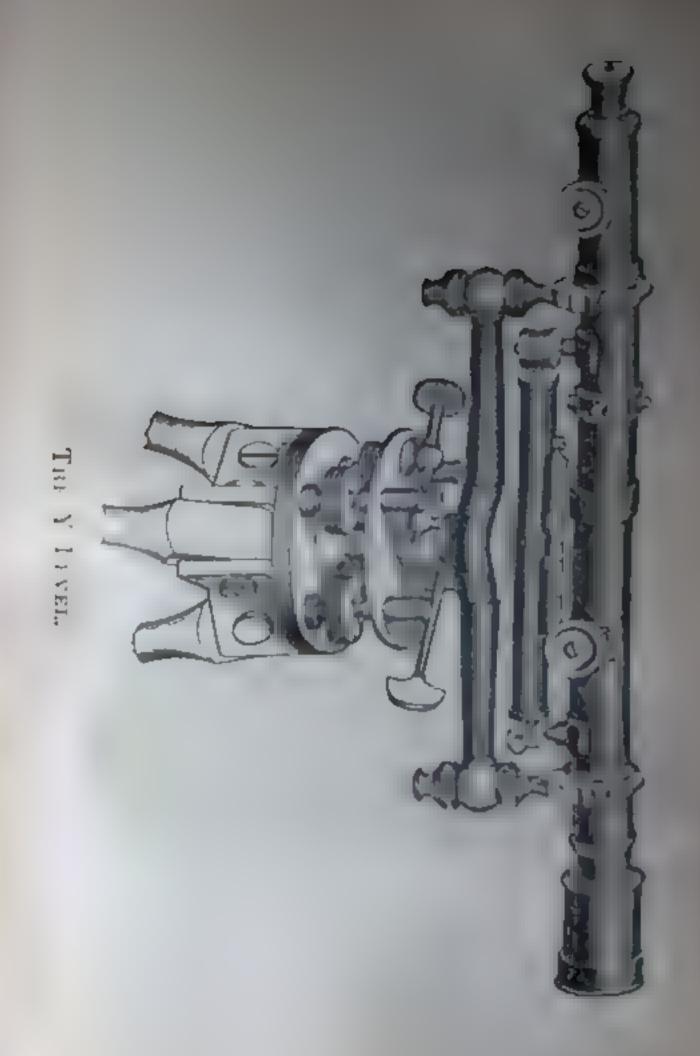
The rack and pinion, AA and CC, are contrivances, the first for centering the eye-piece, and the second for insuring the accurate projection of the object-glass in a straight line

The level is a ground bubble tube, attached to the under side of the telescope, and furnished at each end with arrangements for the usual movements in both horizontal and vertical directions

The tripod head is similar to that in the transit.

333. Adjustments.

1. To adjust the line of collimation, set the tripod firmly, remove the Y pins from the clips, so that the telescope shall turn freely, clamp the instrument to



the triped head, and by means of the leveling and taugert serows, bring either of the wires to bear on a charly marked edge of an object, distant from two to five hundred feet

Turn the telescope half-way round, so that the same wire is brought to bear on the same object.

Should the wire not range with the object, bring it half way back by moving the capstan head screws, BB, at right angles to it, in the opposite direction, on account of the inverting property of the eye-piece, and repeat the operation till it will reverse correctly.

Proceed in like manner with the other wire.

Should both wires be much out, adjust the second after having nearly completed the adjustment of the first, then complete the adjustment of the first.

To bring the intersection of the wares into the center of the field of view, slip off the covering of the eye-piece centering screws, shown at AA, and move, with a small screw-driver, each pair in succession, with a direct motion, as the inversion of the eye-piece does not affect this operation, till the wires are brought, as nearly as can be judged, into the required position.

Test the correctness of the e - ; by revolving the telescope and observing whethe tappears to shift the position of an object

If the position of the object is shifted by revolving the telescope, the centering is not perfectly a complished.

Continue the operaton till the centering is perfect.

2. To adjust the level bubble, clamp the instrument over either pair of leveling screws, and bring the bubble to the middle

Revolve the telescope in the Y's so as to bring the level tube on either side of the center of the level but.

Should the bubble run to one end, rectify the error by bringing it, as nearly as can be estimated, half-way back with the capstan screws in the level holder.

Again bring the level over the center of the bar, and bring the bubble to the center; turn the level to one side, and, if necessary, repeat the operation till the bubble will keep its position when the tube is turn due to either side of the center of the bar.

Now bring the bubble to the center with the leveling screws, and reverse the telescope in the Y's without jarring the instrument. Should the bubble run to either end, lower that end, or raise the other by turning small adjusting nuts at one end of the level till, by estimation, half the correction is made.

Again bring the bubble to the middle, and repeat the operation till the reversion can be made without causing any change in the bubble.

3. To adjust the Y's, or to bring the level into a position at right angles with the vertical axis, so that the bubble will remain in the center during an entire revolution of the instrument, bring the level tube directly over the center of the bar, and clamp the telescope in the Y's, placing it, as before, over two of the leveling screws, unclamp the socket, level the bubble, and turn the instrument half-way around, so that the leveling screws beneath.

Should the bubble run to either end, bring it half-way back by the Y nuts on either end of the bar.

Now move the telescope over the other set of leveling screws, bring the bubble again into the center, and proceed as before, changing to each pair of serews, successively, till the adjustment is nearly completed, which may now be done over a single pair of screws.

334. The Use of the Level.

Set the legs firmly in the ground, test the adjust-

Bring the wires precisely in the focus, and the object distinctly in view, so that the spider lines will appear tist and to the surface of the object, and will not change in position however the eye be moved

The bubble resting in the middle, the intersection of the spider lines will indicate the line of apparent level.

335. Leveling Rod.

The New York Leveling Rod, represented in the engraving with a piece cut out of the middle, so that both ends may be exhibited, consists of two pieces, one sliding from the other.

The graduation commences at the lower end, which is to rest on the ground, and is made to tenths and hundredths of a foot.

A circular target, divided into quadrants of different colors, so as to be easily seen, moves on the front surface of the rod, which reads to six and one-half feet.

If a greater height is required, the horizontal line of the target is fixed at 6½ feet, on Q the front surface, and the upper part of the rod, which carries the target, is run out of the lower, and the reading is obtained on the graduated side up to an elevation of twelve ft.

* A clamp screw on the back is used to fasten the rods together in any position.





336. Definitions.

A level surface is the surface of still water, or any surface parallel to that of still water.

Such a surface is convex, and conforms to the spheroidal form of the earth.

A level line is a line in a level surface

The difference of level of two places is the distance of one above or below the level surface passing through the other.

Leveling is the art of ascertaining the difference of level of two places.

The apparent level of any place is the horizontal plane tangent to the level surface at that place.

The line of apparent level of any place is a horizontal line, tangent to a level line at that place.

The Y Level indicates the line of apparent level and not the true level, which is a curved line.

The correction for curvature is the amount of deviation for a given distance of the line of apparent level from the line of true level to which it is tangent at the point from which the distance is measured.

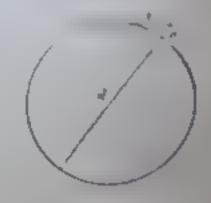
337. Problem.

To compute the correction for curvature.

Let t denote the tangent, c the correction for curvature, d the diameter of the earth.

Then, by Geometry, we have,

$$(d+c) c = t^2, \quad \cdot \quad c = \frac{t^3}{d+c}.$$



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Since c is very small compared with d_i it can be dropped from the denominator without sensibly affect.

1.2 the result.

$$c = \frac{t^2}{d}$$

The are, which is the distance measured, will not it I r perceptibly from the tangent, for all distances at which observations are made, and may be substituted for it.

Calling another distance, t, and the corresponding correction, c', we have,

$$c'=rac{\ell'^2}{d}$$
, $c=c'=-\ell^2-f^2$,

1. The correction for curvature, for a given distance, is equal to the square of the distance divided by the diameter of the earth.

2. The corrections for different distances are to each other as the squares of the distances.

Let us find the correction for the distance 100 chains, calling the diameter of the curth 7920 ;

The correction for any other disting a recognition of any other disting a recognition.

For I mile, 100° 80° 12.5; c_1 ... c_n .031 inches. For muliles, 100° 80° 12.5; c_n ... c_n 8 melas. For muliles, 1° : m° :: 8: c_n ... c_n 8 m° in.

A correction for refraction is sometimes rule by diminishing the correct on for curvature by k of itself

If the leveling matrice at is placed made ty between the two places whose debrace of level is to be found, the curvature and refrection on the two sides of the instrument balance, and the difference of apparent level will be the difference of true level.

338. Problem.

To find the difference of level of two places rusible from a point midway between them or from each other, when the difference of level does not exceed twelve feet,



Let A and B be the two places, and C the place midway from which both are visible.

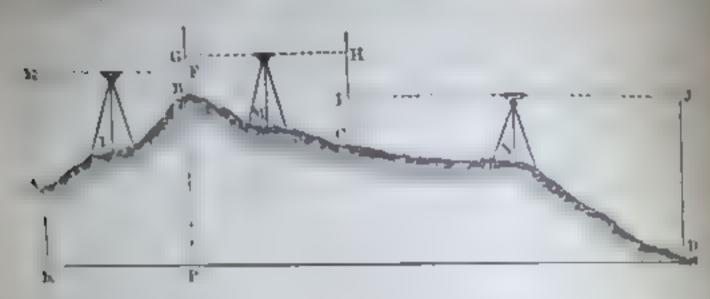
Place the level at C, and let the rod-man set up the leveling rod at A, and slide the vane till he learns, by signal from the surveyor at the level, that its horizont line is in the line of apparent level. Let the height be a curately observed and noted, and the rod be transferred to B, and the height observed and noted as before

The difference of these heights will be the difference of level.

If a gully intervene, so that the line of apparent level, from the intermediate station, would not cut the rod, place the instrument at one station, and take the leight on the staff at the other station marked by the vane when in the line of apparent level, from which subtract the height of the instrument, and the difference corrected for curvature and refraction will be the difference of level required

339. Problem.

To all the difference of level of two places which differ consalerably in level, or which can not be seen from each other.



Let A and D be the places whose difference of level is required.

Place the level at the station L, midway between two convenient points, A and B. Take the backsight to A, and note the height of E. Send the rod to B, and note the height of the foresight at E. Remove the level to M, note the height of the backsight at G and the foresight at H. Remove the level to N, note the height of the backsight at L.

Then will the difference of the sum of the backsights and the sum of the foresights be the difference of level of A and D.

For, we find for the sum of the backsights,

$$AE + BG + CI - AE + BF + FG + CL$$

And, we find for the sum of the foresights,

$$BF + CH + DJ = BF + CI + IH + DJ$$

 $BF + CI + PG$

The sum of the backsights, minus the sum of the foresights. AE + FG = PG AK = d (Frence of level, which in the field notes is denoted by D, L₀ If the sum of the foresights exceeds the sum of the backsights, the point D is below A_i ; if the reverse were true, the point D would be above A_i as indicated by the sign.

It is not essential that the intermediate stations be directly between the places.

340. Field Notes.

Stations.	Backsights.	Forenghia
1	5.40	1.50
2	3 12	5 25
3	2 40	8.16
Sums	19.92	14 91
	14.91	
D. L.	- 3.09	

341. Leveling for Section.

Leveling for Section is leveling for the purpose of obtaining a section or profile of the surface along a given line.

A Bench-mark is made to indicate the beginning of the line by drilling a rock or driving a nail into the upper end of a post. Such marks should be made at different points along the line, to serve as checks in case of a new survey.

It is necessary also to measure the distance between the stations. The bearings of the lines should be taken in case a map or plot is to be made, representing the horizontal surface. In the following table of specimen field notes, S, described as stations; B, bearings, D, distances; B, S, backs sights, F, S, foresights; B, S, $\longrightarrow F$, S, backsights minus for sights; T, D, L, total difference of level; R, remarks, and B, M, bench-mark.

The numbers in the column headed B. S.—F. S. are obtained by subtracting each foresight from the corresponding backsight, observing to write the proper sign.

The numbers in the column headed T.D.L. are obtained by continued additions of the numbers in the column B.S.-F.S., each being the sum of the backsights minus the sum of the foresights, up to a given point, expresses the distance of that point above or below the bench-mark at the beginning of the line.

The minus sign of a result indicates that the sum of the foresights exceeds the sum of the backsights, and hence, that the corresponding state is below the first station; the plus sign indicates the reverse.

In order to bring out prominently the difference of level, the vertical distances are use potted on a much larger scale than the horizontal

Let us suppose the numbers in the column D, express chains, and that the numbers in the following columns express feet.

In the following profile section the horizontal distances are plotted to the scale of 20 chains to an inch, and the vertical distances to the scale of 20 feet to an inch.

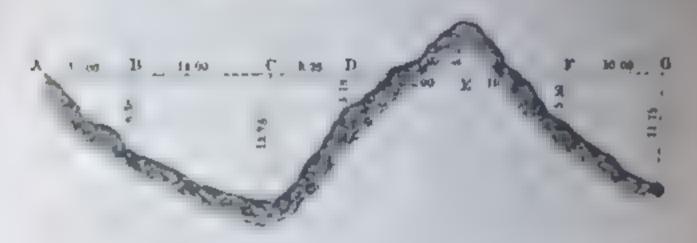
The profile of the section is therefore distorted, the vertical distances being 66 times too great to exhibit their true proportion to the horizontal distances.

The horizontal line, AG, through the point of beginning is called the datum line.

342. Field Notes.

S.	B.	D.	B S.	F S	BS + FS	T.D.I.	R
1	N.	10 00	3 25	11 63	- 8 38	- 8 38	BMs on post
2	N.	14 00	4 80	10.20	- 540	13.78	
3	N.	8 25	12 00	1 40	+ 10.60	- 314	BM on rock.
4	N.10°E	12 00	10.80	2 30	+ 8 50	+ 532	
	N.10°E				10 82		
6	N.	10 00	2.15	8 40	— 6 25	-11 75	BM on oak

343. Profile of Section.



SURVEYING RAILROADS.

344. General Plan.

The surveys for the construction of railroads, applicable also to canals, graded pikes, dikes, etc., are made in the following order.

- I. The reconnoissance, to locate the route. The termini being agreed upon, sometimes several routes are examined, so that an approximate judgment can be formed in reference to the economy of construction and purchasing the right of way, the amount of stock taken at different towns along the route, and the profits from local business.
 - 2. The transit survey, to determine definitely the

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in lidle line along the surface, after the route has been it ciled upon by the preliminary reconnoissance.

- 3. The section leveling, to determine the profile of the middle line along the surface
- The cross-section work, to determine the position and slopes of the sides, so that the amount of cart., to be removed or filled can be estimated.

345. Section Leveling.

Section leveling is simply an application, with slight medications, of leveling for section, before described.

The first bench-mark is assumed at some convenient point near the beginning of the line, and its location described in the column of remarks

The datum line is generally assumed at such a depth below the first bench-mark—for example, at mean high-tide water, in case one end of the route is in the vicinity of tide-water—that its whole length shall be below the section line at the surface

The engineer's chain, 100 feet in length, is usually employed in taking the horizontal distance.

A turning-point is a hard point chosen as far in advance as possible, but not necessarily in exact line, upon which the rod rests while a cure it reading is taken just before it is necessary to character the position of the instrument, whose exact height about the datum line thus becomes known in the new position.

The difference between a turning-point and a beach is this:

A turning-point is merely a temporary point, neither marked nor recorded, used to determine the height of

the instrument in a new position. A bench is both marked and noted, and thus made permanent.

If, however, it is thought best to make a turningpoint permanent, it is marked and recorded, and becomes a bench.

In order that a bench be not destroyed in constructing the road, it should be a little removed from the line surveyed. The location of the benches should be carefully noted, so that they may be readily found from the field notes.

The plus sights are the first readings of the rod, made, free each new position of the instrument, as the rod stands on a bench or turning-point, and are taken to thousandths of a foot.

The minus sights are the other readings, and are taken to tenths, except the last minus sight, before the position of the instrument is changed, which, being taken as the rod stands on a turning-point or beach, is taken to thousandths.

The height of the instrument above the datum line is equal to a plus sight, plus the height of the corresponding bench or turning-point.

The height of the surface above the datum line, at any position of the rod, is equal to the height of the instrument, minus the corresponding backsight.

These heights are taken at intervals of I chain, and at intermediate points where the irregularity of the surface is deemed sufficient to render it important

In the following field notes D, denotes distance; B, bench; T, P, turning-point; +S, plus sight; H, I, height of instrument; -S, minus sight; S, H, surface height; G, H, grade height; C, cut; F, till; R, remarks.

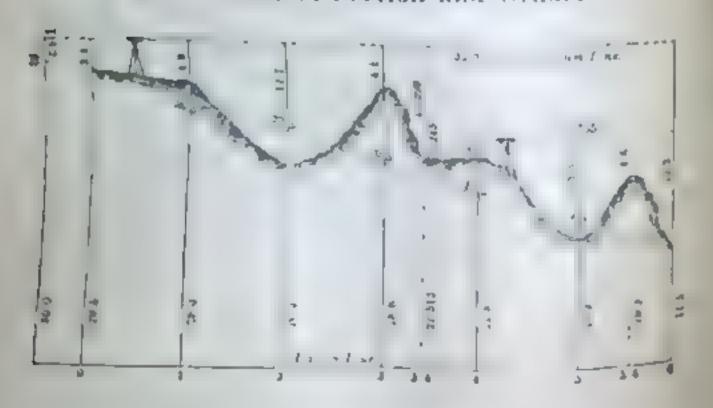
S. N. 30

346. Field Notes.

$D \to S \cap H I$	-8	8 II.	G/H	C.	F.	R.
R 2 01 32911		30				B. 50 ft.
0	3.4	29.5	29.5			E of
1	4.9	28.0	26.5	1.5		Ostake
3 m4	127	20.2	23.5		3.3	
3.	4.1	28.8	20%	83		
T P 2.243 23.755	11 399	21.512	19.2	2.3		
4.	2.0	21.4	17.5	-1-1		
2	12.5	11.3	14.5		1.2	
5.6	4.6	19.2	12.7	1		
R. 1	12.3	11.5	11.5			

The numbers in the horizontal column, T. P, are found thus: The -S, 11.399, is obtained from the first position of the instrument by the reading of the rod on T. P. 21.512 = 32.911 - 11.399. The +S, 2.243, is the reading of the rod from the new position of the instrument. 23.775 = 21.512 + 2.213. The cutting or filling is the difference of S. H, and G. H

347. Profile of Section and Grade.



348. Remarks.

- 1. The grade height at 0, minus the grade at 6, which is 29.5 11.5 18. the descent from 0 to 6. 18 + 6 = 8 = the descent for 1 chain, 29.5 3 = 26.5. G. H. at 1; 26.5 3 = 23.5 = G. H. at 2, etc.
- 2. The establishment of the grade is influenced by the object of the work, economy, the balance of cuttings and fillings, the points desirable for termini, etc.
- 3. The method exhibited above may be extended to any distance.

349. Example.

Fall out the notes of the following table, and make a profile of section and grade from S. H. at 0 to S. H. at 5.

D	8.	11.7	- 8.	S.H.	[a, H]	0	F	R.
B	6.218	36.248		30				R 20 Ω S, of 0,
0			53					.,,,,,
2	10.718		23 11811	ı				
1 P	10.119		7.6					
Ļ			15.0		1			
1			2.1		1	1		

350. Cross-Section Work.

Excavations and embankments are constructed with sloping sides, in order to prevent the sliding of earth down the surface.

The ratio of slope is the vertical distance divided by the horizontal, and is therefore the tangent of the angle which the sloping surface makes with a hori zontal plane.

The usual ratio of slope is \$, and the angle 33° 41'.

Slope stakes are driven to mark where the sloping a leas whether of cutting or filling, will intersect the surface, and thus indicate the boundaries of the work,

The rod used in cross-section leveling is 15 feet long, grad d and plainly marked to feet and tenths, and is not by the leveler at the instruments.

The assistants of the leveler are the redman, arman, and two tapemen.

The Field book is ruled into four columns, headed D, for distance; L for left; C. C. for center-cut; R. for right.

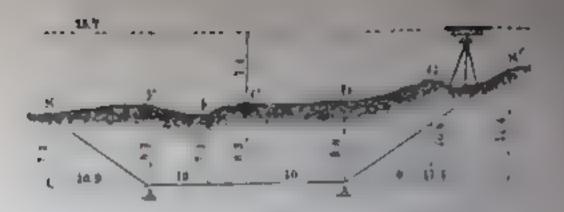
The numbers in the columns D, and C, C, are, respectively, the distance and the corresponding cut, or fill marked minus cut, taken from the field book for section leveling.

The fractions in the columns L, and R, have for their numerators the vertical distances of the cross-section, and for their denominators, the corresponding horizontal distances, from the center or from the vertex of the angle of slope, according as the vertical distance is taken within or without the limits of the horizontal portion of the road.

351. Cross-Section Exervations.

We give the following profile of cross-section, the method of performing the field operations and recording the notes.

Let us suppose the cross-section to be taken at the distance 3 of the field notes of article 313, where the center cut is 8.3; that the mad hed is 20 feet wide, that the ratio of slope is 3, and that both horizontal and vertical distances are plotted to the scale of 20 feet to 1 inch.



Take AA' for the datum line, and suppose the reading at the center stake to be 7.4. The height of the instrument above the datum line is therefore 83 \pm 7.4 \pm 157.

The reading of the rod at the depression F, between the center and the angle A, is 8.5; hence, the cut is 15.7 \times 5.5 \times 7.2 The horizontal distance, CF, is 4 feet; hence, the record in the field notes, as seen in the next reticle in the column L, is $\frac{7.2}{4}$.

The reading of the rod, at the temporary stake E_1 is 7.4; hence, the cut is 15.7-7.4=8.3, and the entry, $\frac{8.3}{A}$.

The point S, where the slope intersects the surface, is found by trial. Since the vertical distance of the slope is $\frac{1}{2}$ of the horizontal, then ES, if horizontal, would be $\frac{1}{2}$ of EA, which is 12.4; but, on account of the inclination of the surface, ES will be less, say 10 feet. Setting the rod 10 feet out from E, the reading is 83, and hence the cut = 15.7 - 8.3 = 7.4. Now, $\frac{1}{2}$ of 74 is 11.1; hence, the assumed distance, 10 feet, is too small

For a second trial, take 11 feet out from E, at which the reading of the rod is 8.4, and the cut 7.8. Now, $\frac{1}{2}$ of 7.3 . 10.9, which lacks but .1 of 11, and is sufficiently accurate. The record for the slope stake, in the column L, is $\frac{7.3}{10.9}$.

The reading of the rod at the stake D is 6.9; hence, the cut is SS, and the record in the column R is $\frac{SS}{A'}$

The reading at the elevation G is 5.1; hence, the it is 10.6. The horizontal distance, DG, is 9 feet; hence, the record is $\frac{10.6}{9}$.

To find S' where the slope intersects the surface, since, on account of the rising of the surface, it is more than $\frac{1}{2}$ of 8.8, which is 13.2, take, for a first trial, 18 fet out from D, at which point the realing of the rod is 4.5, and hence the cut 15.7 — 4.5 11.2. Now, I of 11.2 = 16.8; hence, 18 feet is too far out.

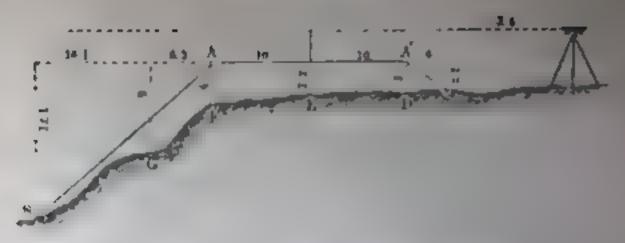
For a second trial, take 17 feet out from D. The reading of the rod is 4.3, and the cut 15.7-4.3=11.4. Now, $\frac{1}{4}$ of 11.4=17.1, which is sufficiently accurate; hence, the record for the slope stake S', in the column R, is $\frac{11.4}{17.1}$.

352. Field Notes.

353. Cross-Section Embankments.

The following is the profile of the cross section drawn to a scale of 20 feet to 1 inch, taken at the distance 5 of the field notes of article 346, where the filling is 3.2, now called a minus cut, and written — 3.2.

Take AA', which is the horizontal top of the embankment 20 feet wide, for the datum line.



The ratio of slope, in case of embankments, is - f.

The reading of the rod at the center stake is 6.6, and the height of the instrument, with reference to the datum line, is the algebraic sum of the reading of the rod and the minus cut, which is 6.6 — 3.2 — 3.4

If the instrument should be below the datum line, the reading of the red would be numerically less than the minus cut, and the height of the instrument would be negative.

The readings of the other points along the surface SS, subtracted from the height of the instrument, will give the corresponding minus cuts.

The reading at A is 7.4, the cut, -4, and the record, $\frac{-4}{A}$.

The reading at G is 12.4, the cut, -9, the horizontal distance FG, 6.3, and the record, $\frac{-9}{6.3}$.

To find the position of the slope stake S, take for the first trial 20 feet out from F, where the reading is 15 and the cut, — 12.6. Now, — 12.6 \times — $\frac{1}{2}$ — 18.9; hence 20 feet is too far out.

Next try 18 feet out, where the reading is 15.5, and the cut, -12.1. Now, $-12.1 \times -\frac{1}{2} = 18.1$, which is sufficiently accurate; hence, the record for the slope stake S is $-\frac{12.1}{18.1}$.

RAILROADS.

361

The reading at A' is 6.4, the cut, -3, and the reading at A' is 6.4, the cut, -3, and the reading at A' is 6.4, the cut, -3, and the

To find the position of the slope stake S', take for the first trial 5 feet out from D_i where the reading is 6.2, and the cut, -2.8. Now, $-2.8 \times -\frac{1}{2}$ 4.2; hence, 5 feet is too far out.

Next take 4 feet out, where the reading is 6.1, and the cut, 27. Now, $27 - \frac{1}{2} - \frac{4}{4}$; hence, the record for the slope stake S' is $\frac{-27}{4}$.

354. Field Notes.

355. Remark.

It sometimes occurs that an excavation will be required on one side, and an embankment on the other. Guided by the stakes and field notes, the excavations and embankments can be correctly made

356. Computation of Earth-work.

The computation of earth-work is the determination of the volume of excavation or embankment

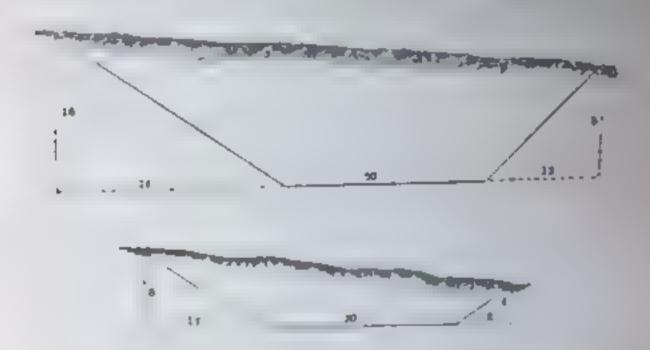
The cross-sections, being taken, wherever necessary, at every 100 feet or less, divide the exervations of embankments into blocks, which may be regarded as frustums of pyramids.

Denoting the areas of the sections regarded as bases of the frustum by b and b', respectively, the length by l, and the volume by v, we have the formula,

$$v = \frac{1}{3}l(b+b'+1-bb').$$

357. Examples.

1. The length of an excavation is 100 feet; find the volume, the two ends being thus represented:



The area required, in each case, is the area of the whole figure, regarded as a trapezoid, which is one-half the altitude multiplied by the sum of the parallel bases, minus the sum of the two triangles; hence,

$$b = 28 \times 24 - (24 \times 8 + 12 \times 4) = 432.$$
 $b = 19 - 12 - (12 + 4 + 6 \times 2) + 168.$
 $c = \frac{1}{3} \times 100 (432 + 168 + 1/432 \times 168).$
 $c = 28980$ cubic feet = 1073 cubic yards.

2. Compute the volume of the embankment whose horizontal breadth at the top is 16 feet, from the following field notes:

 4 6	Fa. c.		
,,		-	
		71. 1	

D	L	c.c.	
5	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	10 15	-9.5 - 8.6 $A' - 13$ $-14.2 - 13$ $A' - 19.5$
_		-	Ans. 1607 (u.)

358. Remarks.

1. The above method of computing earth-work is called by engineers The mean average method,

2. The method known as The arithmetical mean method is easier than the above, though less accurate.

The following is the formula:

3. The volume can also be computed as a rectangular presmoid.

4. Irregularities in the cross-section surface line, as elevations, depressions, or a curvity, and the line, must be considered.

Thus, the elevation may be regarded as a triangle, its area computed and added to



the trapezoid before the area of the two triangles at the right and left be deducted

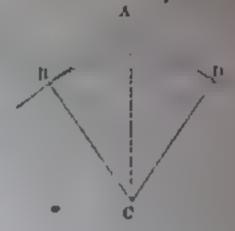
359. Railroad Curves.

In the preliminary - irvey of a railroad, any change in direction is made by an angle which must, in the final survey, be replaced by a curve, to which the sides of the angle are tangents

Let the annexed diagram represent such an angle and curve.

Run out one of the tangents, as B.t, to E, and let A denote the external angle EAD.

Then we shall have C = A, since each is the supplement of BAD, the angles B and D being right angles.



Let r = BC, the radius of curvature, and t = AB, the tangent.

Then, (1)
$$t = r \tan \frac{1}{2}A$$
, (2) $r = \tan \frac{1}{3}A$

The degree of curvature is the number of degrees in an arc whose length is I chain or 100 feet.

360. Problem.

Given the degree of curvature, to find the radius; and, con-

$$\frac{2\pi r}{360} = \frac{\pi r}{180} = 1^{\circ}$$
 of circumference,

$$\frac{d \pi r}{180} = d^{\circ}$$
 of circumference.

Hence,
$$\frac{d \pi r}{180} = 100$$
, $\begin{pmatrix} (1) & r = \frac{18000}{d\pi} \end{pmatrix}$.

Having found the radius of curvature, we can find the tangent, or the distance from the vertex of the angle to the point where the curve begins by formula (1) of the preceding article.

361. Examples.

- Find r of 1° of curvature and t, if .1 40°,
 this, r 5729.58 ft., t 2087.4 ft.
- 2. Find r of 2° of curvature and t, if $A = 40^{\circ}$.

 Ans. r = 2864.79 ft., t = 1043.7 ft.
- 3. Find r of 3° of curvature and t, if A = 50°,
 Ans. r = 1909.86 ft., t = 890.6 ft.
- 4. Find r and d, if A == 35° and t == 1000 ft.
 Ans. r == 3171.6 ft , d == 1° 48′ 23″.
- 5. Find r and d, if $A = 100^{\circ}$ and t = 1 mile.

 Ans. r = 4430.4 ft, $d = 1^{\circ}$ 17' 35".

362. Location of the Curve.

First Method.

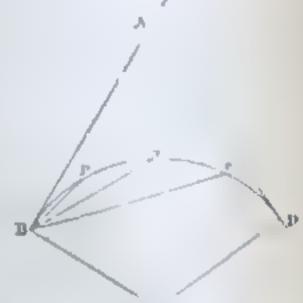
Let each of the ares, Bp, pq, qr, ... be 1 chain, then will the number of degrees in

each, or in the corresponding

angle at the center, be equal

to d, the degree of curvature.

The angle ABp, formed by a tangent and a chord, is measured by one-half the are Bp, and is therefore equal to $\frac{1}{2}d$.



Each of the inscribed angles, pBq, qBr, is measured by one-half the intercepted arc, and is therefore equal to $\frac{1}{2}d$.

Having determined the point B_i , where the curve begins, the transitman sets his instrument at this point, and directs it to A_i . He then turns it an angle equal to $\frac{1}{2}d_i$, on the side toward the curve.

The chainmen then take the chain, the follower placing his end at B, and the leader drawing out the chain at full length toward A, is directed by the transituum into line so as to locate the point p, at which the axman drives a stake.

The transitman again turns his instrument an angle equal to $\frac{1}{2}d$, the chainmen advance, the follower plaining his end of the chain at p, the leader again drawing out the chain at full length, is directed by the transitman so as to locate the point q, at which the aximan drives a stake, and so on.

The last distance will usually not be 1 chain; but if a be the number of preceding deflections, the last angle of deflection, since the sum of all the deflections is equal to $\frac{1}{2}C = \frac{1}{2}A$, will be equal to

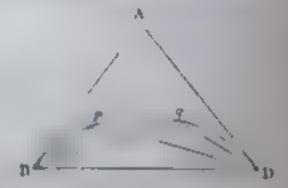
$$\frac{1}{2}A - \frac{1}{2}dn$$
.

It is to be observed that the chord is made equal to 1 chain instead of the are; but as the radius is much greater than the chord, the are and chord will not differ materially, and no appreciable error arises in practice.

Second Method.

Points on the curve may be located by the use of two

transits, without the use of the im, as may be desirable, in the curve is to be located in marshy ground or shallow water.



Let one transit be placed at B and another at D, the extremities of the curve.

Direct the transit at B to A, the one at D to B, then turn each to the right an angle equal to $\frac{1}{2}d^{\circ}$.

The intersection of the lines will determine p, where the axmun, directed by both transitmen, drives a stake.

In like manner other points can be located.

If A is visible from D, but not B, direct the transit at D to A; then, to locate p, turn it to the left an angle equal to $\frac{1}{2}A^{\circ} = \frac{1}{2}d^{\circ}$.

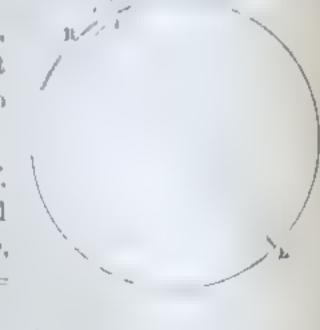
To locate q, turn the transit at D from p to the right an angle equal to $\frac{1}{2}d^{\circ}$, or from A to the left an angle equal to $\frac{1}{2}A^{\circ} - d^{\circ}$, and the transit at B to the right from p an angle equal to $\frac{1}{2}d^{\circ}$, or to the right from A an angle equal to d° , and so on.

Third Method.

Let B be the point where the curve begins. Take

Bm equal to I chain. Then, to find the length of the offset mp, complete the circle, draw the diameter BE, let fall the perpendicular pn to BE, and draw pE.

In the right triangle BpE, Bp is a mean proportional between BE and Bn; hence, $BE \times Bn = Bp^2$; but BE = 2r, Bp = 1, and Bn = mp,



$$mp = \frac{1}{2\bar{r}}$$

To find q, produce Bp till pa = 1 chain, and draw tr, tangent to the curve at p.

Then,
$$spv = tpB = mBp = vpq$$
.

For the first and second are vertical, and all the rest are included between tangents and equal chords.

... spq = 2 mBp, ... the arc sq = 2 arc mp, Or, the arcs being small, do not differ materially from their chords,

$$\therefore sq \approx 2mp = \frac{1}{r}.$$

Hence, to locate a curve by this method without the transit, commence at B, where the curve is to begin, take Bm=1 chain in the direction of the straight track, make the offset $mp=\frac{1}{2r}$, produce Bp till p^s-1 chain, make the offset sp equal to twice the first offset, produce pq till the produced part = 1 chain, make an offset equal to the last, and so on.

Fourth Method.

It is evident from the diagram that

$$mp = BC - nC$$
.

But BC = r, and $nC = 1/r^2 - t^2$, u

$$\cdot, \quad mp =: r \to 1 \cdot r^2 \to t^2.$$

By giving to t different values, other points of the curve can be determined.



It is evident from the diagram that

$$mp = mC - Cp$$
.

But $mC = 1^rr^2 + t^2$, and Cp = r.

$$: mp =: \sqrt{r^2 + t^2} - r.$$

In this method the offset is not made at right angles to the tangent,

but in a direction toward the center, which is supposed to be visible from m.



369

The precising methods apply to points of the curve I chain or 100 feet from each other, which will be a flatent for the excavations or embankments.

15 fore laying the track, stakes are driven at points on the curve, distant from each other about 10 feet.

363. Problem.

To locate intermediate points on the curve.

Let the diameter in the diagram be parallel to the

chain == 100 feet, the ordinates a, b, c, d, c, f, e, d, c, b, a be 10 feet from each other, and e, w, x, y, z, y, x, w, v be offsets from the chord to the



curve, corresponding to the ordinates b, c, d, c, f, e, d, c, b.

The square of an ordinate is equal to the rectangle of the segments into which it divides the diameter.

$$a^2 = -(r - 50) \cdot (r + 50), \quad a = 1 \cdot (r - 50) \cdot (r + 50).$$
 $b = 1 \cdot (r - 40) \cdot (r + 40), \quad v = h - a$
 $c = 1 \cdot (r - 30) \cdot (r + 30), \quad u$
 $d = 1 \cdot (r - 20) \cdot (r + 20), \quad x = d - a.$
 $c = 1 \cdot (r - 10) \cdot (r + 10), \quad y = c - a.$
 $f = r, \qquad z = f - a.$

364. Example.

Find the radius of a 1° curvature, and the offsets from the chord of 100 feet to the curve.

Ans
$$\begin{cases} r = 5729.58 \text{ ft., } v = .08 \text{ ft., } w = .14 \text{ ft.} \\ x = 19 \text{ ft., } y = .21 \text{ ft., } z = .22 \text{ ft.} \end{cases}$$

TOPOGRAPHICAL SURVEYING.

365. Definition and Method.

Topographical surveying is that branch in which the form of the surface, the situation of pends, streams marshes, rocks, trees, buildings, etc., are considered and delineated.

The surface is supposed to be intersected by horizontal planes equally distant from each other, and the curves formed by the intersection of the planes and the surface projected on a horizontal plane

These projections will be nearer together or farther apart, according as the slope of the surface approaches a vertical or a horizontal position.

The operations are of two kinds - field operations and plotting.

366. Field Operations.

Field operations consist in finding and recording points of the curves of intersection of the surface and the horizontal planes, the course of streams, and the situation of noteworthy objects on the surface.

Range with the level, or transit theodolite, which is more convenient in topographical operations, stakes marked as in the annexed diagram, and cause them to be driven into the ground, at a horizontal distanction each other of 100 feet or less, varying with the inequality of the surface and the degree of accuracy with which it is desirable that the work be executed

Find by the eye, or by the instrument if necessary, the lowest point in the field, at which make a permanent beach-mark, and assume for the plane of reference the

Lorizontal plane passing through this point, which we will suppose to be C_1 .

Piece the instrument at some convenient station, S, from which take the reading of the rod at C₁, which appear to be 10.378, and outer this as a backsight in the field notes.

Take the readings of the rod at as many stakes as

possible from the station S. Suppose these readings to be C_2 , 6.481; C_3 , 1.214; D_4 , 8.235; D_2 , 6.378; D_3 , 4.102; D_4 , 2.304, and enter these readings is the field notes as foresights, placing the smallest reading, C_3 , last.

At Ca drive a small stake for a check

Subtract the foresight C_2 6.481 from the backsight 10.378, and enter the difference in the column of difference, headed D_{ij} also in the column of total difference of level above C_0 , headed T_i , D_i , L_i

Subtract each of the remaining foresights from the next preceding one, and enter the results, with their proper signs, in the column D

Add each result to the previous total difference of level, and enter the results in the column T. D. L.

The total difference of level for C_4 is also found by subtracting the foresight of C_5 from the backsight of C_1 , which, compared with the result before found, will serve as a check

Move the instrument to S'_1 and take a backsight to the check stake C_3 , and the foresights to as many of the remaining stakes as possible, suppose all of them and enter the readings in the field notes as before.

Subtract the first of these foresights from the backsight C_2 , and add the result to the total difference of level for C_2 , and enter the sum in the column T.D.L.

Subtract each of the following foresights from the next preceding foresight, and enter the result, with its proper sign, in the column D., and add it to the next preceding difference of level, and enter the sum in the column T. D. L.

As a check, subtract the foresight of B_* from the backsight C_* ; the difference will be the height of B_* above C_* , which add to the former check number, which is the difference of level of C_* and C_1 , and the sum will be the total difference of level of B_* and C_1 .

Compare the explanations of this article with the field notes of the following article.

367. Field Notes.

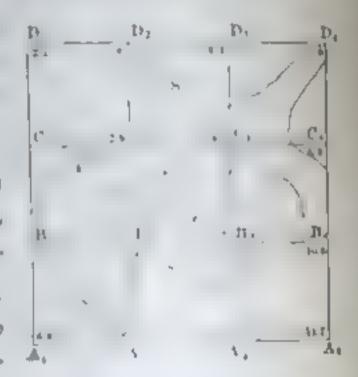
$r=\frac{1}{R_{-}S_{r}}$	$F \sim$	D.	7 D L.	R.
C 10.378	$C_1 = 6.481$ $D_1 = 8.235$ $D_2 = 6.378$ $D_3 = 4.102$ $D_4 = 2.304$	+ 2 276 + 1 798	$egin{array}{ c c c c } C_4 & 0.000 & \\ \hline C_7 & 3.897 & \\ D_4 & 2.143 & \\ D_4 & 4.000 & \\ D_3 & 6.276 & \\ D_4 & 8.074 & \\ \hline \end{array}$	
1% 9:687	$C_{\rm f} = 1.214$ $C_{\rm f} = 12.000$ $B_{\rm g} = 11.845$ $B_{\rm g} = 5.184$	- 2313 + 0.155 + 6.661	$R_2 = 13.667$	Check 9 164
	$R_4 = 8.311$ $A_4 = 12.000$ $A_1 = 11.321$ $CA_1 = 10.987$ $A_4 = 7.125$	+ 0.679 + 0.334	$A_1 = 0.851$ $A_2 = 7.564$	
	B ₃ 0 1 12		$B_3 = 18.719$	Check 9 555 18719

368, Plotting.

Let the annexed diagram be a plot of the ground on which is written, with red ink, the height to tenths, taken from the field notes, of the surface, at each stake, above the plane of reference passing through C_1 .

Let us suppose that the horizontal planes intersecting the surface are 4 feet apart.

The intersection of the surface and the plane 4 feet above the plane of reference crosses the line $A_1 D_1$ between the points $B_1 C_1$, at a point 4 feet above C_1 .



To determine this point, observe that the rise from C_1 to B_1 is 7 feet. Then the distance on this line from C_1 to the point where the height above C_1 is 4 feet is found by the proportion,

$$7:4::100:x, x=571.$$

This method assumes the ascent to be uniform between B_i and C_i ; but this point can be tested and other points of the curve found as follows: Set up the instrument at S_i and make the backsight to C_i 10.378, the same as before, then depress the year on the red 4 feet—that is, to the reading 6.378.

Now let the rodman set up the rod at the point between C_t and B_t determined from the proportion, and let the surveyor observe whether the horizontal wire of the telescope ranges with the horizontal line of the vane; if not, let the rod be moved a little toward B_t of C_i till they do range, and at the point thus determined let a stake marked 4 be driven by the axman.

An inspection of the plot will show that the curve passes between B_2 and C_2 at a distance from C_2 found from the proportion,

$$9.8 \pm .1 \pm 100 \pm z$$
, . . $x = 1$.

Let the rodman advance toward this point, pausing at one or two intermediate points, and at this point, whose positions are definitely determined and marked

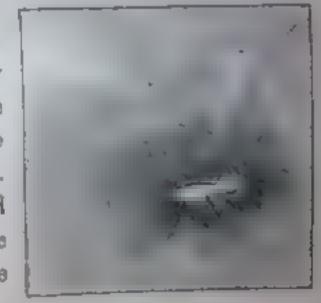
In a similar manner determine where the curve crosses C_2 C_4 and trace it to D_2 .

In like manner, trace the curves of intersection of the surface and planes, 8 feet, 12 feet, and 16 feet above the plane of reference, and let these curves be marked on the ground by stakes numbered 8, 12, and 16, respectively.

The horizontal distance of each stake from two sides of a square can be measured and recorded. From this record the surveyor can draw the curves on the plot as exhibited above.

369. Shading.

The slopes may be represented to the eye by short lines drawn perpendicular to the curves, marking the intersection of the surface with the horizontal planes. These lines are heaviest and closest where the slopes are steepest, and lighter where the slopes are less abrupt.



370. Conventional Signs.

The following conventional, though not altogether art trary signs, are used to indicate objects worthy of unter-

Waste.	Pasture.	Markey	Facilda.
Savil.	Gardens	Distreta	*ATTO
, ,		C. C	
Cetten	Huntre	Variant	H (-44.
	7		
Tumpeke, I		Harland, men a me	
Constant Road,		Harthard area	22.000
Fast Poths		Jesses Const.	
Lack Root,		5 II	
Store Bringe,	-: 3	West less	4 100
Suspension Resign.	2 .	P-152 - 2 (1 - 2	
Carnage Ford,	h	Hirw Erst	
Canal & Lock,	*	State Deci-	107
Do Water Mit.	-1 Radrand Station.	 Tamemark street 	× re₹
	Telegraph Station.	T - most	p pard
"an Post Other.		who have a	
D Hotel	A Monnth	True	
Cost in History	I Way math	a subsequence of	A 2 1 - 3 E
Bulling Woo	L 2 Nile Stone.	C Rock torre	4
" Finan	Later Kar	". Nu ker recke	_ \ Carrent.
⊕ 6 at 1 □) Saur	a typer	î te
If Tim.	3 1/21	Q Membry	• -

371. Finishing a Map.

The points of compass are indicated as is usual, the top of the map denoting the north, etc., etc.

The meridian, both true and magnetic, should be drawn, and the variation of the needle indicated.

The lettering should be executed with care, after printed models of various styles.

The border may be made by a heavy line, relieved by a light parallel.

The title, in ornamental letters, should occupy one corner of the map, with the name of the locality, the dates of the survey and drawing, and the names of the surveyor and draughtsman.

The scale of horizontal distances, for finding and comparing distances on the map, and the scale of construction, used in the smallest measurements required in projecting dimensions in the drawing, should be accurately drawn in some convenient position within the border.

Parallels of latitudes and meridians, in extended surveys, should be drawn in their true position.

BAROMETRIC HEIGHTS.

372. Preliminary Remarks.

The barometer affords an approximative method for ug the difference of level of two stations.

To attain to as great a degree of accuracy as possible, it is important to employ two good barometers, one at the lower and the other at the upper station

Before using the barometers, they should be carefully compared by frequent trials, and the variation ascertained, which is to be allowed for in the observations

Increased accuracy is attained by making repeated observations, and taking the mean of the results.

To guard against varying local conditions of the atmosphere affecting pressure, beside difference of elevation, the stations should not be distant from each other more than four or five miles; and the observations should be made when there is no wind.

373. Builey's Formula.

The subjoined formula requires a knowledge, at both stations, of the height of the column of mercury, its temperature as indicated by an attached thermometer, the temperature of the air as indicated by a detached thermometer, and the latitude of the locality

Let d denote the difference of level in feet;

4, the latitude of the place in degrees;

h, T, t, respectively, the height of the b rometer, the temperature of the mercury, and the temperature of the nir at the lower station;

h', T', t', respectively, the same at the upper station.

Then,
$$d = 60345.51 [1 + .001111 (t + t' - 61)]$$

 $\times (1 + .002695 \cos 2t) \times \log \frac{h}{h' [1 + .0001 (T - T')]}$

Let
$$A = \log \{60345.51 \{1 + .001111 (t + t' - .04)\}\}$$
,
 $B = \log (1 + .002695 \cos 2t)$,
 $C = \log [1 + .0001 (T - T')]$,
 $D = \log h - (\log h' + C)$.
 $\therefore \log d = A + B + \log D$.

This formula is applied by the aid of the tables:

374. Howlet's Tables.

Table A, for Detached Thermometer.

1.0	.l.	t - t	.1	$t \cdot t'$	al.	$t \cdot t'$	A.
10	4,74914	46°	4 77187	91°	1 79348	1362	1.81407
	.74966	47°	.77.286	920	.79395	1374	,81452
65 7g	.75017	480	77285	932	.79442	1,50	,83,4998
3° 4°	75069	49°	.77335	949	.79489	1392	81511
FO	,75120	50°	.77384	95°	.79535	1407	,81586
100	75172	51°	.77433 1	(H)2	.79542	1412	816.39
D	P.C. 113-3	52°	.77482	970	.798-28	1425	,81674
	73274	53°	.77530 t	9×6	.79675	1432	.81719
*		54°	.77579	997	.79721	144°	.81763
97	75326	55°	.77628	300"	.79768	1452	.81807
1.	2 111	66°	.77677	101°	.79814	1462	.81851
11	7.3328	57°	007 ,5	102°	7986£	1472	81896
1.5	2014	58°	77774	1032	759907	1482	.81940
1 1	41.31	20g	77823	104°	.79958	1 kgs	81984
147	7 1083	600	.77871	105	79999	1507	82028
101	75633	612	77919	1068	,80045	151°	,82072
16	7365	62	77968	107	.80091	1520	82116
37	7,733	61	78016	1081	.80137	1532	82160
*	75250	64°	78005	1092	.80183	Lot 1	82204
10.	73537	65	78113	110°	,80229	1551	K2248
50.	75555	file.	78161	1110	80275	1562	.82291
21°	7.7938	67	75200	112°	80321	107°	82 540
1.5	75 (89)	657	78257	1132	80367	158°	185 (2.0)
23	76039	69	78505	1142	,80413	1 39°	82423
21	76090	70"	78353	115	801.8	1602	82466
212	76140	71	78401	1160	80504	1610	,82 (10)
26.	76 (90	25,	78449	1177	,80550	162°	83.63
47	76241 76291	731	78497	1180	80595	1632	82597
24		713	78544	1392	80641	164°	,82640
10	76342	730	783/12	1201	SDOSE	1652	382654
303	.76392	760	,78640	121°	,807.31	166°	827.27
312	.76442	m = 3	75657	122	80777	1670	.82770
3.22	76492	783	78735	1237	80822	1687	,82814
137	76542 76592	71)2	78783	1241	80867	1690	.82877
ı		50	788,0	1350	.80913	1701	8,2800
	76642	811	78877	1260	80958	1717	8,294.1
30	T6004	Q.	78925	1270	.81008	172]	8,2080
17	76.13	130	78072	12%	.81048	173]	9/309,90
	76792	84	79019	1,2413	81093	1749	8.072
10	76842	3	79066	130°	811.8	1750	83315
40	76831	Nis	79113	1310	81183	1767	8 (158
11	76940	870	79160	1.32	81228	1770	ह्मा
4.	74,990	24.	79.917	1.15	81273	1787	8.211
	77039	5.9	79.254	1.44	ा आसर्	179	,83287
11	.77089	901		Let'	81362	180	61173
£ ≥2	• 77188						

Take B, for Latitude.

	B	t,	B	1,	B.	1.	B_{r}
0,	0.00117	27	евонов	50	1 99980	500	1,99945
41	00116	3113	000.33	510	99976 (002	99941
6'	00114	33	(10045	522	319973	tide	.,99981
132	.00111	2005	,000,03	532	, 19968	667	
120	00107	521	00021	54°	19996	Peto	.,99913
152	30100	42"	00012	552	Center .	75	199499
143	(1001)5	40	-{кини)	56	Otherste	807	
212	.00057	44	1.99988	572	, 999 2	551	99985
210	00075	4192	99964	240	11/2/12/21	002	299883

Table C, for an Attached Thermanter.

T - T'	C	T-T	C.	T-T'	C	1 1	C
09	0.00000	121	0.00052	241	0.00104		0.00156
13	HOUSEN,	132	Otto	252	00108	7	00161
92	(Реизия)	144	DOOGL	26	00215	7	0016
33	00013	15°	00065	97	00[17]	91	00 [6]
40	00017	163	раныч	25	0012	, C	0017
50	00022	173	00074	24	06.125	3.)	00174
6°	.00026	182	00078	30	ab t		0018.
70	,000 30	1412	00082	31	+10)	0018
gs.	000035	2112	00087	3.2	(N1)	1.5	.0019
90	(2)(0)(1)	21	00091	22.3	0011.	1.4	0010
10°	00043	53,	000005	11	00148	14	_0020
il°	.00045	2)	00100	35	.00152	,	(9020

375. Examples.

1. At the mountain Guanaxuato, in Mexico, lat. 21°, Humboldt made the following observations:

Barometric column, h=30.05, h'=23.66.

Attached thermometer, $T=77^{\circ}.6$, $T'=70^{\circ}.4$.

Detached thermometer, $t=77^{\circ}.6$, $t'=70^{\circ}.4$. $\log d=A+B+\log D$.

$$\log h (30.05) \sim 1.47784 \qquad A = 1.81940$$

$$\log h'(23.66) = 1.37402 \qquad B = 0.00087$$

$$\text{Table } C \text{ gives } C = 0.00031 \quad \log, D = 1.01498$$

$$\log h' + C = 1.37433 \quad \log, d = 3.83525$$

$$D = \log h + (\log h' + C) = 0.10351 \quad \therefore, d = 6843 \text{ ft.}$$

2. Find the difference of level of two stations, lat. 42°, from the following data:

$$h = 30, T = 75^{\circ}.5, t = 75^{\circ}.$$

 $h' = 25, T' = 70^{\circ}.3, t' = 70^{\circ}.$ Ans. 5195. (t.

3. Find the difference of level of two stations, lat. 45°, from the following data:

$$\frac{h}{h'} = \frac{29.2}{27.1}, \quad \frac{T}{T'} = \frac{80^{\circ} 3}{77^{\circ} A}, \quad \frac{t}{t'} = \frac{80^{\circ}}{77^{\circ}}, \right\} \quad Ans., \quad 2149.9 \text{ ft.}$$

1 Find d, lat 50°, from the following data:

$$\frac{h}{h'} = \frac{29}{28}, \frac{T}{T'} = \frac{60^{\circ}.1}{59^{\circ}.1}, \frac{t}{t'} = \frac{60^{\circ}.}{59^{\circ}.1}$$
 Ans. 973 8 ft.

376. Leveling with one Barometer.

Take the observations at the lower station, then proceed to the upper station and take the observations there, and note the interval of time which has intervened, then go back to the lower station and at the expiration of an equal interval repeat the observations.

Reduce the mercurial column of the second observation at the lower station to what it would have been at the temperature of the first observation, on the principle that mercury expands or contracts .0001 of its volume for each degree of increase or diminution of temperature.

Then take the arithmetical mean of this reduced height and the first observed height for the height at the lower station, the mean of the temperature denoted

by the detached thermometer at the lower station for ti. temperature of the air at that station, and the temperature denoted by the attached thermometer at the first observation for the temperature of the merenty, then proceed as if the observations had been taken with two barometers.

377. Examples.

1. Lower stat. { 1st obv., h = 29.62, $T = 56^{\circ}.5$, $t = 56^{\circ}$. } 2d obv., h = 29.63, $T = 63^{\circ}$, $t = 61^{\circ}$. Lat. 41° 4, upper sta, R 28 94, T' 57° 5, C 57°.

Reducing h of 2d obv. from $T=63^{\circ}$ to $T=56^{\circ}5$, we have,

Reduced $h = 29.63 (1 - 6.5 \times .0001) - 29.611$.

Mean $t = \frac{56^{\circ} + 61^{\circ}}{2}$ 58° 5 $t + t = 58^{\circ}.5 + 57^{\circ}$ 11 5 and T T' 56 5 - 57 5 1 log h | 29 6155 | 1 17173 | 4 | 1 80481 log h' (28.94 | 1.46150 | / 0.00014 $C = -0.00001 \log D - 2.00280$ $\log h' + C = 1.46146 \log d + 2.80755$ $D = \log h + (\log h' + C)$ 0.01006 0 612 feet 2. Lat. 40°; upper sta h' 286, T' 62, t 60° Lat. 40°; upper sta h' 286, T' 62, t 62°. 3. {Lower sta. { 1st oby., h = 29.65, T = 70, $t = 50^{\circ}$, 29.65, T = 40, $t = 46^{\circ}$. Lat. 50° ; upper sta. h' = 27.6, T' = 40, $t' = 45^{\circ}$. Ans. d 1909) ft.

NAVIGATION.

PRELIMINARIES.

378. Definition and Classification.

Navigation is the art of ascertaining the place of a ship at sea, and of conducting it from port to port.

There are two methods of finding the place of a ship:

- I. By dead reckoning; that is, by tracing from the record the courses and distances sailed.
- 2. By Nautical Astronomy; that is, by deducing the latitude and longitude of the place of the ship from celestial observations.

The first method is subdivided into the following:

Plane sailing, parallel sailing, middle latitude sailing, Mercator's sailing, and current sailing.

379. The Mariner's Compass.

The magnetic needle rests on a pivot, so as to turn tr ly.

The compass box is suspended by gimbals or rings. turning on axes at right angles to each other, thus securing a horizontal position notwithstanding the rolling motion of the ship.

A circular card, whose circumference is divided into thirty-two equal parts, called points, each of which is

rests upon the needle, with which it turns freely.



N. b. E. is read north by east; N. N. E., north northeast, etc.

380. Table of Points and Angles.

	No	th.	Son	eth	Augher
1 2 3	N.b.E. N.N.E. N.E.b.N	N.b.W. N.N.W. N.W.b.N	S b.E. S S E. S.E.b.S.	S,b.W. S S.W. S,W.b.S.	11° 15' 22° 30' 33° 45'
4	NE	N.W.	SE	11 -	45° 0′
1 5	NEBE	N.W.b W	SEbE	$S(M, 1, M_s)$	56° 15′
6	ENE.	W.N.W.	ESE	$-W \otimes W$	67° 30′
7	E b.N	W b N.	EbS.	WbS	78° 45′
8	E.	W	E.	W	90° 6′

Note 1.— $\frac{1}{4}$ point = 2° 48' $\frac{1}{4}$, $\frac{1}{4}$ point = 5° 37' $\frac{1}{4}$, $\frac{3}{4}$ point = 8° 26' $\frac{1}{4}$.

Note 2.— The compass is placed near the helm, at the stern, and the line from the center of the compass to the ship's head indicates the track of the ship.

351. Variation and Deviation of the Compass.

The variation of the compass is the angle included between the magnetic meridian and the true meridian.

The amount of variation is ascertained by Nautural Astronomy.

The deviation of the compass is the deflection of the needle from the magnetic meridian, caused by the iron in the ship.

The amount of deviation is ascertained by special experiments.

382. Course, Leeway, Rhumb Line.

The compass course of a ship, at any point, is the angle which her track makes with the magnetic meridian at that point.

The true course of a ship, at any point, is the angle which her track makes with the true meridian at that point.

In the compass course, the deviation is supposed to be ascert fined and allowed for, but not the variation; but in the true course, both the deviation and variation.

The leeway is the oblique motion of the ship, caused by a side wind driving the ship along a track oblique to the fore-and-aft line, and therefore not indicated by the compass.

The amount of leeway, under a wind of a given obliquity and velocity, for each ship with a given freight, is best found by trial.

A rhumb line is the track of a ship which continues to make the same angle with the meridians. It is also called a loxodromic curve.

PLANE SAILING.

385

Sure the meridians converge, the rhumb line is a spiral curve.

In what follows we shall suppose that proper allowan es have been made for the variation and deviation of the compass, and, therefore, that the courses given at the true courses.

383. The Log and Log Line.

The log, a drawing of which is annexed, is a board in the form of a quadrant whose radius is about six inches, the circular part of which is loaded with lead, sufficient to

cause it to sink so that the vertex shall be just above the surface.

6.5

The log line is a line about 120 fathoms in length, and so attached to the log as to keep its face toward the ship, that it may, by the resistance it encounters from the water, unwind the line from a recl as the vessel advances.

The log line is divided into equal parts called know, each knot being the of a nautical mile, or 50% feet.

The time is measured by a sand glass, through which the sand passes in The of an hour, or in hof a minute.

Since the number of knots in a nautical mile is equal to the number of half-minutes in an hour, it follows that the number of knots run off in half a minute is equal to the number of miles the ship is sailing an hour.

The divisions of the line are marked by strings passing through the line and knotted, the number of knots in the string indicating the number of parts between

it and that point of the line where the divisions commence at that end of the line next to the log

The stray line is about 10 fathoms of the end of the line from the log to the point where the divisions begin. This portion allows the log to settle in the water, clear of the ship, before the measurement of the rate begins.

The termination of the stray line is marked by a piece of red cloth.

The sand glass is turned the instant this cloth passes the reel, which is stopped the moment the sand has run out.

The number of knots on the string which marks the last division run from the reel, indicates the rate of sailing.

PLANE SAILING.

384. Single Courses.

Let P be the pole of the earth; RQ, the equator; AD, a rhumb line divided into AB, BC, CD, etc., parts so small that we may regard them as straight lines; and the triangles ABE, BCF, CDG, plane triangles and similar, which give the continued proportions:

AB:AE::BC:BF::CD:CG.

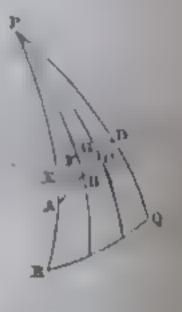
AB : EB :: BC : FC :: CD : GD.

Since the sum of the antecedents is to the sum of the consequents as one antecedent is to its consequent, we have,

 $AD: AE + BF + CG:: AB \cdot AE$

 $AD : EB + FC + GD :: AB \cdot EB$.

S. N. 33.



Now let a right triangle, ABC, be constructed, in which C is the course or the argle which the rhumb line makes with the no ridians r = CB = AD, the rhumb of the first figure; l = CA = AE + BF + CA = difference of latitude; <math>d = CA = AE + BF



EB + FC + GD = the sum of the elementary departures.

We may now, without supposing the ship to sail on a plane, replace the surface on which it as tually sails by a plane surface, and hence the name plane scaling.

385. Table of Cases.

٢	Giren.	Rq.			Form	das.	
1	r, C ,	i, d		ŧ	r cos C,	d	r sin C
2	r, l,	$C_{i}(d)$	CON	e^{-}	7	1	1 r2 t
3	r, d.	$C_i I$	5111	C	$-\frac{d}{r}$,		1 ri- 12
ŧ	10,1	$r_{i}(\theta)$		t.	l asc.		I tan C
5	c, d,	$r_i = l$		I,	$\frac{d}{\sin C}$	1	tan C
. 6	, d.	r. C		1*	$1/l^2 + d^2$,	tan C	d .

Note 1.—I in miles may be reduced to degrees by dividing by 60.

Note 2 — Examples in case I. may be solved by the Traverse table.

386. Examples.

1. A ship sails 105 miles N. E. by N., from latitude 50°; required the latitude in which the ship then is, and the departure made.

Ans. 51° 27' 3 N , d 58 34 mm.

2. A ship sailed between S. and W 118 miles, making the difference of latitude 1144, required the course and the departure made.

Ans. 31 pts W of S, d = 939 mi.

3 A ship in latitude 3° 52′ S, sails between N, and W, 1065 miles, making a departure of 939 miles; required the course and the latitude in which she then is Ans. N. W. b. W. 3W., Lat. 4° 30′ N.

1 A ship ran from latitude 38° 32' N. to latitude 36° 56' N. on a course S. E. by S. 4 E., required the distance sailed and the departure made

Ans. r = 129.56 mis d -87.009 not

5. A ship sailed S. 56° 47′ E, from latitude 50° 13′ N, till her departure was 82 miles; required r and latitude in.

Ans. r = 98 mi., lat. 49° 19′ N,

6. A ship from latitude 36° 12' N. sails between S. and W till she is in latitude 35° 1' N. having made 76 miles of departure; required r and C

Ans. r = 104 mi, C = S. 46° 57' W.

387. Compound Courses.

A compound course or traverse is the sigsag course which a ship usually takes in a voyage of considerable length.

Working the traverse is the computation of a single course and distance from the place of departure to the place of destination.

To do this, find by the Traverse table the latitude and departure of each course. The difference of the sum of the northings and the sum of the southings will be the latitude of the single course required, and the date rence of the sum of the eastings and the sum of the westings will be the departure, both of the name of the greater. Then proceed as in last article,

388. Examples.

1. A ship sailed from latitude 51° 24' N. as follows: S. E. 40 miles, N. E. 28 miles, S. W. by W. 52 miles, N. W. by W. 30 miles, S. S. E. 36 miles, S. E. by E. 58 miles; required the latitude in, and the single equivalent course and distance.

Solution.

Courses	Dist	N/L	S L	F(D)	W D
S. E.	40		25.3	28.3	
N. E.	28	19.8		10.8	112
8. W b W.	52		25.9		219
	30	16.7			
S. S. E.	36		93.3	138	
S. E. b. E.	58		32.2	45.3	
		36.5	122.7	110 1	68.1
			365	65.1	
			86.2	1 42	
tan C	7 : : : : : : : : : : : : : : : : : : :	12 .		9 25	5+
r =	= 1 l2.	-di =	95,87	mî.	

N.

2 Given the following courses and distances S W W. 62 miles, S by W 16 miles, W. 18 40 miles, S W

² W. 29 miles, S. by E. 30 miles, S. ³ E. 14 miles; required 4, C, and \(\tau\).

Ans. l = 1° 55′ S, C S. 43° 11′ W., r 158 mi

3. A ship, from latitude 1° 12' S, has sailed as follows: E. by N. & N. 56 nules, N. & E. 80 miles, S. by E. 1 E. 96 miles, N. 1 E. 68 miles, E. S. E. 40 miles, N. N. W. & W. 86 miles, E. by S. 65 miles; required the latetude in, C, and r.

Ans. Lat. in, 0° 48' N., C 51° 47' E., r 193.8 mi.

PARALLEL SAILING.

389. Definition and Principles.

Parallel sailing is that case of sailing in which the track is on a parallel of latitude.

Let EFQ be the equator;

GAB, the parallel of the track;

r = AB = the distance sailed;

L = FQ = the difference of longitude; l = QB = the latitude of the track.

Since similar arcs are to each other as their radii,

(1) DB : CQ :: AB : FQ.

Consider the radius CQ as the unit of the first couplet, then DB will be the natural co-eine of latitude; and take I mile as the unit in the second couplet, put r for AB, L for FQ, then (1) becomes,

(2)
$$\cos l = 1 : r : L, \ldots$$
 (3) $L = \frac{r}{\cos l}$

We can compute L in (3) by taking nat cos l, or by introducing R and taking log cos l. In either case L will be found in miles, since r is given in miles; but L can be reduced to degrees by dividing by 60.

Let r and r, measured on the parallels whose latitudes are l and l, respectively, be the distances between two meridians whose difference of longitude is L.

$$\cos l + 1 + i + i + L_i$$
 $\therefore \cos l + \cos l + i + i + l_i$

Hence, The distances between two meridians, measured on differ it parallels, are as the co-sines of the latitudes of those principles.

To find the length of a degree of longitude on any parallel, observe that at the equator 1° of lon. = 60 nautical miles, and that $\cos t = 1$, then we shall have,

1 :
$$\cos l'$$
 :: 60 : r' , ... $r' = 60$ cas l' .

390. Examples.

- 1. A ship in latitude 49° 32' N., and longitude 10° 16' W., sails due W. 118 miles; required the longitude arrived at.

 Ann. 13° 18' W.
- 2. A ship in latitude 53° 36' N., and longitude 10° 18' E., sails due W. 236 miles; required the longitude arrived at.

 Ann. 3° 40' E.
- 3. A ship in latitude 32° N. sails 6° 24' due W.; required d.

 Ans. d = 325.6 mi.
- 4. A ship sails 310 miles from longitude \$1° 36' W. to longitude 91° 50' W.; required the latitude of the track.

 Ans. 59° 41'.

MIDDLE LATITUDE SAILING

391. Definition and Principles.

Middle latitude sailing is a combination of plane sailing and parallel sailing, on the supposition that the departure in plane sailing is equal to the distance

between the meridians passing through the extreme points of the rhumb line, measured on the middle parallel between these points.

Let AD be a rhumb line; IK, the middle parallel; m, the latitude of IK; then d = EB + FC + GD - IK.

For r, formula (3), parallel sailing, substitude d or its value as found in plane sailing; and for $\cos l$ substitute $\cos m$, then we shall have,

$$L = \frac{d}{\cos m} = \frac{\tau \sin C}{\cos m} = \frac{1}{\tau^2} - \frac{l^2}{l^2} = \frac{l \tan C}{\cos m}.$$

Note 1.—Remember that in these formulas t denotes the difference of latitude; L, the difference of longitude in miles; d, the departure; τ , the distance run or the rhumb line; C, the course, and m, the middle latitude

Note 2.—The middle latitude is the half sum of the extreme latitudes; or the less latitude, plus the half difference of latitude; or the greater latitude, minus the half difference of latitude.

Note 3.—That the departure is not strictly equal to the middle-latitude distance between the meridians, through the extremities of the rhumb line, is thus shown:

Suppose a ship to sail on this middle latitude from one of the meridians to the other, then the distance sailed will be the departure; but if a second ship were to sail from a lower latitude on the first meridian, and a third ship, from a higher, to the same place, the departure of the second would be greater, and the departure of the third would be less than that of the first.

It is necessary, therefore, to make the correction for middle latitude as found in the table for such corrections.

The following is the rule for correcting the middle

Ald to the uncorrected middle latitude the correction f and in the table under the difference of latitude, and of posite the middle latitude - the sum m' is the corrected middle latitude.

...
$$L = \frac{d}{\cos m'} \cdot \frac{r \sin C}{\cos m'} \cdot \frac{1}{\cos m'} \cdot \frac{t^2}{\cos m'} \cdot \frac{l^2}{\cos m'} \cdot \frac{l \tan C}{\cos m'}$$

392. Examples.

1. A ship from latitude 51° 18' N., longitude 9° 50' W., sails S. 33° 8' W. 1024 miles; required the latitude and longitude in.

$$l = r \cos C_1$$
 ... $l = 857.4 \text{ mi.} = 14^{\circ} 17'$.

... 51° 18'— 14° 17' = 37° 1', the lat. in.

151° 18'+37° 1') == 44° 9½'= mid. lat., correction 27'.

 $44^{\circ} 9\frac{1}{4}' + 27' = 44^{\circ} 36\frac{1}{2}' = m' =$ corrected mid. lat.

$$L = \frac{r \sin C}{\cos m'}$$
, $L = 786.3 \text{ mi.}$ 13° 6'.

2. A ship, from latitude 52° 6 N., and longitude 35° 6 W., sails N. W. by W. 229 miles; required the latitude and longitude arrived at.

Aus. Lat. 54° 13' N. and lon. 40° 23' W.

3. A ship from latitude 49° 57' N., and longitude 5° 11' W., sails between S. and W. till she is in latitude 38° 27' N., when she has made 440 miles departure; required C, r, and the longitude in.

Ans. C = S. 32° 32′ W.; r = 818 mi.; Ion. in, 15° 28′ W.

4. A ship from latitude 37° N., longitude 22° 56′ W., sails N. 33° 19′ E. till she is in latitude 51° 18′ N. What longitude is she in?

Ans. 9° 45′ W.

5. A ship from latitude 40° 41′ N., longitude 16° 37′ W, sails between N and E, till she is in latitude 43° 57′ N., and finds that she has made 248 miles departure; required C, r, and longitude in.

Ans. $C = 51^{\circ} 41'$ E.; r = 316 mi.; lon. in, 11° W.

MERCATOR'S SAILING.

393. Definitions and Principles.

Mercator's chart, so called from its originator, Gerrard Mercator, a Fleming, who first published it in 1556, is a representation of the surface of the earth on the supposition that the earth is a cylinder.

The meridians are thus represented parallel and every-where too far apart except at the equator.

To guard as much as possible against distortion, the distances between the parallels are proportionally increased.

The surface is thus relatively magnified more and more toward the poles.

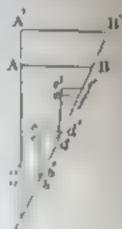
Mercator's sailing is the method of computing the difference of longitude from the principle on which Mercator's chart is projected.

The mathematical theory of this method was developed, and the Table of Meridional Parts, necessary to its application, computed by Edward Wright, an Englishman, in 1599.

Let CA and AB, respectively, be the difference of latitude and departure corresponding to the rhumb line CB, and let
CA be produced to A' till A'B', the corresponding departure, is equal to the differ-

ence of longitude of C and B. C.I' is called the meridto-of defence of latitude, which is simply the proper difference of latitude increased till the corresponding departure is equal to the difference of longitude corresponding to the proper departure.

To find the meridional difference of latitude, let Cb, bd, df, ... be indefinitely small portions of the rhumb line CB. Ca, bc, dc, ... corresponding differences of latitude; ab, cd, cf, ... corresponding differences of departure; Cu', bc', dc', ... corresponding meridional differences of latitude; a'b', c'd',



departures ab, ad, of, ... whose latitudes are l, l', l'', ...

Then, as found in Parallel sailing,

$$ab: a'b' :: \cos l: 1.$$
but $ab: a'b' :: Ca: Ca'.$

$$\therefore \cos l: 1 :: Ca: Ca', \quad \therefore Ca' = \frac{Ca}{\cos l}.$$
but $\frac{1}{\cos l} = \sec l, \quad \therefore Ca' = Ca \sec l.$
In like manner, $bc' = bc \sec l',$

$$de' = dc \sec l''.$$
But $CA' = Ca' + bc' + dc' + ...$

Substituting the values of Ca', bc', de', ... and making Ca = bc = de = ... = 1', we have,

$$CA' = \sec l + \sec l' + \sec l'' + .$$

Commencing at the equator, and putting m. p. for meridional parts, and taking natural secants, we have,

m. p. of
$$1' = \sec 1'$$
.
m. p. of $2' = \sec 1' + \sec 2'$.

By substituting and condensing, we have,

$$m \ p \ of \ 1' = 1\ 00000000 + 1.00000002 + 2.00000002$$

 $m \ p \ of \ 3' = 2.00000002 + 1\ 00000004 + 3.00000006$
 $m \ p \ of \ 4' = 3\ 00000006 + 1.00000007 + 4\ 00000013$

The accuracy of the result is increased by taking the parts still smaller, as ½'.

Having found the meridional latitude corresponding to C, and also to A, their difference will be the meridional difference of latitude found from the table; and the corresponding departure, A'B', will be the difference of longitude.

Denoting the proper difference of latitude CA by I, the meridional difference of latitude by I', the departure AB by A, and the difference of longitude A'B' by L, the triangles CAB and CA'B' give,

$$1: \tan C :: l' : L, \quad \therefore L = l' \tan C.$$

$$l: \quad d:: l' : L, \quad \therefore L = \frac{l'd}{l}.$$

394. Examples in Single Courses.

1. A ship from latitude 52° 6' N., and longitude 35° 6' W., sails N. W. by W. 229 miles; required the latitude and longitude in.

$$l = r \cos C = 229 \cos 56^{\circ} 15', ... l$$
 127 3 mi. 2° 7' lat. in = 52° 6' N. + 2° 7' N · 54° 17' N

49.

m. p. of 54° 13′ 3868 But $L = l' \tan C$, m. p. of 52° 6′ 3657 . . $L = 211 \tan 56$ ° 15′. . . l' = 211 or $L = 315.8 \, \mathrm{mi}$. 5° 16′.

·. lon in 35° 6' W. + 5° 16' W. = 40° 22' W.

2 A ship from latitude 51° 18' N., and longitude 9° 50 W., sails S. 33° 8' W. 1024 miles; required the latitude and longitude in.

Ans. Lat. in 37° 1' N.; lon. in 22° 50' W.

3. Required the course and distance from Ushant, latitude 48° 28' N., longitude 5° 3' W., to St. Michael's, latitude 37° 44' N., longitude 25° 40' W.

Ans. S. 54° 30′ W., r = 1106 mi.

- 4. A ship from latitude 51° 9' N. sails S. W. b. W. 216 miles; required the latitude in, and the difference of longitude made. Ans. Lat. 49° 9' N., L. 4° 39'.
- 5. A ship sails from latitude 37° N., longitude 22° 56′ W., on the course N. 33° 19′ E., till she arrives at latitude 51° 18′ N.; required the distance is the longitude arrived at. Ans. 1027 mi., lon 4° 47′ W.
- 6. A ship sails N. E. b. E. from latitude 42° 25' N., and longitude 15° 6' W., till she finds herself in latitude 46° 20' N.; required the distance such d. and the longitude in.

 Ans. Dist., 423 mi.; lon. 6° 55' W.

395. Examples in Compound Courses.

1. A ship from latitude 60° 9' N, and longitude 1° 7' W, sailed as follows: N. E. b. N., 69 miles; N. N. E., 48 miles; N. b. W. ½W, 78 miles; N. E., 108 miles; S. E. b. E., 50 miles; required the latitude and longitude in, and the direct course and distance.

r								•
Courses,	II_{id}	$\langle N \rangle L_c$	$\langle S_{i} \rangle L_{i}$	Lat	m. p.	m d t	F, L	$W L_{\perp}$
	_		-				***	
N. E. b. N.	6.7	57-4		60597	4525			i
, N. N. E.	48	444		61°6′	4641	116	77.5	
NPAM	78	74.6		61°50′	4733	92	38 1	1
N E.	108	76.4		63°5′	4895	102		49,
S. E b E.	50		27.8	64214	5007	172	172.0	
		252.8		632537	5003	64	95.8	
		27.8						
Dif lat.	1	225 m	i 3	° 45′ N			383.4	

Dif. lon. L = 334.4 mi. Lat. Left = 60° 9' N. Dif. lon. = 5° 34' E Dif. Lat. = 3° 45' N. Lon. left 1° 7' W. Lat. in 63° 54' N. Lon. in = 4° 27' E.

m. p. of lat. in $(63^{\circ} 54') = 5005$. m. p. of lat. left $(60^{\circ} 9') = \underline{4525}$. Meridional dif. lat. = l' = 480.

$$\tan C = \frac{L}{t'} = \frac{334.4}{480}, \dots, C = N, 34° 53' E.$$

$$r = \frac{l}{\cos C} = \frac{225}{\cos 34° 53'}, \dots, r = 273 \text{ mi.}$$

2. A ship from latitude 38° 14' N., and longitude 25° 56' W., has sailed the following courses: N. E. b. N. 4E., 56 miles; N. N. W., 38 miles; N. W. b. W., 46 miles; S. E., 30 miles; S. b. W., 20 miles; N. E. b. N. 60 miles; required the latitude and longitude in, and the direct single course and distance.

Ans Lat. in, 40° 2', 3 N.; lon. in, 25° 30' W; C = N, 10° 33' E., r = 110.2 mi.

396. Correction for Middle Latitude.

We are now prepared to understand how the correction for middle latitude, before used, is found,

CURRENT SAILING.

399

I d in tea the proper difference of Intitude;

I, the meridional difference of latitude;

I, the didlers nee of longitude;

m, the middle latitude uncorrected;

c, the correction;

w, the middle latitude corrected.

Then, by Plane, Middle latitude, and Mercator's sailing,

$$\tan C = \frac{d}{l} = \frac{L \cos m'}{l} = \frac{L}{l'}, \quad \therefore \cos m' = \frac{l}{l'}.$$

From which m' is readily found.

Then, c = m' - m. ... m' = m + c.

CURRENT SAILING.

397. Definition and Principles.

Current sailing is the sailing of a ship as affected by a current.

Irrespective of the current the ship would move, in a certain time, a certain course and distance

The current alone would carry the ship, in the same time, a certain other course and distance.

The actual track of the ship, which is the resultant of the two, will bring her to the same portion as if she had sailed separately the two tracks.

Current sailing may therefore be treated as Plane sailing, compound courses.

The set of the current is its direction.

The drift of the current is its velocity.

The set and drift of a current may be ascertained by taking, a short distance from the ship, a boat, which is kept from being carried by the current by letting down, to a considerable depth, a heavy weight, which is attached by a rope to the stern of the beat

The log being thrown from the boat into the current, the direction in which it is carried, or set of the current, is determined by the boat compass, and the rate at which it is carried, or drift of the current, by the number of knots of the log line run out in half a minute.

398. Examples.

1. A ship sails N. W. a distance, by the log, of 60 miles, in a current that sets S. S.W., drifting 25 miles in the same time; required the course and distance.

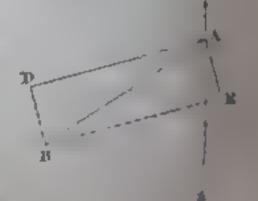
Courses.	Dust	N/L_c	S L	E,D	W, D
N. W S. S. W.	60 25	424	23.1		42 4 9.6
	ı	19.3.		d	52.
$tan C = \frac{d}{t}$	5. 19	3, .·.	. C	N. 69°	3S' W
r 1 /2 : 1	₹ 2 1	(19.3)	دن <u>+</u> د	$1^2 = 5.$	5 5.

2. A ship, sailing 7 knots an hour, is bound to a port bearing S. 52° W., through a current S. S. E., 2 miles an hour; required the course.

Let AB be the direction of the port.

AE, the direction of the current,

AD, the required direction, = 7. Complete the parallelogram, PR.1 = BAE = 52° + 22° 30' - 74° 30'. Then we have,



 $AD:DB:\sin DBA:\sin DAB.$

$$\therefore \sin DAB = \frac{2 \sin DBA}{7}.$$

... $DAB = 15^{\circ} 59'$ $C = 15^{\circ} 59' + 52^{\circ} = 67^{\circ} 59'$.

3. A ship runs N. E. by N. 18 miles in 3 hours, in a current W. by S. 2 miles an hour; required the course Ans. $C = N. b. E. \frac{1}{2}E.$, r = 14 mi.and distance.

4. In a current S. E. by S. 11 miles an hour, a ship sails 24 hours as follows: S.W., 40 m.les; W. S W., 27 nules; S by E, 47 miles; required the direct course and Ans. C = S. 11° 50′ W., r = 117 mi. the distance.

5. The port bears due E., the current sets S. W. by S. 3 knots an hour, the rate of sailing is 4 knots an hour; required the course steered. .1. N. 51° E.

6. A ship sailing in a current has, by her reckoning, run S. by E. 42 miles, and, by observations, is found to have made 55 miles of difference of latitude, and 18 miles of departure; required the set and drift of the Ans. Set, S. 62° 12' W.; whole thrift, 30 mi. current

PLYING TO WINDWARD.

399. Definitions.

Plying to windward is the zigzag course which a ship makes by tacking when she encounters a foul wind.

Starboard signifies the right side.

Larboard signifies the left side.

The starboard tacks are aboard when a ship plies with the wind on the right.

The larboard tacks are aboard when a ship plies with the wind on the left.

A ship is said to be close-hauled when she sails as nearly as possible toward the point from which the wind is blowing.

400. Examples.

1. Being within sight of my port bearing N. by E. 3E., distant 18 miles, a fresh gale sprung up from the N. E. With my larboard tacks aboard, and close-hauled within six points of the wind, how far must I run before tacking about, and what will be my distance from the port on the second board?

Let A be the place of the ship; P, the port; AB, the distance of the first board; BP, that of the second; WA or W'B, the direction of the wind.

Then, WAB = W'BC = W'BP =

6 points.

ABP = 16 points - 12 points = 4 pointsPAW = NAW - NAP = 4 points $-1\frac{1}{2}$ points $-2\frac{1}{2}$ points.

 $PAB = PAW + WAB = 2\frac{1}{2}$ points + 6 points points.

 $APB = 16 \text{ points} - (PAB + ABP) = 3\frac{1}{2} \text{ points}.$

sin ABP: sin APB:: AP: AB, ... AB =: 1615 mi. sin ABP: sin BAP:: AP: BP, ... BP 25 23 mi.

2. If a ship can lie within 6 points of the wind on the larboard tack, and within 51 points on the starboard tack; required her course and distance on each tack to reach a port lying S. by E. 22 miles, the wind being at S. W.

Starboard tack, S. b. E. JE 23 66 mi. Larboard tack, W. N. W. 279 mi.

S. N. 34

A ship is bound to a port 80 miles distant, and directly to windward, which is N. E. by N. ½E., and proposes to reach her port at two boards, each within 6 is uts of the wind, and to lead with the starboard tack, required her course and distance on each tack

Ann. | Starboard tack, N. N. W. J.W., 104.5 mi.

4. Wishing to reach a point bearing N. N. W. 15 miles, but the wind being at W by N. I was obliged to ply to windward the ship, close-hauled could make way within 6 points of the wind; required the course and distance on each tack.

Ans. { Larboard tack, N. b. W. 17.65 mi. Starboard tack, S. W. b. S. 4.138 mi.

TAKING DEPARTURES.

401. Explanation.

Before losing sight of land, at the boring of a voyage, the bearing and distance of the characters, as a light-house or headland. In the reverse bearing and distance of which are entered as the first course and distance on the leg beat.

The bearing is taken by the compass; but the distance is sometimes estimated by the eye, as can be done with considerable accuracy by navigators of experience.

A more correct method of taking a departure is by means of data, obtained by taking the bearing at two different positions of the ship, the distance between these positions being measured by the log.

402. Examples.

1. Sailing down the channel, the Eddystone bore N W. by N., and after running W. S W. 18 miles, it bore N. by E.; required the course and distance from the Eddystone to the place of the last observation.

$$E = NAE + N'BE = 4$$
 points.
 $A = 16$ points $-(NAE + BAS) = 7$ pts.
 $\sin E : \sin A :: AB : BE,$
 $BE = 24.97.$

- 2. At 3 o'clock P. M. the Lizard bore N. by W. W., and after sailing 7 knots an hour, W. by N. W., till 6 o'clock, the Lizard bore N. E. E.; required the course and distance from the Lizard to the place of the last observation.

 Ans. S.W. W., 1935 mi
- 3. In order to get a departure, I observe a headland of known latitude and longitude to bear N. E. by N. and after sailing E by N. 15 miles, the same headland bore W. N. W.; required my distance from the headland at each place of observation.

Jus. 8.5 mi. and 108 mi.

Remark —To find the latitude and longitude of a ship by means of celestial observations, requires a knowledge of Nautical Astronomy; but a thorough discussion of this subject would require an amount of space for exceeding our limits.

TABLES.

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Logarithms of Numbers to 100.

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5	1,50%	1 375	4.800	1.399	4 794		4.788	1.441	5
G	5.768	1 654	5.760	1.679	5,753	1.704	5.745	1.729	6
7	6721	199	6.720	1.959	6.712		6.703	2017	7
- 8	7.6 (0)	22.5	7.680	2 2.9	7,671	2 272	7.661	2 306	8
- 51	85.1	2.481	8.640	2.518	8,629	_	8,613	25.0	9
30	93-13	27.6	9,600	2,798	9.28×	2.840	9.576	2.882	110
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7	0.74	3.828	6 131 1	2 986	6.318		6 305	3041	3
5	7,250	3.381	7235	3 113	7 221	3 444	7 206	3 476	8 9
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1	5564	1.816	3556	1831	3.348	1 847	3 540	1 862 2 628	3
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28'-31'45'	TRAVERSE	TABLES.	58°15′ 62°
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3 2 31 1404	26 7 1 06	3 481 1970	1471 1 15 4
5 4 219	1 42 2 143	4 1 2 2 162	14, 11 5
6 5-24× 2-800	5 200 2 182	3 201 3 34	1, 1 5
7 12 21	6 107 5 100	6002 7417	60 7
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2 1 2 1081	1728 1008	3.725 (1015)	1 14 2
3 216 150	2002 1013	1725 1015 2387 1525	
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3 2 0 1 1 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1	2012 1013 110 2013 110 2	#725 1015 2 85 1525 2 85 2 85 3 85 2 85 2 85 3 80 2 85 3 80 3 100 3 100	1 14 2 3 4 5 6 7 8 9 15 1 10 1 10 1 10 1 10 1 10 1 10 1 10
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p.	Lat	Dep.	Lat.	Dep.	Lat.	Dep	lat.	Dep.	, D
	3.2	07	32	15'	3.3		33	45	
	415	301	.846	7.534	.543		841	1116	ī
2	16.9	1,060	1 691	1,007	1 687				-
3	2 544	1.590	2.537	1.601	2 530		2.523	1.623	3
4	30+2	2,120	3,333	2,134	3 374		3 364	2.164	4
5	4 240	26 (0)	4 229	2.668	4 217		4.205	2,705	5
5	3.088	3,180	5 074	3.202	5 060		5,048	324	7
7	5 956 6 784	3 709 1 239	5,920	3.735	5 904		5 887 6 728	3747	8
8 9	76.3	4 769	7612	4 802	7 591	4 836	7,563	4 709	9
10	× 150	5 209	8 457	5.3.6	8 1 14	5 373	8410	5.410	10
	0.4	o'	57	45'	57	30'	57	15	
	84	O'	23°	15/	33	30'	33	4.5	
1	5,1	F III	N aty	548	.534	5002	Sol	1 .0 10	
13	-1 < 77	Edser	1.673	1.097	1 668	1 104	1 663	11111	2
15	2.516	1 634	2 509	1.645	2,502	1 656	2 49 4 3 326	2 502	3 4
	1111	$\frac{2179}{2723}$	3,345	2.193 2.741	3.336 4 169	2.208 2.760	4 157	2775	5
1.	5 032	3 268	5018	3 290	5 (10.3	3 312	4 989	3333	6
71	5.871	3 412	5 854	3 838	5 837	3 864	5.820	3 459	7
- 54	6.709	4 357	6 690	4 386	6 671	4 416	6 652	4415	81
- 9	7.545	4 902	7 537	4 933	7 505	4 967 5 519	7 453	500) 550,	10
10	4 (87)	5 446		5 4 5 3	8 3 39	807	80	- '	
		ø		5'		30'		13:	
		9	827	503	521	555	- K221	,570	1
2	829 1 658	1115		1.120	1,645	1 133	1633	1140	2
3	2487	1 678	2.480	1.688	2 472	1600		1710	3
4	3 316	2237		2 251		2,256		2.280	4
5	110000	2.796		3,377		2 832 3 398		2 850 3 420	5 6
6	4 974 5 803	3 355		3.940		3 965		3 990	ř
7 8	6 632	1 474		1.502		4 551	6573	\$ 560 F	8
9	7.461	5 033		5,065		5 (17)5		5 1 30 1	$\frac{9}{2}$
10	8.290	3 501		5,628		5 (4.4)			10
		D'		1284		\$0'		13/	-1
		574	.817 j	15V	814	381	5 51	354	1
1	1.648	1147	1 6 13	1.154	1628	1 161	163	1168	9
3	2457	1 721	2 450	1731	2.442	1.742	2400	173	3
4	3 277	224	3 267	2 301	3 236	2311	3 2 16	3.337	4
5	4 000	2868	4 083	5 x 20	4 071	유생하		2021 3005	5 6
6	4.215	3 141	4 9 10	3 463	4 555	3 181		4 (PR)	7
7	5731	4 015	5,716	4.010	6513	4 640		4 674	8
8	6.553 7.372	4.589 5.162	7,350	5 194	7 327	5 (2:24)		525	9 +
10	8 192	5 736	8.166	5 771	8 141	3 807	8 [16]	5 54 1	10
-	33		54	45	51		54		-
D.	Dep.	Late	Dep.	Lat	Dep	Lah	Dep	Lat 1	~_
				81					

38	3°-3	9°45'		TRAV	ERSE	TAE	LES.	1	50°15′-	-54°
Г	nl	Iat	Dep.	Las	Dep	Lan	Dep.	Lat	Dep.	D.
н	-	36	275		15	3.6	30'	36	45'	
ı	-	,809	,588	.806	.591	.804	,595	.801	,598	1
и	2	1,618	1.176	1.613	1.183	1.608	1.190	1,603	1.197	2
ш	3	9,497	1.763	2.419	1.774	2.412	1.784	2,404	1.795	3
	3	3,236	2.351	3,226	2,365	3.215	2.379	3,205	2,393	4
-1	5	4,045	2,939	4.032	2,957	4.019	2,974	4,006	2,992	5
-1	6	4.854	3.527	4.839	3,548	4,823	3,569	4.808	3,590	6
н	2	5,663	4.115	5.645	4.139	5.827	4.164	5,609	4.188	7 8
	8	6.472	4,702	6,452	4.730	6.431	4.759	6.410	4.787	
	9	7.281	5,290	7,258	5,322	7.235	5.353	7.211	5.385	9
	10	8,090	5,878	8,064	5.913	8,039	5.948	8,013	5.983	10
		34	a. '	53	45/	23.		53°	10/	
		37	A COLUMN TWO IS NOT THE OWNER.	37	15'	37	_	37°	45"	_
	1	.799		.796	.605	.793	.609	.791	.612	1
	20	1.597		1,592	1.211	1,587	1.218	1,581	1.224	3
	3		The second	2.388	1.816	2,380		2.372 3.163	2.449	4
	4	3,195		3,184	2.421 3.026	3.173	3.044	3,953	3.061	5
	5 6			4.776	3,632	4.760		4.744	3,673	6
	7	5,590		5,572	4.237	5,553	4.261	5.535	4,286	7
	8	0.000							4.898	8
	9								5,510	9
	10		6,018	7.960	6,053	7.934	6.088	7.907	6.122	10
		63	Of O	23	45'	32	30'	52	15'	
		38	0'	38	15	35	30'	381	45'	
		.788					.623	.780		1
		1.570								2
	1 3									3
	_	3,15								4 5
		3,940							B. C. Company of the	100
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		6.30								-690
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	10		6.157	7.853	6,191	7,826	6,225			10
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	D,	Dep.	Lat	Dep.	Lat	Dep.	Lat	Dep.	Lat	D.
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D.	Lat	Dep	Lat	De	n.	Lat		Dep	1 4.	. 1	*	l.
		00		00 15	-		_	30		-	Dep 45	. D.
1	.76	.64	3 .76	3 6	46		60	.64		58	.65	F -
3	1.53:		6 1.52			1.5		1.29		_	1.30	_
			ALC: NAME OF TAXABLE PARTY.		38	2.2		1.94		_	1.95	
4	3.064		20022			3,0		2.59		_	2.61	
5		0.000		ACCUPATION NAMED IN	_	3.80		3.24		_	3.26	
7				COLUMN TO SERVICE SERV	_	4.50	-	3.89		_	3.91	
8			2000000		•	6.08	_	4.546		_	4,569	
9		(2)	-		_	6.84	_	5.190	_		$\frac{5.229}{5.876}$	_
10					_	7.60	-	6.494		~	5.528	-
_	50	0'	49	-			THE R. P.	30'	of the latest designation of the latest desi	9 1		-
	41	0'	41	0 15		4		30			15'	
1	.755	.656	.752	.65	9	.74	9	.663	.74	6	.666	
2	1.509	1.312			_	1.49	_	1.325		_	.332	2
3	2.264	1.968	2.256		_	2.24	_	1.988		_	.998	
5	3.019	3,280	3.007		_	2.996 3.748	-	$\frac{2,650}{3.313}$	2.98		.664	5
6	4,528	3.936	4.511	3,956	_	4.494	_	3.978	4.47		995	6
7	5,283	4.592	5,263	4.615	_	5.243	-	1.638	5.22		661	171
8	6.038	5,248	6.015	5,275	_	5.992	_	108.6	5,968	_	327	8
9	6.792	5,905	6.767	5,934		6,741	15	964	6.715		993	9
10		6,561	7.518	6.593	-	7,490	- Branch		7.461		659	10
_	49		48"	45'	-	46			48			-
-	.743	,660	740	.672	-	,737	1	676	.734	425	79	7
2	1.486	1.338	1.480	1.345		1.475	_	351	1.469		158	2
3	2 220	2.007	2.221	2.017	_	2.212	_	027	2.203	_	36	3
4	2.973	2.677	2,961	2.689		2,949	_	702	2,937	27	_	41
4 5 6	3.716	3.346	3.701	3.362	_	3,686		378	3,672	3.3	_	5
7	4.459 5.202	4.015	4.441 5.182	4.707	_	5.161		054 729	5.140	4.7		7
8	5,945	5.353	5.922	5,379		5.898		405	5,875	5.4		8
9	6.688	6.022	6.662	6.051	_	5,636		080	6,609	8.1		9
10	7.431	6.691	7.402	6.724		7,373	6.	758	7.343	6,7	88	10
	45	0,	47°	45'		47°	30		47	15	_	-1
	43°		43°	15'	-	43	30	and the last	43	-	000	
1	.731	.682	.728	1,370		.725 1.451		688	.722 1.445		383	2
3	1,463 2.194	1.364 2.046	1.457 2.185	2,056		2.176		085	2.167		175	3
4	2.925	2.728	2,913	2741		2,901	_	753	2.889	2.7	66	4
5	3.657	3.410	3,642	3.426		3.627	3,	442	3,612	3.4	58	5
6	4.388	4.092	4.370	4.111	10	4,352		130	4.334	4.3		6
7	5.119	4.774	5.099	4.796		5.078		818	5.057	4.8		7 8
8	5,851	5.456	5.827	5.481		5.803		507	5,779 6,501	6.2		9
10	6.582	6 138	6.555	6.167		6,528		884	7.224	6.9		10
10	7.314	6.820		45	-	46	_	_		15	-	
D.		_		Lat	-	Dep			Dep.	L	12	D
-	Dop.	Late	Dep.	Tot		-						

TRA	VERS	E TAI	BLES.		45°-	46
Lat	Dep.	Lat	Dep	Lat	Dep.	D,
64	15	441	30	44	45"	
716	,698	.713	,701 1,402	7.710	704	1

E	0.	Lat.	Dop	Lat	Dep.	Lat	Dep	Lat	Dep.	D,
ı.	-1	48	W.	84"	13	44	30	44	45	
u	1	.719	.690	.716	,698	.713	.701	.710	.704	
н	2	1.439	1.389	1.433	1.396	L427	1.402	1.420	1.408	2
и	3	2.158	2,084	3 143	2.093	2.140	2,103	2.131	2 112	3
1	4	2.877	2,779	2.865	2.791	2.853	2,804	2.841	2.816	4
ı	3	2,597	3.473	3,582	3,489	3,566	8,505	3,551	3.520	5
1	15	4.316	4.168	4.298	4.187	4.280	4.205	4.261	4.224	6
ı	7	5.035	4.863	5.014		4.993	4,906	4.971	4.928	7
1	8	5,755	5,557	5,730	5,582	5,706	5.607	5.682	5,632	8
ı	9	8,474	6,252	6.447	6,280	6,419	6.308	6.392	6,336	9
и	10	7,193	6.947	7,163	6.978	7.133	7.009	7.102	7.040	10
п		46' 0'		4.5	437	45	30'	45	-	
1		43	W	45	137	451	30'	45"	45'	
	1	-707	.707	.704	.710	.701	.713	.698	.716	1
	12	1,414	1.414	1.408	1.420	1.402	1 427	1,396	1.433	2
ı	3	2.121	2,121	2.112	2,131	2,103	2.140	2.093	2.149	3
в	4	2.823	2.828	2.816	2.841	2,804	2.853	2.791	2.865	4
п	3	3,836	3,536	3,520	3.551	3,505	3,566	3.489	3.582	5
1	6	4.243		4.224	4.261	4.205	4.280	4.187	4.298	6
1	7	4.950		4.928	4,971	4,906	4.993	4.885	5.014	7
	8	5,657	5,657	5,632		5,607	5.706	5.582	5.730	8
1	9	6,364		6,336	6.392	6.308	6.419	6.280	6.447	D
1	10	7,071	7,071	7.040		7.009	7.133	6.978	7,163	10
1		45	0,	44	45	44"	3.0	44	-	
	D	Dep.	Lan	Dep.	Lat	Dep.	Lat.	Dep.	Lat.	D.

MISCELLANEOUS TABLE.

WHEN DIAMETER == 1.				1.00.
Circumference of circle, z,		×	3.14159	0.49715
Area of circle,	*		.78540	9.89509-10
Contents of sphere, Earth's equatorial radius, in miles,		4	.52360	9.71900-10
Earth a polar radius, in miles,	-	4	3962.57	3,59798 3,59652
Compression, 1 + 299,1528,			0.00334	7,52411-10

EQUIVALENTS.

	American	yard foot	11 11 11 11 11 11 11 11 11 11 11 11 11	1869,40831 .21689 1.50866 .91444 .48217 .30481 .15639 .93835 .96435	nautical miles, 9.93830-19 meters, 3.20667 German geograph. miles, 9.33624-10 Russian versts, 0.17859 meters, 9.96115-10 Vienna klafter, 9.68320-10 meters, 9.48403-10 toises, 9.19421-10 Parisian feet, 9.97236-10 Vienna feet, 9.98423-10 Spanish feet, 0.03900	
1		26	-	1.09395	Spanish feet, 0.03900	

		MER	IDIONA	L PART	S.		
Deg.	0'	10"	20	30"	40	60'	
0	0.0	9.9	19.9	29.8	39.7	49.7	
1	59.6	69.5	79.5	89.4	99.3	109.3	
2 3	119.2	129.2	139.1	149.0	159,0	168.9	ľ
3	178.9	188.8	198.8	208.7	218.7	228.6	ı
4	238.6	248.6	258,5	268.5	278,4	288.4	ŀ
5	298,4	308,4	318,3	328,3	338.3	348,3	ı
6	358.3	368,3	378.2	388.2	398.2	408.2	ı
7	418.3	428.3	438.3	448.3	458.3	468,3	I
8	478.4	488.4	498.4	508.5	518,5	528.6	4
9	538.6	548,7	458.8	568.8	578.9	589.0	1
10	599.1	609,2	619.3	629,4	639,5	649,6	ŀ
11	659.7	669.8	680.0	690,1	700.2	710.4	ı
12	720.5	730,7	740.9 .	751.0	761.2	771.4	I
13	781.6	791.8	802.0	812.2	822.5	832.7	Į
14	842,9	853.2	863,4	873.7	884.0	894.2	ı
15	904.5	914.8	925.1	935.4	945,7	956,1	ı
16	966.4	976.7	987.1	997.5	1007.8	1018.2	ı
17	1028,6	1039.0	1049.4	1059.8	1070.2	1080.7	ı
18	109L1	1101.6	1112.0	1122.5	1133.0	1143,5	ı
19	1154.0	1164.5	1175,1	1185.6	1196.1	1206,7	l
20	1217.3	1227.9	1238,5	1249.1	1259.7	1270.3	ı
21	1281.0	1291.6	1302.3	1313.0	1323,7	1334.4	ı
22	1345.1	1355.8	1366,6	1377.3	1388.1	1398.9	
23	1409,7	1420.5	1431.3	1442,1	1453.0	1463,8	l
24	1474.7	1485,6	1496.5	1507.4	1518.4	1529,3 1595.4	۱
25	1540.3	1551.3	1562.3	1573.3	1004.0	1050.3	
25	1606.4	1617.5	1628.6	1639.7	1650.8	1661.9	
27	1673.1	1684.3	1695,5	1706.7	1717.9	1729.1	
28	1740.4	1751.7	1762.9	1774.3	1785,6	1796.9	
29	1808.3	1819.7	1831.1	1842.5	1854.0	1865,4 1934,6	
30	1876.9	1888.4	1899,9	1911.4	1923.0	15940	ı
31	1946.2	1957.8	1969.4	1981.1	1992.8	2004.5	
32	2016.2	2028.0	2039.7	2051.5	2063,3	2075.2	
33	2087.0	2098.9	2110.8	2122.7	2134.7	2146.7	
34	2158.6	2170.7	2182.7	2194.8	2206.9	2219.0	
35	2231.1	2243.3	2255,5	2267,7	2279.9		
36	2304.5	2316.8	2329.2	2341.5	2353,9	2366.4 2441.5	
37	2378.8	2391.3	2403.8	2416.3	2428.9	2517.7	
38	2454.1	2466.8	24795	2492.2	2504.9 2582.0	2594.9	
39	2530.5	2543.3	2556.2	2569.1 2647.1	2660.2	2673.3	
40	- 2607.9	2621.0	2634.0	20411	200018		
	3				THE RESERVE AND ADDRESS OF THE PARTY NAMED IN	07400	

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Deg.	0	10	30,	30"	40'	50'
43	2847.4	2861.1	2874.8	2888.5	2902.2	2916.0
44	2929.9	2943.7	2957.6	2971.6	2985.6	2999.6
43	3013.7	3027.8	3042.0	3056.2	3070.4	3084.7
46	3099,0	3113.4	3127.8	3142.3	3156,8	3171,3
47	3185.9	3200,5	3215.2	3230.0	3244.7	3259,6
44	3274.5	3289.4	3304.3	3319.4	3334,4	3349,6
435	3364.7	3380.0	3395.2	3410,6	3425,9	3441.4
513	3456,9	3472.4	3488.0	3503.7	3519,4	3535,1
51	3550,9	3566.8	3582.8	3598.7	3614.8	3630,9
-53	3647.1	3663.2	3679,6	3696.0	3712.4	3728,9
22	3745.4	3762.0	3778.7	3795.4	3812.2	3829,1
54	3846.0	3863.1	3880.1	3897.3	3914.5	3931,8
130	3949.1	3966.6	3984.1	4003.7	4019.3	4037.0
56	4054.8	4072.7	4090.7	4108,7	4126.9	4145.1
57	4163.3	4181.7	4200.2	4218.7	4237.3	4256.0
58	4274.8	4293.7	4312.7	4331.7	4350.9	4370,1
59 60	4389.4	4408,9	4428,4	4448.0	4467,7	4487.5
00	4507.5	4527,5	4547.6	4567,8	4588.1	4608,6
61	4629.1	4649.8	4670,5	4691.4	4712:4	4733.5
63	4754.7	4776.0	4797,5	4819.0	4840,7	4862.5
53	4884.5	4906,5	4928.7	4951.0	4973.5	4996,0
64	5018.8	5041.6	5064.6	5087.7	5111.0	5134.4
00	01000	5181.7	5205,5	5229,5	5253,7	5278.0
66	5302.5	5327.1	5351,9	5376.9	5402.1	5427.4
67	5452,8	5478,5	5504.3	5530,3	5556,5	5582.9
68	5609,5	5636,3	5663,2	5690,4	5717.7	5745.3
70	5773,1	5801.1	5829.3	5857.7	5886.3	5915.2
10	5944.3	5973,6	6003,2	6033,0	6063,1	6093,4
71	6124.0	6154.8	6185,9	6217.2	6248,9	6280,8
72	6313.0	6345,5	6378.2	6411.3	6444.7	6478.4
73	6512.4	6546.8	6581.5	6616.5	6651.8	6687.6
74	6723,6	6760.1	6796,9	6834.1	6871.7	6909.7
-75	6948.1	6987.0	7026,2	7065,9	7106,1	7146.7
76	7187.8	7229.3	7271.4	7313.9	7357,0	7400.6
77	7444,8	7489.5	7534,8	7580.7	7627.0	7674.3
78	7722.1	7770,5	7819.6	7869,4	7919.9	7971.1
79	8023.1	8075,9	8129.5	8184,0	8239,3	8295.4
80	8352,5	8410.6	8469.6	8529,7	8590.8	8653.0
81	8716.3	8780.9	8846.6	8913.6	8981.9	9051.6
82	9122.7	9195.3	9269,4	9345.2	9422.7	9501.9
83	9583,0	9666,0	9751.1	9838.3	9927.8	10019,6
84	10114.0	10211.0	10310.8	10413.6	10519.6	10628.8
85	10741.7	10858.4	10979,2	11104.3	11234.2	11369.1

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